

CITY OF SACRAMENTO
1231 I Street, Sacramento, CA 95814

Permit No: 0500295

Insp Area: 4
Thos Bros: 278B3

Site Address: 3720 PRESIDIO ST SAC
Parcel No: 252-0131-016

Sub-Type: NSFR
Housing (Y/N): N

CONTRACTOR

OWNER
HUSK JEFFERY W
7006 WOODMORE OAKS DR
ORANGEVALE CA 95662

ARCHITECT

Nature of Work: SFR 1,324 sf, att garage 464sf., covr patio 123sf & porch 60sf

CONSTRUCTION LENDING AGENCY : I hereby affirm under penalty of perjury that there is a construction lending agency for the performance of the work for which this permit is issued (Sec. 3097, Civ. C).

Lender's Name _____ Lender's Address _____

LICENSED CONTRACTORS DECLARATION: I hereby affirm under penalty of perjury that I am licensed under provisions of Chapter 9 (commencing with section 7000) of Division 3 of the Business and Professions Code and my license is in full force and effect.

License Class _____ License Number _____ Date _____ Contractor Signature _____

OWNER-BUILDER DECLARATION: I hereby affirm under penalty of perjury that I am exempt from the contractors License Law for the following reason (Sec. 7031.5, Business and Professions Code; any city or county which requires a permit to construct, alter, improve, demolish, or repair any structure, prior to its issuance, also requires the applicant for such permit to file a signed statement that he or she is licensed pursuant to the provisions of the Contractors License Law (Chapter 9 (commencing with Section 7000) of Division 8 of the Business and Professions Code) or that he or she is exempt therefrom and the basis for the alleged exemption. Any violation of Section 7031.5 by any applicant for a permit subjects the applicant to a civil penalty of not more than five hundred dollars (\$500.00);

I, as a owner of the property, or my employees with wages as their sole compensation, will do the work, and the structure is not intended or offered for sale (Sec. 7044, Business and Professional Code: The Contractors License Law does not apply to an owner of property who builds or improves thereon, and who does such work himself or herself or through his/her own employees, provided that such improvements are not intended or offered for sale. If, however, the building or improvement is sold within one year of completion, the owner-builder will have the burden of proving that he/she did not build or improve for the purpose of sale.)

I, as owner of the property, am exclusively contracting with licensed contractors to construct the project (Sec. 7044, Business and Professions Code: The Contractors License Law does not apply to an owner of property who builds or improves thereon, and who contracts for such projects with a contractor(s) licensed pursuant to the Contractors License Law).

I am exempt under Sec. _____ B & PC for this reason: _____

Date _____ Owner Signature _____

IN ISSUING THIS BUILDING PERMIT, the applicant represents, and the city relies on the representation of the applicant, that the applicant verified all measurements and locations shown on the application or accompanying drawings and that the improvement to be constructed does not violate any law or private agreement relating to permissible or prohibited locations for such improvements. This building permit does not authorize any illegal location of any improvement or the violation of any private agreement relating to location of improvements.

I certify that I have read this application and state that all information is correct. I agree to comply with all city and county ordinances and state laws relating to building construction and hereby authorize representative(s) of this city to enter upon the abovementioned property for inspection purposes.

Date 8-18-05 Applicant/Agent Signature _____

WORKER'S COMPENSATION DECLARATION: I hereby affirm under penalty of perjury one of the following declarations:

I have and will maintain a certificate of consent to self-insure for workers' compensation as provided for by Section 3700 of the Labor Code, for the performance of work for which the permit is issued.

I have and will maintain workers' compensation insurance, as required by Section 3700 of the Labor Code, for the performance of the work for which this permit is issued. My workers' compensation insurance carrier and policy number are:

Carrier _____ Policy Number _____ Exp Date _____

(This section need not be completed if the permit is for \$100 or less) I certify that in the performance of the work for which this permit is issued, I shall not employ any person in any manner so as to become subject to the workers' compensation laws of California and agree that if I should become subject to the workers' compensation provisions of Section 3700 of the Labor Code, I shall forthwith comply with these provisions.

Date 8-18-05 Applicant Signature _____

WARNING: FAILURE TO SECURE WORKER'S COMPENSATION COVERAGE IS UNLAWFUL AND SHALL SUBJECT AN EMPLOYER TO CRIMINAL PENALTIES AND CIVIL FINES UP TO ONE HUNDRED THOUSAND DOLLARS (\$100,000) IN ADDITION TO THE COST OF COMPENSATION, DAMAGES AS PROVIDED FOR IN SECTION 3706 OF THE LABOR CODE, INTEREST AND ATTORNEY'S FEE.

THIS PERMIT SHALL EXPIRE BY LIMITATION IF WORK IS NOT COMMENCED WITHIN 180 DAYS.

PERMIT NO.
0500295

AREA NO.
4

CITY OF SACRAMENTO
2101 ARENA BLVD, #200
BUILDING INSPECTIONS DIVISION

WHEN CORRECTIONS HAVE BEEN MADE, CALL 808-7622 FOR REINSPECTION OF WORK.

JOB LOCATION 3720 Presidio

INSPECTION REQUESTED ~~FR~~ Roof

THE UNDERSIGNED BUILDING PLUMBING MECHANICAL ELECTRICAL
INSPECTOR THIS DAY INSPECTED THIS STRUCTURE FOR THE REQUESTED INSPECTION AND FOUND THE FOLLOWING VIOLATIONS OF CITY AND/OR STATE LAWS GOVERNING SAME:

1) Complete lateral
A-S through A-8

Ca 1WSU/Action *
Kenney

717-2632

INSPECTOR Vic Gumbert DATE 12/2/05

BUILDING INSPECTIONS 808-5716

INSPECTOR'S COPY



Downtown Permit Center
 1231 I Street, Suite 200
 Sacramento, CA 95814
 Help Line: 1-916-264-5656

CITY OF SACRAMENTO
 DEVELOPMENT SERVICES DEPARTMENT
 BUILDING DIVISION
www.cityofsacramento.org

North Permit Center
 2101 Arena Blvd., Suite 200
 Sacramento, CA 95834
 Inspection: 1-916-808-4577

SITE DRAINAGE AND ENCROACHMENT QUESTIONNAIRE

PARCEL # 252-0131-016 PERMIT # _____
 SITE ADDRESS 3720 Presidio Street ACREAGE _____

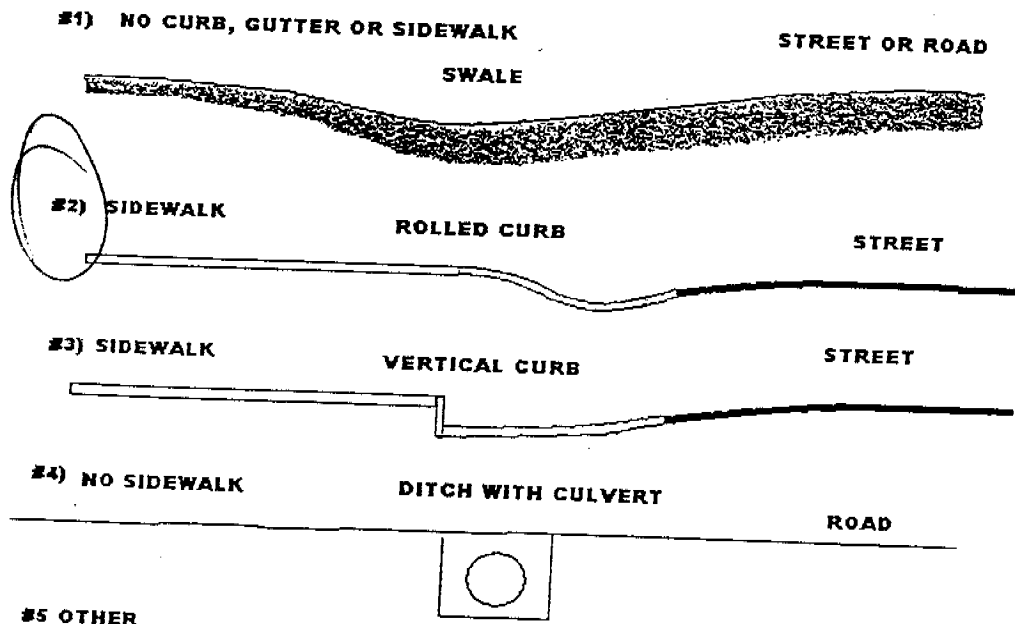
The City of Sacramento requires a building site to be graded to drain correctly and site drainage routed to an approved location. To help us understand the site drainage for your project and determine if a driveway permit or an encroachment permit is required please answer the following questions. All questions must be answered.

- | | | |
|--|-------------------------------------|---|
| 1. Are there existing structures on the site? | Y | <input checked="" type="radio"/> N |
| 2. Is there an existing concrete or paved driveway to this parcel from the street? | Y | <input checked="" type="radio"/> *N |
| 3. Will the existing access to this parcel be changed in any way for this project? | *Y | <input checked="" type="radio"/> N |
| 4. Are all portions of the lot higher than the crown of the street? | <input checked="" type="radio"/> Y | *N |
| 5. Are all portions of the lot higher than the back of the sidewalk? | <input checked="" type="radio"/> Y | *N |
| 6. Is there a curb and gutter at the street level? | <input checked="" type="radio"/> *Y | N |
| 7. Is there a sidewalk with a curb and gutter at the street? | <input checked="" type="radio"/> *Y | N |
| 8. Is the curb at the street square? | *Y | <input checked="" type="radio"/> N N/A |
| 9. Is there a rolled curb at the street? | <input checked="" type="radio"/> Y | N N/A |
| 10. Is there a drainage ditch or culvert at the street? | Y | *N <input checked="" type="radio"/> N/A |
| 11. Does the lot drain from back to front? | <input checked="" type="radio"/> Y | *N |
| 12. Does the lot drain from front to rear? | Y | <input checked="" type="radio"/> *N |
| 13. Does another lot drain across this parcel? | *Y | <input checked="" type="radio"/> N |
| 14. Does the lot drain from side to side? | <input checked="" type="radio"/> *Y | N |
| 15. Does the site have an existing low area or drainage swale? | *Y | <input checked="" type="radio"/> N |
| 16. Does the drainage swale drain to an adjacent parcel? | *Y | <input checked="" type="radio"/> N N/A |
| 17. Does the drainage swale drain to the street? | <input checked="" type="radio"/> Y | *N N/A |
| 18. Will existing drainage be re-routed? | *Y | <input checked="" type="radio"/> N |
| 19. Will drainage ditches or culverts be constructed or modified? | *Y | <input checked="" type="radio"/> N N/A |
| 20. Did this project require approval from the Zoning Administrator? | *Y | <input checked="" type="radio"/> N |
| 21. Did the project require approval from the Planning Administrator? | <input checked="" type="radio"/> *Y | N |

SITE DRAINAGE AND ENCROACHMENT QUESTIONNAIRE

- | | | |
|---|----|----------|
| 22. Is there any tree, telephone pole, guy wire or similar obstruction located at the front of the property adjacent to the street or road? | *Y | (N) |
| 23. Is this a corner lot? | *Y | (N) |
| 24. Is the posted speed limit on this street greater than 25 MPH? | *Y | (N) |
| 25. Is this parcel located on a four-lane street? | *Y | (N) |
| 26. If site is greater than 1/2 acre has an erosion and sediment control plan been submitted? | Y | *N (N/A) |
| 27. If site disturbs 1 acre or more has a copy of the State General Permit NOI and SWPPP been submitted? | Y | *N (N/A) |
| 28. If site is part of a larger subdivision greater than 1 acre has a copy of the State General Permit NOI and SWPPP been submitted? | Y | *N (N/A) |

CIRCLE THE DRAWING NUMBER BELOW THAT BEST ILLUSTRATES THE EXISTING CONDITION AT THE LOCATION OF THE PROPOSED DRIVEWAY OR SITE ACCESS.



PROVIDE
DETAIL HERE

The information provided on this document is accurate. I understand that if this form is incomplete, contains inaccurate or misleading information, the project located at this address may be delayed until any drainage or encroachment issues are resolved to the satisfaction of the City of Sacramento.

SIGNED [Signature] DATE 8/10/10
 TITLE Manager
 PHONE NO. 916 331 2410

April 13, 2005

Power of Attorney

To Whom It May Concern:

Sheryl's LLC hereby grants Power of Attorney to Richard Hobbs to pull Permits and handle any and all legal issues pertaining to 3720 Presidio Street, Sacramento, CA

If you have any questions, please feel free to contact me at 916.334.9467

A handwritten signature in black ink, appearing to read "Rob Martinson", with a long horizontal line extending to the right.

Rob Martinson
Agent/Manager
916-334-9467tel
916-334-9541fax
erasma@aol.com



February 10, 2006

HUSK JEFFERY W
7006 WOODMORE OAKS DR
ORANGEVALE CA 95662

Subject: **PERMIT EXPIRATION**

Permit Due to Expire on: March 20, 2006

Inspection Area: 4

Property at: 3720 PRESIDIO ST
Activity #: 0500295
Issued: August 18, 2005
Owner: HUSK JEFFERY W
Contractor:
Nature of Work: SFR 1,324 sf, att garage 464sf., covr patio 123sf & porch 60sf

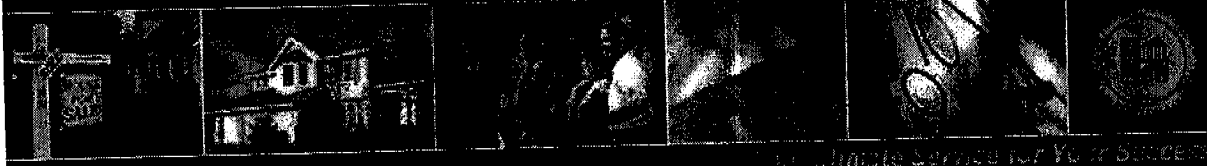
Your attention is directed to the current status of your permit: **ISSUED**

Each building permit issued by the Planning and Building Department shall expire by limitation and become null and void if the building or work authorized by such permit is not commenced within 180 days from the date of such permit, or if the building or work authorized by such permit is suspended or abandoned at any time after the work is commenced for a period of 180 days.

Our files show no progress on your permit. Please call 916-808-7622 for a progress or final inspection BEFORE the above permit expiration date. If no contact has been made with our office by the above expiration date, we will assume this project has been abandoned and therefore consider your permit expired.

You are further notified that once this permit has expired, a **NEW PERMIT** must be obtained before any further work is done and you will be required to make any changes necessary to meet the requirements of the present Sacramento City Code.

By: *Scott Ensor*
Scott Ensor
Building Inspections Division



Primary Owner: SHERYLS LLC,

Secondary Owner:

Mail Address: 7031 WATT AVE
NORTH HIGHLANDS CA 95660

Site Address: 3720 PRESIDIO ST
SACRAMENTO CA 95838

Assessor Parcel Number: 252-0131-016

Phone: - -

Census Tract: 0064.00

Housing Tract Number:

Lot Number:

Page Grid: 278 -B3

Legal Description: Abbreviated Description: S 50 FT OF N 350 FT OF W 1/2 BLK 24 E
DEL PASO H TS ADD NO 1

Property Characteristics:

Bedrooms :	Year Built :	Square Feet :
Bathrooms :	Garage :	Lot Size : 6,098 SF
Total Rooms :	Fireplace :	Number of Units : 0
Zoning : R1:SINGLE FAMILY	Pool :	Use Code : Residential-Vacant Land

Sale & Loan Information

Transfer Date : 12/13/2004	Seller : HUSK, JEFFERY W	
Transfer Value : \$120,000	Document # : BK-PG: 20041213-2829	Cost/Sq. Feet : N/A
First Loan Amount : \$230,000	Lender : HORAN INVESTMENT CO MTG	

Assessment & Tax Information

Assessed Value : \$120,000	Percent Improvement :	Homeowner Exemption :
Land Value : \$120,000	Tax Amount : \$130.42	Tax Rate Area : 3-089
Improvement Value :	Tax Account ID :	Tax Status : Current
Market Improvement Value :	Market Land Value :	Market Value:

Data Deemed Reliable, But Not Guaranteed.
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JWH CUSTOM HOMES

P.O. Box 601177 Sacramento, CA 95860
Phone: 916-486-1905 Fax: 916-486-1567
CA License # 832269

March 13, 2006

Mr. Scott Ensor
City of Sacramento
Building Inspections Division
2101 Arena Blvd, Suite 200
Sacramento, CA 95814

Activity Number: 0500295
Property at: 3720 Presidio Street

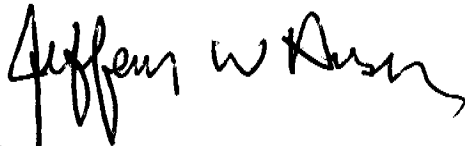
Dear Mr. Ensor,

In reference to the property listed above, the construction of this single family residence is not being done by my company. My partner and I purchased the vacant lot on Presidio Street in September of 2004 and within a few months, sold it to Sheryls, LLC (see attached tax scroll).

I would presume that any issues regarding the permit and its expiration should be directed to them.

If you have any further questions, please feel free to call Brandon at my office at 916-486-1905.

Sincerely,



Jeffery W. Husk



Custom Homes

SERVING SACRAMENTO, EL
DORADO AND PLACER COUNTIES



3706 Mission Ave - Suite 27 - Carmichael, CA
 Phone (916) 485-4100 Fax (916) 485-4105

Truss Calculation Transmittal

CUSTOMER: MEEKS E.G.

TRACKING #: J5-277

PROJECT: FOBES / PRESIDIO

ATTENTION:

CITY COPY

PLAN: SFD

DATE: 8-22-05

ELEV: ROOF

SETS:

OPTION: N/A

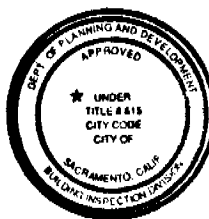
MANUFACTURER: LATHAM TRUSS

BLDG DPT	ARCHITECT	PROJECT ENGINEER	CONTRACTOR
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NOTES:

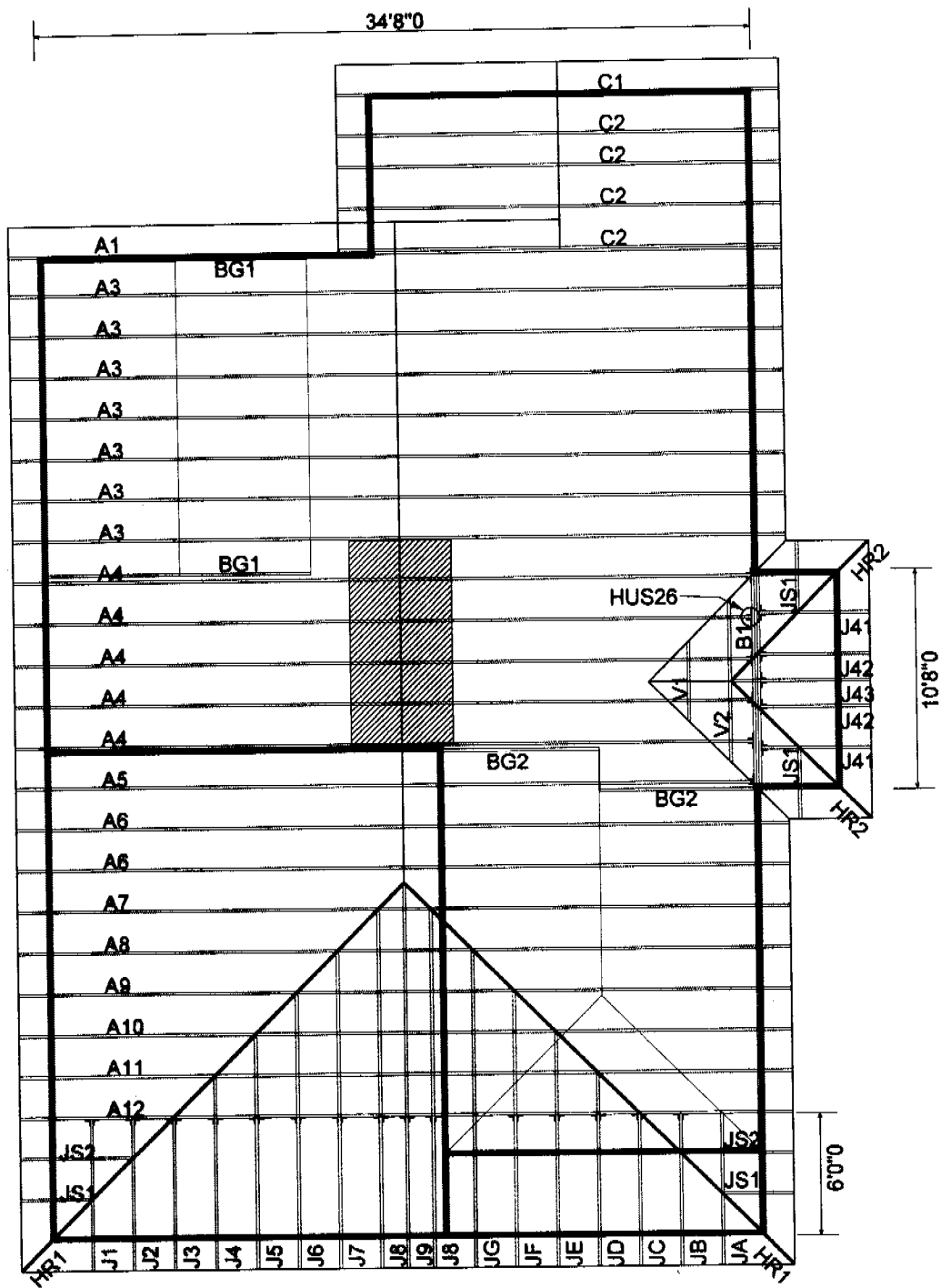
- 1) All lateral bracing specified is for bracing compression members and must be installed. Top chords are assumed to be laterally braced by roof sheeting or spaced sheeting. Bottom chords are to be laterally braced by 1x4 @10'-0" O.C. maximum or rigid ceiling.
- 2) Verification of design criteria, loading, deflection, framing methods, wind bracing and other lateral bracing that is always required is the responsibility of the Building Designer / Project Engineer. All truss designs must be reviewed and approved by Building Designer / Project Engineer prior to manufacturing.
- 3) Due to variations in weather, lumber dimensions and moisture content at the time of fabrication, the truss manufacturer will not be responsible for dimension variances of 1/4" or less.

CITY COPY
 4) Do not cut or alter trusses.



This set of plans and specifications must be kept on the job at all times and it is unlawful to make any changes or alterations from the same without written permission from the Building Inspection Division. The approval of this plan and specification SHALL NOT be held to permit or approve the violation of any City Ordinance or State Law.

SR 10/12/05
05-00295
 Revision to
 3742 PRESIDIO ST.



PRECISION TRUSS, INC.
 EXCELLENCE IN COMPONENT DESIGN
 3706 Mission ave. Ste 27 - Carmichael, CA 95821
 Phone (916) 485-4100 Fax (916) 485-4105

CUSTOMER: MEEKS - E.G.		DESIGNER: BART THOMPSON	
PROJECT: FORBES / PRESIDIO		SCALE: 1/8" = 1'	
PLAN: SFD	SITE ADDRESS: 3720 PRESIDIO ST		
ELEV: ROOF	CONTACT: ED	PH) (916) 412-4143	
OPTION: N/A		JOB # J5-277	

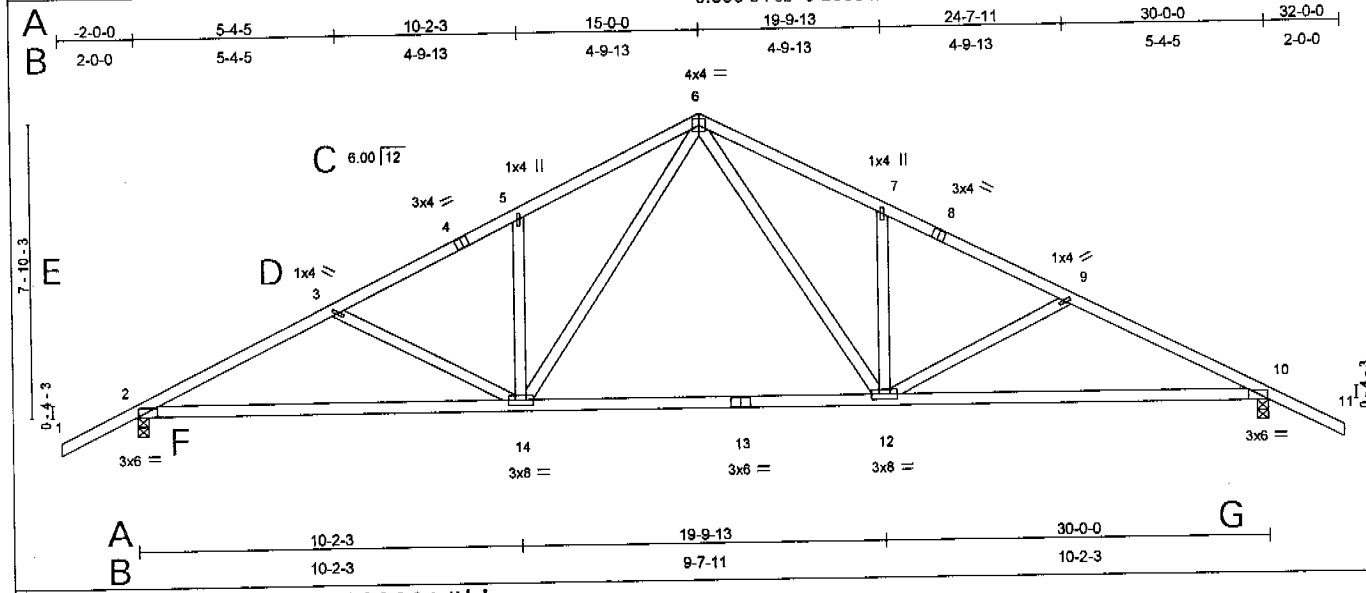


Plate Offsets (X,Y): [2:0-3-0,0-1-4], [10:0-3-0,0-1-4]

LOADING (psf)	SPACING	J M CSI	N DEFL	P PLATES	GRIP
TCLL 20.0	2-0-0	TC 0.29	Vert(LL) -0.09 14 >999	MI20	249/190
TCDL 10.0	Plates Increase 1.15	BC 0.83	Vert(TL) -0.39 12-14 >907		
BCLL 0.0	Lumber Increase 1.15	WB 0.36	Horz(TL) 0.07 10 n/a		
BCDL 10.0	Rep Stress Incr YES		1st LC LL Min l/defl = 240	Weight: 158 lb	
	Code BOCA/ANSI95				

LUMBER
 TOP CHORD 2 X 4 SYP No.2
 BOT CHORD 2 X 4 SYP No.2
 WEBS 2 X 4 SYP No.3

BRACING
 TOP CHORD Sheathed or 4-2-1 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 8-6-11 oc bracing.

REACTIONS (lb/size) 2=1317/0-3-8, 10=1317/0-3-8
 Max Horz 2=-175(load case 5)
 Max Uplift 2=-341(load case 4), 10=-341(load case 5)

FORCES (lb) - First Load Case Only
 TOP CHORD 1-2=26, 2-3=-2024, 3-4=-1722, 4-5=-1722, 5-6=-1722, 6-7=-1722, 7-8=-1722, 8-9=-1722, 9-10=-2024, 10-11=26
 BOT CHORD 2-14=1794, 13-14=1140, 12-13=1140, 10-12=1794
 WEBS 5-14=-294, 7-12=-294, 3-14=-288, 6-14=742, 6-12=742, 9-12=-288

NOTES
 1) Unbalanced roof live loads have been considered for this design.
 2) Wind: ASCE 7-98 per BOCA/ANSI95; 90mph; h=25ft; TCCL=5.0psf; BCDL=5.0psf; occupancy category II; exposure C; enclosed; MWFRS gable end zone; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.33; plate grip DOL=1.33.
 3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 341 lb uplift at joint 2 and 341 lb uplift at joint 10.

- LOAD CASE(S)** Standard
- | | |
|--|---|
| A Cumulative Dimensions | M TC, BC, and Web Maximum Combined Stress Indices |
| B Panel Length (feet - inches - sixteenths) | N Deflections (inches) and Span to Deflection Ratio |
| C Slope | O Input Span to Deflection Ratio |
| D Plate Size and Orientation | P MiTek Plate Allowables (PSI) |
| E Overall Height | Q Lumber Requirements |
| F Bearing Location | R Reaction (pounds) |
| G Truss Span (feet - inches - sixteenths) | S Minimum Bearing Required (inches) |
| H Plate Offsets | T Maximum Uplift and/or Horizontal Reaction if Applicable |
| I Design Loading (PSF) | U Required Member Bracing |
| J Spacing O.C. (feet - inches - sixteenths) | V Member Axial Forces for Load Case 1 |
| K Duration of Load for Plate and Lumber Design | W Notes |
| L Code | X Additional Loads/Load Cases |



MAX LOADING (psf)	SPACING	2-0-0
TCLL 25.0	Plates Increase	1.15
TCDL 14.0	Lumber Increase	1.15
BCLL 0.0	Rep Stress Incr	YES
BCDL 10.0		
MINIMUM LUMBER SIZE AND GRADE		
TOP CHORD	2 X 4 HF, DF-L No.1	
BOT CHORD	2 X 4 SPF, HF, DF-L No.2	

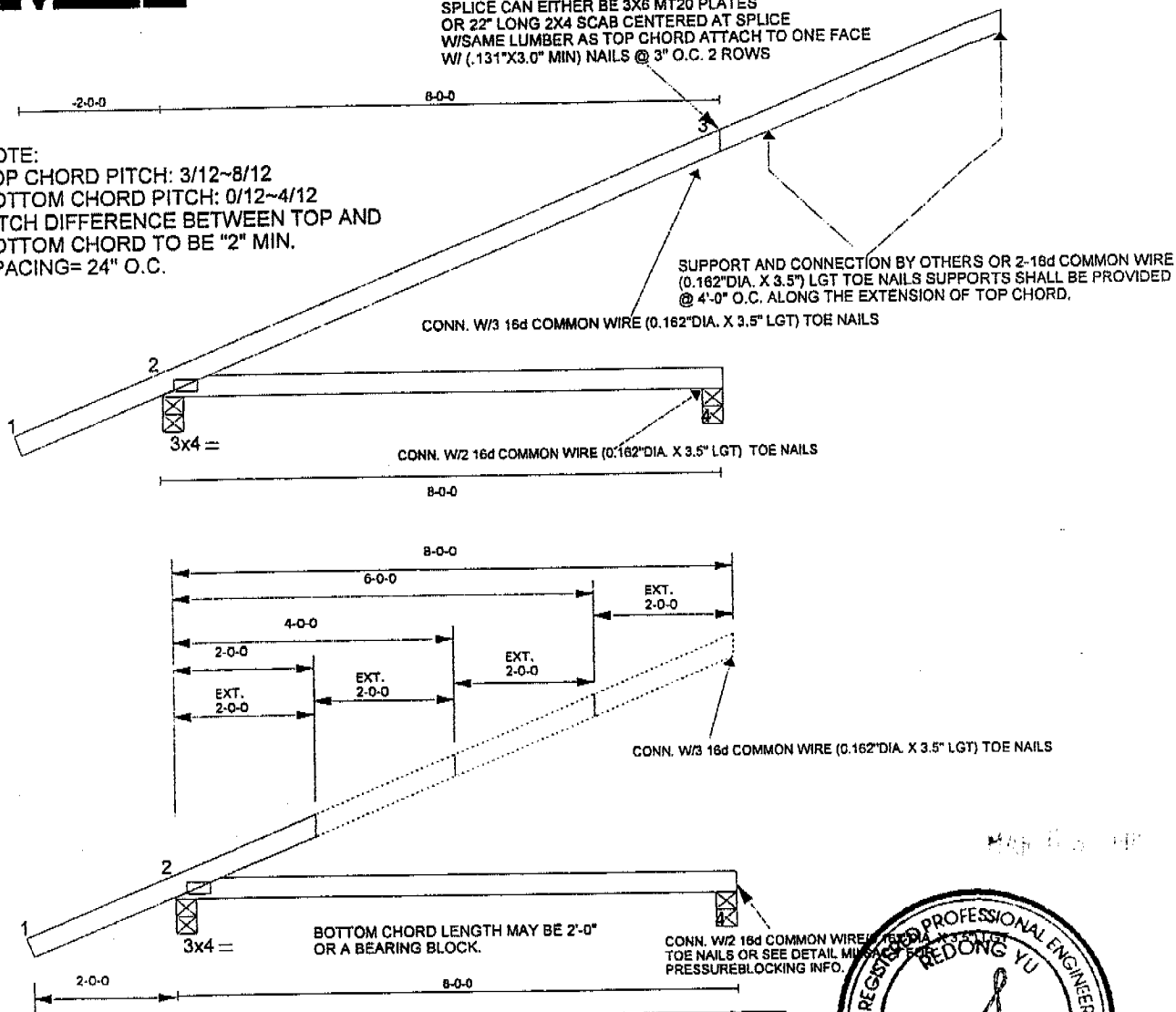
BRACING
 TOP CHORD Sheathed.
 BOT CHORD Rigid ceiling directly applied.

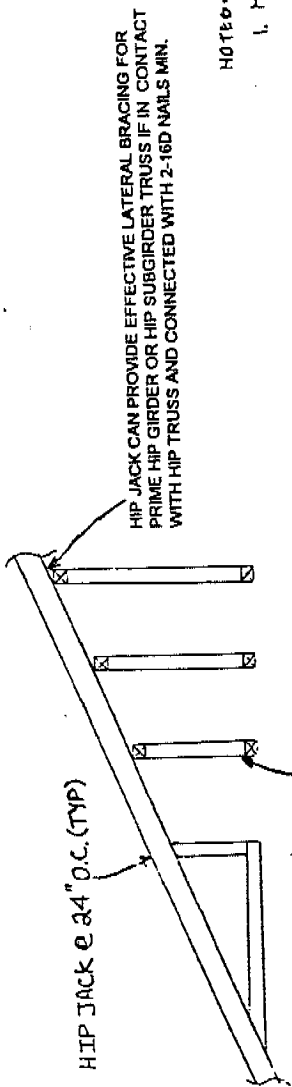
MITek Industries, Inc.
 Western Division

LENGTH OF EXTENSION
 AS DESIGN REQ'D 20'-0" MAX

SPLICE CAN EITHER BE 3X6 MT20 PLATES
 OR 22" LONG 2X4 SCAB CENTERED AT SPLICE
 W/SAME LUMBER AS TOP CHORD ATTACH TO ONE FACE
 W/ (.131"X3.0" MIN) NAILS @ 3" O.C. 2 ROWS

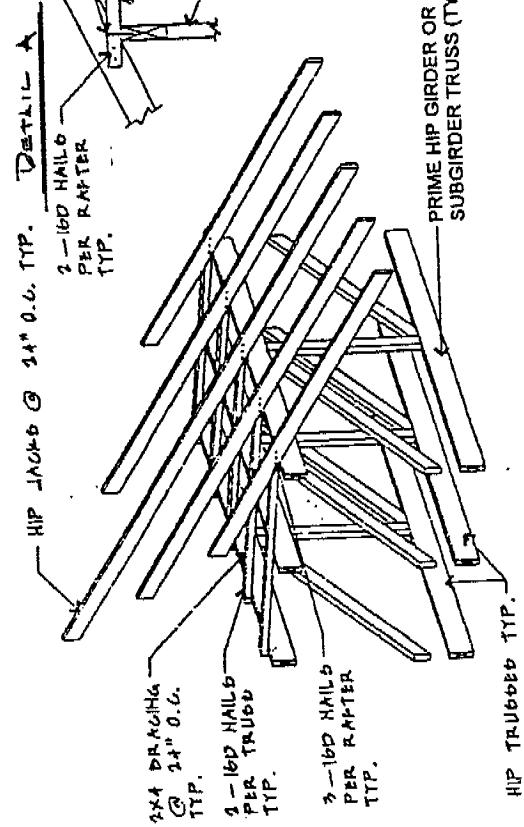
NOTE:
 TOP CHORD PITCH: 3/12~8/12
 BOTTOM CHORD PITCH: 0/12~4/12
 PITCH DIFFERENCE BETWEEN TOP AND
 BOTTOM CHORD TO BE "2" MIN.
 SPACING= 24" O.C.





HIP JACK CAN PROVIDE EFFECTIVE LATERAL BRACING FOR PRIME HIP GIRDER OR HIP SUBGIRDER TRUSS IF IN CONTACT WITH HIP TRUSS AND CONNECTED WITH 2-16D NAILS MIN.

PRIME HIP GIRDER OR HIP SUBGIRDER TRUSS (TYP)



HIP JACK @ 24" O.C. TYP. DETAIL A

2-16D NAIL PER RAFTER TYP.

2x4 BRACING @ 24" O.C. TYP.

3-16D NAIL PER TRUSS TYP.

3-16D NAIL PER RAFTER TYP.

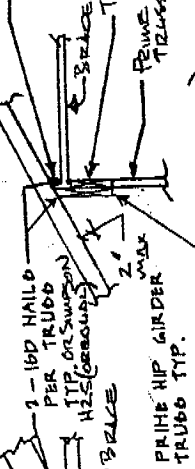
PRIME HIP GIRDER OR HIP SUBGIRDER TRUSS (TYP)

HIP TRUSSED TYP.

NOTED:

1. MAXIMUM LOADING T.C. 30 POF LL 15 POF DL
2. 15% STRESS INCREASE
3. BRACING DETAIL FOR LATERAL SUPPORT OF TOP CHORD ONLY. BRACING HAS NOT BEEN DESIGNED FOR WIND OR SEISMIC LOADS.
4. SEE DETAILED ENGINEERING FOR TRUSSES AND HIP JACKS.

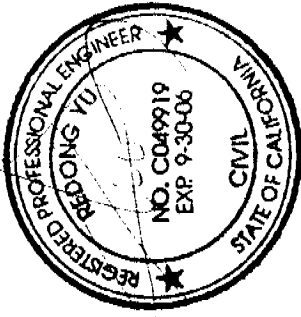
DETAIL B



2x6 (MIN) LEDGER ATTACH TO TRUSS T.C. WITH 2-ROWS 10d NAILS @ 12" O.C.

NOTE:

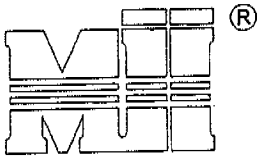
RESTRAINT REQUIRED AT EACH END OF BRACE AND AT 20'-0" INTERVALS.
REFER TO HIP-91 SUMMARY SHEET FOR RECOMMENDATIONS OF THE TRUSS PLATE INSTITUTE.



HIP TRUSS LATERAL BRACING DETAIL

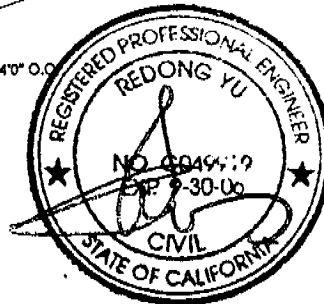
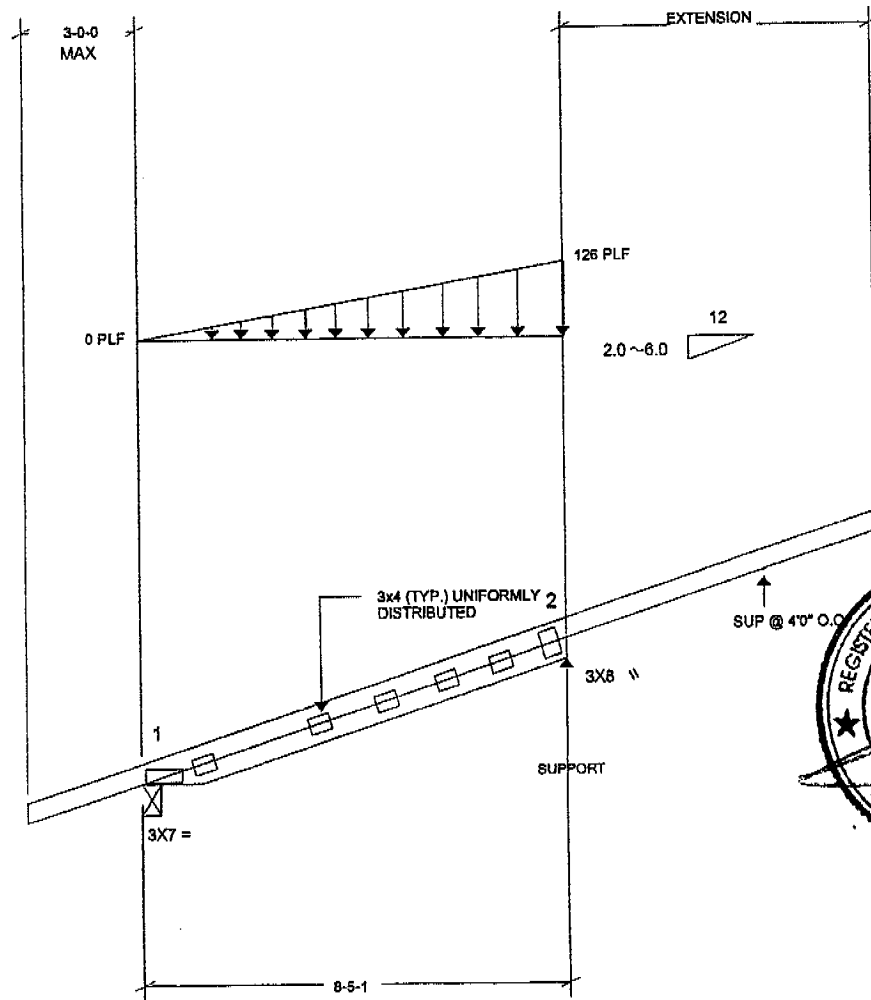
2 JULY 2003

CORN16/14-2X4



MITek Industries, Inc.

NOTE: ALL CONNECTIONS TO SUPPORTS BY OTHERS



MAY 13 2003

PLATES ARE MITEK M20

MINIMUM GRADE OF LUMBER
TOP CHORD: 2X4 NO.2 DF-G

LOADING (PSF)
L D
TOP 16 14

REACTIONS
JT 1= 177
JT 2= 354

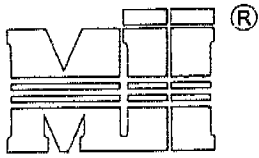
STR. INC.: LUMB = 1.25 PLATE = 1.25
REPETITIVE STRESSES NOT USED

SPACING: 24.0 IN. O.C.
NO. OF MEMBERS = 1

PLATE MUST BE INSTALLED ON EACH FACE OF JOINT, SYMMETRICALLY (EXCEPT AS SHOWN) DESIGN CONFORMS WITH NDS DESIGN SPECS, UBC97/ANSI96
THIS DESIGN IS FOR TRUSS FABRICATION ONLY. FOR PERMANENT AND TEMPORARY BRACING (WHICH IS ALWAYS REQ'D) CONSULT BLDG ARCHITECT OR ENGINEER

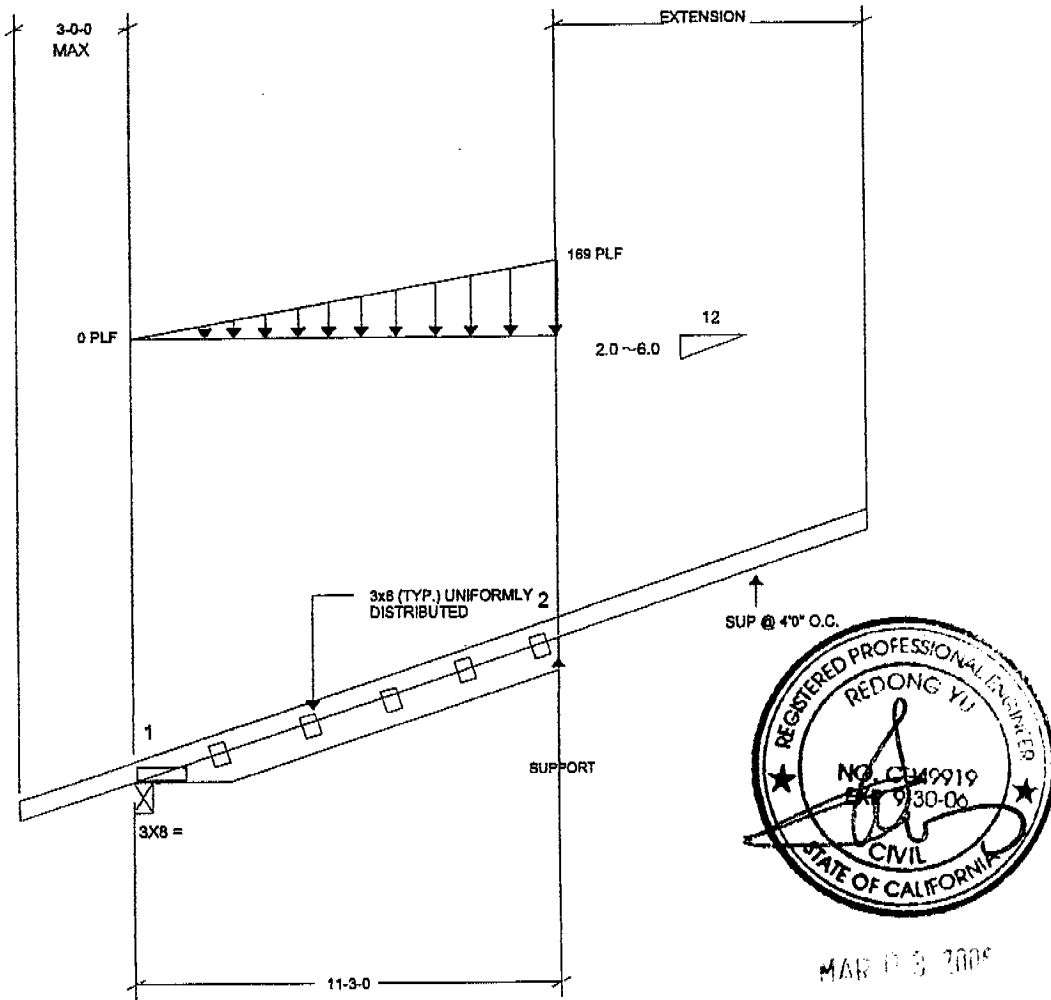
2 JULY 2003

CORN16/14-2X4



MITek Industries, Inc.

NOTE: ALL CONNECTIONS TO SUPPORTS BY OTHERS



PLATES ARE MITEK M20

MINIMUM GRADE OF LUMBER
TOP CHORD: 2X4 NO. 2 DF-G EXCEPT
1-2 2X6 DF-G No. 2

LOADING (PSF)
L D
TOP 16 14

REACTIONS
JT 1= 317
JT 2= 634

STR. INC.: LUMB = 1.25 PLATE = 1.25
REPETITIVE STRESSES NOT USED

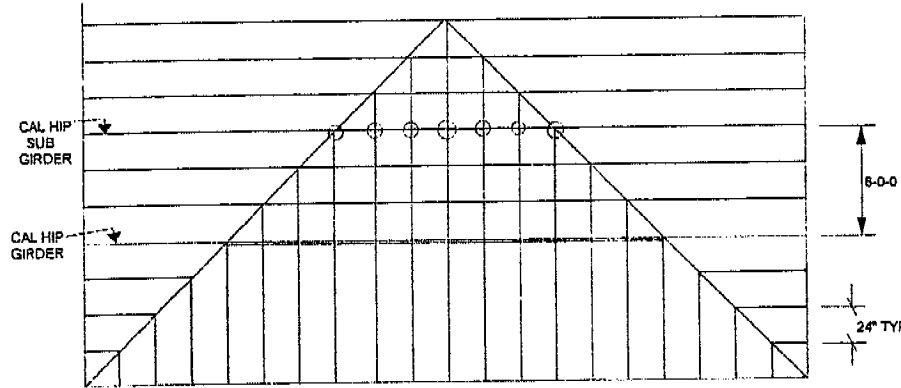
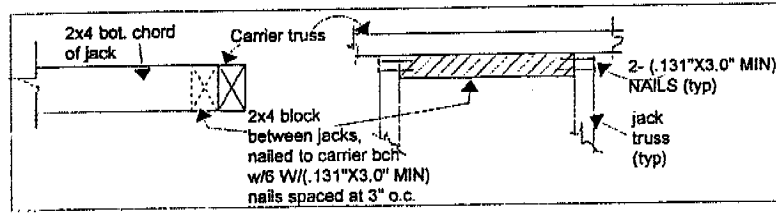
SPACING: 24.0 IN. O.C.
NO. OF MEMBERS = 1

PLATE MUST BE INSTALLED ON EACH FACE OF JOINT, SYMMETRICALLY (EXCEPT AS SHOWN) DESIGN CONFORMS WITH NDS DESIGN SPECS, UBC97/ANSI95
THIS DESIGN IS FOR TRUSS FABRICATION ONLY. FOR PERMANENT AND TEMPORARY BRACING (WHICH IS ALWAYS REQ'D) CONSULT BLDG ARCHITECT OR ENGINEER

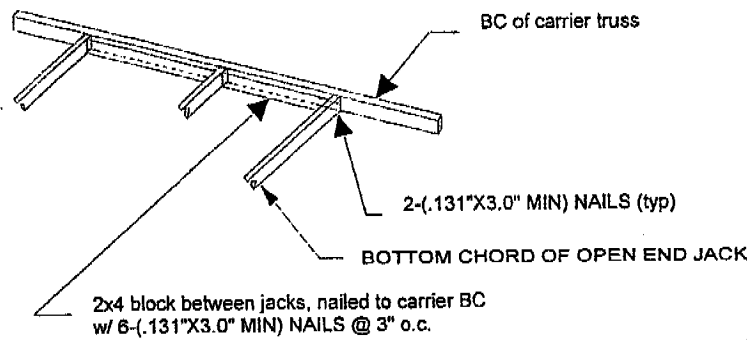


Loading (PSF):

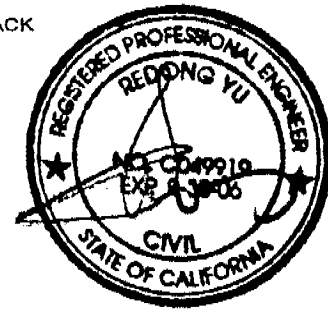
BCDL 10.0 PSF MAX



PARTIAL FRAMING PLAN OF CALIFORNIA HIP SET WITH SUB GIRDER



APR 29 2005



WARNING - Verify design parameters and READ NOTES ON THIS AND REVERSE SIDE BEFORE USE.
 Design valid for use only with MITek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

7777 Greenback Lane
Suite #109
Citrus Heights, Ca 95610

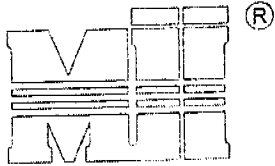


JANUARY 21, 2000

L-BRACE DETAIL

ST - L-BRACE

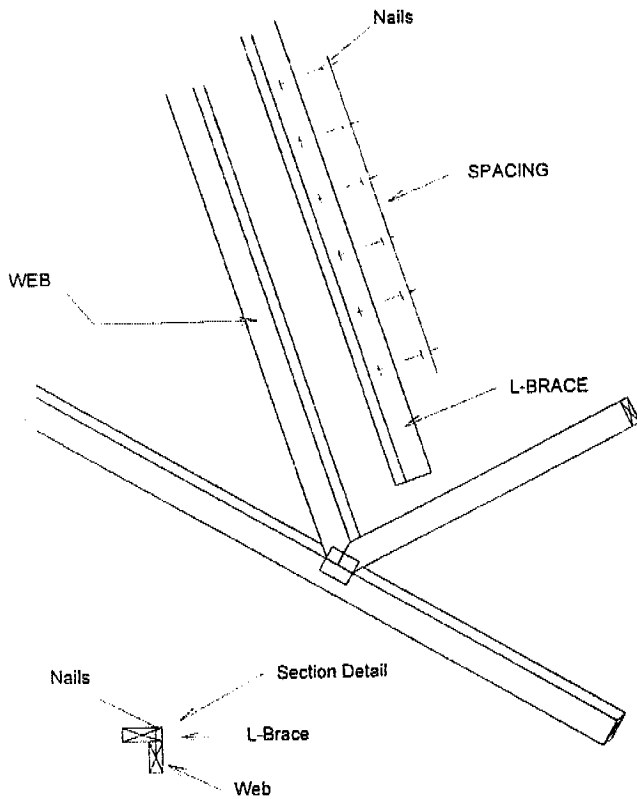
MiTek Industries, Inc. Chesterfield, MO.



Nailing Pattern		
L-Brace size	Nail Size	Nail Spacing
1x4 or 6	10d	8" o.c.
2x4, 6, or 8	16d	8" o.c.

Note: Nail along entire length of L-Brace
(On Two-Ply's Nail to Both Plies)

Note: L-Bracing to be used when continuous lateral bracing is impractical. L-brace must cover 90% of web length.



L-Brace Size for One-Ply Truss

Specified Continuous Rows of Lateral Bracing

Web Size	1	2
2x3 or 2x4	1x4	***
2x6	1x6	***
2x8	2x8	***

*** DIRECT SUBSTITUTION NOT APPLICABLE.

L-Brace Size for Two-Ply Truss

Specified Continuous Rows of Lateral Bracing

Web Size	1	2
2x3 or 2x4	2x4	***
2x6	2x6	***
2x8	2x8	***

*** DIRECT SUBSTITUTION NOT APPLICABLE.

L-Brace must be same species grade (or better) as web member.

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE
 Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component to be installed and loaded vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult QST-88 Quality Standard, DSB-88 Bracing Specification, and HIB-81 Handling, Installing and Bracing Recommendation available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719



OCTOBER 11, 2001

T-BRACE / I-BRACE DETAIL

ST - T-BRACE

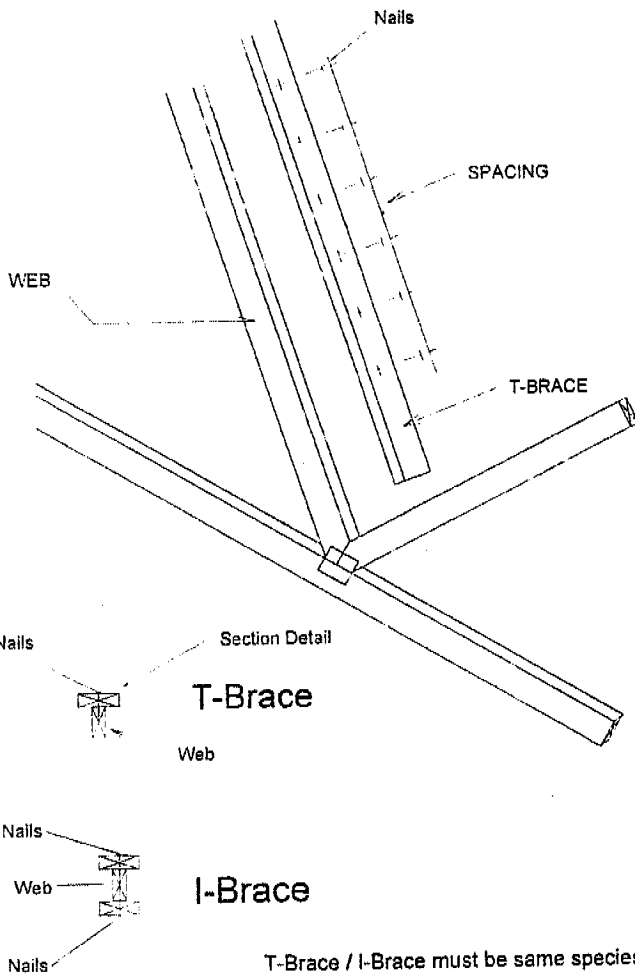
MITek Industries, Inc. Chesterfield, MO.



T-Brace size	Nailing Pattern	
	Nail Size	Nail Spacing
1x4 or 6	10d	8" o.c.
2x4, 6, or 8	16d	8" o.c.

Note: T-Bracing to be used when continuous lateral bracing is impractical. T-brace must cover 90% of web length.

Note: Nail along entire length of T-Brace (On Two-Ply's Nail to Both Plies)



T-Brace Size for One-Ply Truss

Web Size	Specified Continuous Rows of Lateral Bracing	
	1	2
2x3 or 2x4	1x4	1x4 I-Brace
2x6	1x6	2x6 I-Brace
2x8	2x8	2x8 I-Brace

*** DIRECT SUBSTITUTION NOT APPLICABLE.

T-Brace Size for Two-Ply Truss

Web Size	Specified Continuous Rows of Lateral Bracing	
	1	2
2x3 or 2x4	2x4	2x4 I-Brace
2x6	2x6	2x6 I-Brace
2x8	2x8	2x8 I-Brace

*** DIRECT SUBSTITUTION NOT APPLICABLE.

T-Brace / I-Brace must be same species grade (or better) as web member.

Warning - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE
 Design valid for use only with MITek connectors. This design is based only upon parameters shown, and is for an individual building component to be installed and spaced vertically. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult QST-88 Quality Standard, DSB 89 Bracing Specification, and HB-91 Handling, Installing and Bracing Recommendation available from Truss Plate Institute, 583 D'Oroff Drive, Madison, WI 53719



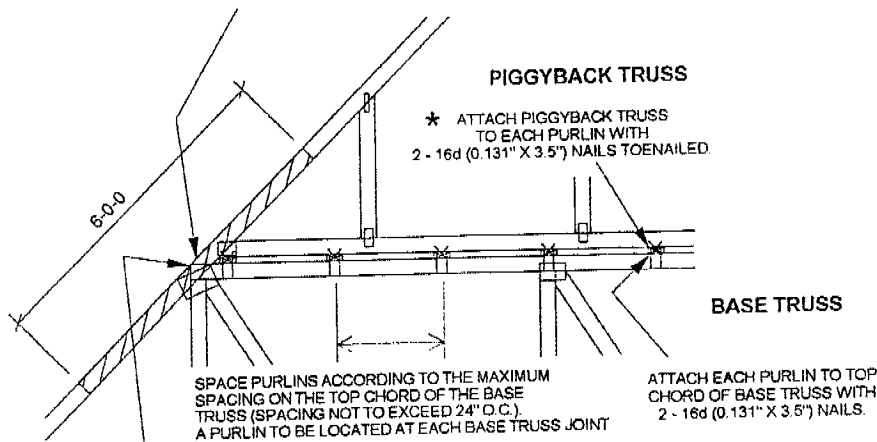
OCT 29, 2004

STANDARD PIGGYBACK TRUSS CONNECTION DETAIL

ST-PIGGY



* 2 x $\frac{1}{2}$ x 6'-0" SIZE TO MATCH TOP CHORD OF PIGGYBACK. ATTACHED TO ONE FACE OF TOP CHORD WITH 2 ROWS OF 10d (0.131" X 3") NAILS SPACED 6" O.C.



* FOR PIGGYBACK TRUSSES WITH SPANS < 12' SCAB MAY BE OMITTED PROVIDED THAT: ROOF SHEATHING TO BE CONTINUOUS OVER JOINT (SHEATHING TO OVERLAP MINIMUM 12" OVER JOINT)

* CAP CONNECTION IS MADE TO RESIST UPLIFT. SEE MAXIMUM CONNECTION CAPACITIES AND COMPARE WITH ENGINEERING DRAWING CONNECTION CAPACITIES FOR SCABS, PURLINS, AND SHEATHING MAY BE COMBINED WHEN DETERMINING OVERALL UPLIFT CAPACITY.

ALL VALUES SHOWN BELOW ARE BASED ON LOAD DURATION OF 1.33

MAXIMUM UPLIFT SCAB CAPACITY USING (10) 10d (0.131" X 3") NAILS:

SYP = 1409 LBS
 SPF = 1090 LBS
 DF = 1290 LBS
 HF = 1117 LBS
 SPF-S = 957 LBS

MAXIMUM UPLIFT PURLIN CAPACITY USING (2) 16d (0.131" X 3.5") NAILS:

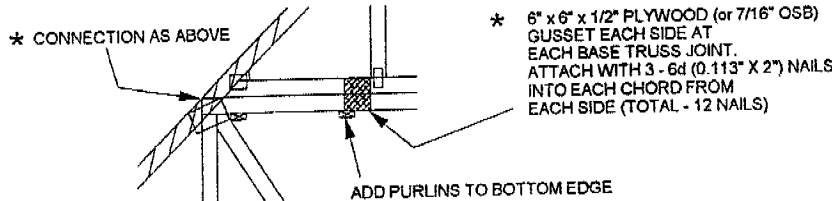
SYP = 155 LBS
 SPF = 79 LBS
 DF = 122 LBS
 HF = 83 LBS
 SPF-S = 54 LBS

MAXIMUM UPLIFT SHEATHING CAPACITY USING 1/2" SHEATHING AND (2) 8d (0.131" X 2.5") NAILS:

SYP = 109 LBS
 SPF = 55 LBS
 DF = 85 LBS
 HF = 58 LBS
 SPF-S = 37 LBS

IF NO GAP EXISTS BETWEEN CAP TRUSS AND BASE TRUSS:

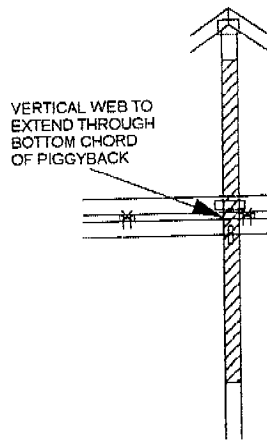
REPLACE THE NAILING OF CAP TRUSS TO PURLINS WITH GUSSETS AS SHOWN, AND APPLY PURLINS TO LOWER EDGE OF BASE TRUSS TOP CHORD AT SPECIFIED SPACING SHOWN ON BASE TRUSS DESIGN DRAWING.



MAXIMUM UPLIFT GUSSET CAPACITY USING 7/16" GUSSETS AND (6) 6d (0.113" X 2") NAILS:

SYP = 399 LBS
 SPF = 367 LBS
 DF = 391 LBS
 HF = 367 LBS
 SPF-S = 343 LBS

FOR LARGE CONCENTRATED LOADS APPLIED TO CAP TRUSS REQUIRING A VERTICAL WEB:



- 1) VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS MUST MATCH IN SIZE, GRADE, AND MUST LINE UP AS SHOWN IN DETAIL.
- 2) VERTICAL WEBS OF PIGGYBACK MUST RUN THROUGH BOTTOM CHORD SO THAT THERE IS FULL WOOD TO WOOD CONTACT BETWEEN WEB OF PIGGYBACK AND THE TOP CHORD OF THE BASE TRUSS.
- 3) CONCENTRATED LOAD MUST BE APPLIED TO BOTH THE PIGGYBACK AND THE BASE TRUSS.
- 4) ATTACH 2 x $\frac{1}{2}$ x 6'-0" SCAB TO EACH FACE OF TRUSS ASSEMBLY WITH 2 ROWS OF 10d (0.131" X 3") NAILS SPACED 6" O.C. FROM EACH FACE. (SIZE AND GRADE TO MATCH VERTICAL WEBS OF PIGGYBACK AND BASE TRUSS.) (MINIMUM 2X4)
- 5) THIS CONNECTION IS ONLY VALID FOR A MAXIMUM CONCENTRATED LOAD OF 4000 LBS (@1.15). REVIEW BY A QUALIFIED ENGINEER IS REQUIRED FOR LOADS GREATER THAN 4000 LBS.
- 6) FOR PIGGYBACK TRUSSES CARRYING GIRDER LOADS, NUMBER OF PLYS OF PIGGYBACK TRUSS TO MATCH BASE TRUSS.

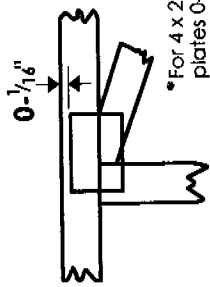
MAXIMUM UPLIFT SCAB CAPACITY USING (20) 10d (0.131" X 3") NAILS:

SYP = 2819 LBS
 SPF = 2181 LBS
 DF = 2580 LBS
 HF = 2234 LBS
 SPF-S = 1915 LBS

Symbols

PLATE LOCATION AND ORIENTATION

* Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths. Apply plates to both sides of truss and securely seat.



* For 4 x 2 orientation, locate plates 0-1/16" from outside edge of truss.

* This symbol indicates the required direction of slots in connector plates.



* Plate location details available in Mitek 20/20 software or upon request.

PLATE SIZE

4 X 4

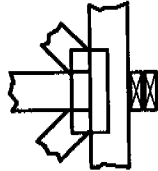
The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING



Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

BEARING



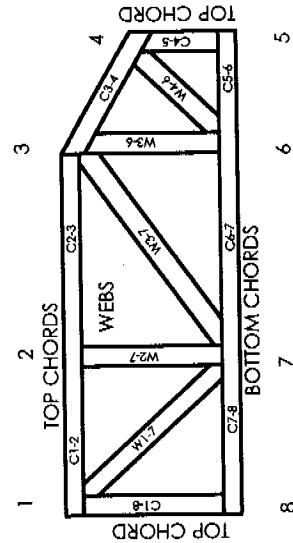
Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

Industry Standards:

ANSI/TPI1: National Design Specification for Metal Plate Connected Wood Truss Construction.
 DSB-89: Design Standard for Bracing.
 BCS11: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System

6-4-8 dimensions shown in ft-in-sixteenths



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

CONNECTOR PLATE CODE APPROVALS

BOCA	96-31, 95-43, 96-20-1, 96-67, 84-32
ICBO	4922, 5243, 5363, 3907
SBCCI	9667, 9730, 9604B, 9511, 9432A



Mitek Engineering Reference Sheet: MI1-7473

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCS11.
2. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
3. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
4. Cut members to bear tightly against each other.
5. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI1.
6. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI1.
7. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
8. Unless expressly noted, this design is not applicable for use with fire retardant or preservative treated lumber.
9. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
10. Plate type, size, orientation and location dimensions shown indicate minimum plating requirements.
11. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
12. Top chords must be sheathed or purlins provided at spacing shown on design.
13. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
14. Connections not shown are the responsibility of others.
15. Do not cut or alter truss member or plate without prior approval of a professional engineer.
16. Install and load vertically unless indicated otherwise.

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MiTek Industries, Inc.

7777 Greenback Lane
Suite 109
Citrus Heights, CA, 95610
Telephone 916/676-1900
Fax 916/676-1909

Re: **J5-277**
MEEKS E.G. / PRESIDIO

The truss drawing(s) referenced below have been prepared by MiTek Industries, Inc. under my direct supervision based on the parameters provided by Latham Truss Inc..

Pages or sheets covered by this seal: R17952412 thru R17952429

My license renewal date for the state of California is September 30, 2006.

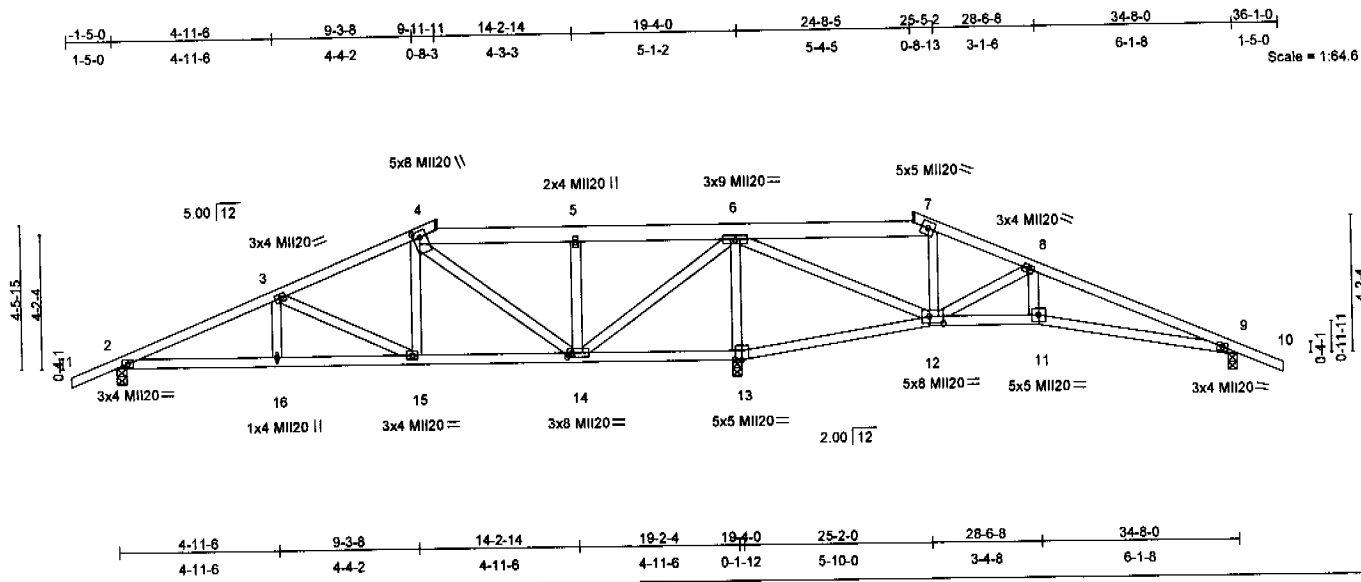


August 22, 2005

Yu, Ray

The seal on these drawings indicate acceptance of professional engineering responsibility solely for the truss components shown. The suitability and use of this component for any particular building is the responsibility of the building designer, per ANSI/TPI-2002 Chapter 2.

Job	Truss	Truss Type	Qty	Ply	MEEKS E.G. / PRESIDIO	R17952413
J5-277	A10	ROOF TRUSS	1	1	Job Reference (optional)	
LATHAM TRUSS, VALLEJO, CA, 94592						6,200 s Jul 13 2005 MiTek Industries, Inc. Fri Aug 19 15:19:27 2005 Page 1



LOADING (psf)	SPACING	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 16.0	Plates Increase 1.25	TC 0.22	Vert(LL)	-0.06	12-13	>999	360	220/195
TCDL 14.0	Lumber Increase 1.25	BC 0.29	Vert(TL)	-0.11	12-13	>999	240	
BCLL 0.0	Rep Stress Incr YES	WB 0.47	Horz(TL)	0.02	9	n/a	n/a	
BCLD 8.0	Code UBC97/ANSI95	(Simplified)						Weight: 172 lb

LUMBER
TOP CHORD 2 X 4 DF No.1&Btr G *Except*
4-7 2 X 6 DF No.2 G
BOT CHORD 2 X 4 DF No.1&Btr G
WEBS 2 X 4 DF Std G

BRACING
TOP CHORD Sheathed or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing, Except:
6-0-0 oc bracing: 13-14,12-13.

REACTIONS (lb/size) 2=655/0-3-8, 13=1686/0-3-8, 9=459/0-3-8
Max Horz 2=10(load case 4)
Max Uplift 2=-48(load case 4), 9=-50(load case 3)
Max Grav 2=689(load case 6), 13=1686(load case 1), 9=477(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/16, 2-3=-1032/22, 3-4=-641/44, 4-5=-221/68, 5-6=-221/68, 6-7=-85/52, 7-8=-97/44, 8-9=-614/9, 9-10=0/15
BOT CHORD 2-16=0/946, 15-16=0/946, 14-15=0/589, 13-14=-813/22, 12-13=-880/24, 11-12=0/548, 9-11=0/570
WEBS 3-16=0/165, 3-15=-391/19, 4-15=0/310, 4-14=-525/0, 5-14=-293/49, 6-14=0/1146, 6-13=-1455/37, 6-12=0/990, 7-12=-264/33
8-12=-514/12, 8-11=0/239

- NOTES**
- Unbalanced roof live loads have been considered for this design.
 - This truss has been designed for the wind loads generated by 80 mph winds at 25 ft above ground level, using 5.0 psf top chord dead load and 5.0 psf bottom chord dead load, 100 mi from hurricane oceanline, on an occupancy category I, condition I enclosed building, of dimensions 45 ft by 35 ft with exposure B ASCE 7-93 per UBC97/ANSI95. If end verticals or cantilevers exist, they are exposed to wind. If porches exist, they are not exposed to wind. The lumber DOL increase is 1.33, and the plate grip increase is 1.33.
 - Provide adequate drainage to prevent water ponding.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - A plate rating reduction of 20% has been applied for the green lumber members.
 - Bearing at joint(s) 9 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

LOAD CASE(S) Standard



August 22, 2005

<p>WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 BEFORE USE. Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BC511 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.</p>	<p>7777 Greenback Lane Suite 109 Citrus Heights, CA, 95610</p>
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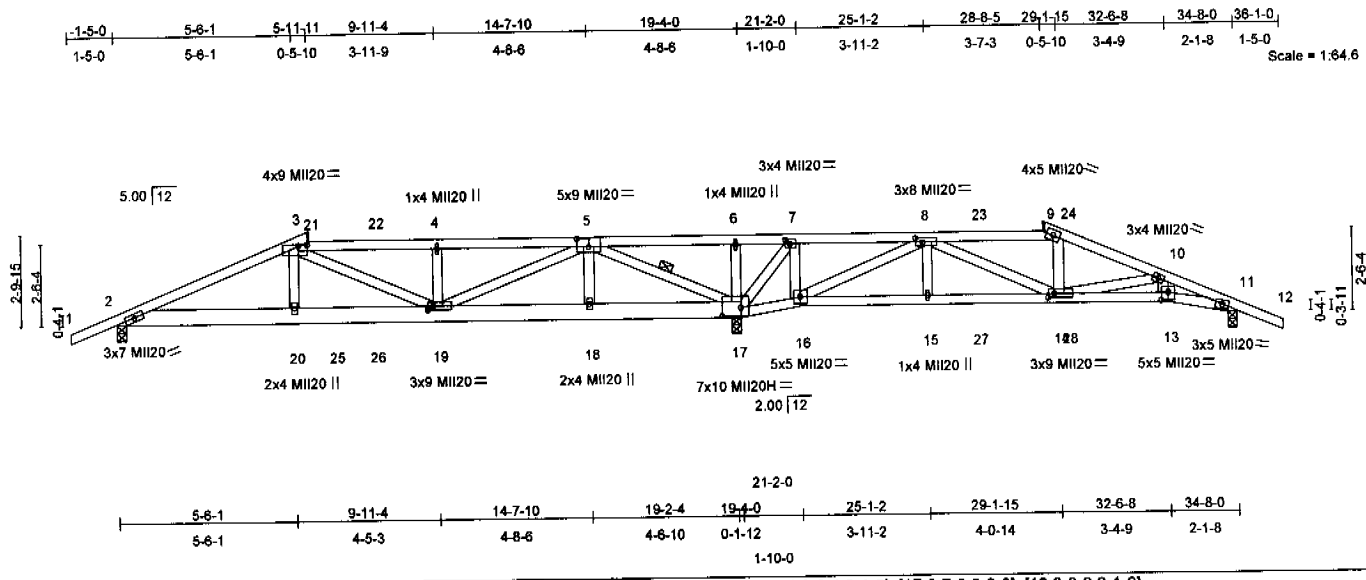


Plate Offsets (X, Y): [3:0-3-4,0-0-8], [5:0-4-8,0-3-0], [7:0-1-12,0-1-8], [8:0-2-8,0-1-8], [13:0-2-8,0-3-0], [14:0-2-4,0-1-8], [17:0-7-0,0-3-0], [19:0-2-0,0-1-8]					
LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc) l/defl L/d	PLATES	GRIP
TCLL 16.0	Plates Increase 1.25	TC 0.68	Vert(LL) -0.07 19 >999 360	MI20	220/195
TCDL 14.0	Lumber Increase 1.25	BC 0.42	Vert(TL) -0.16 19-20 >999 240	MI20H	165/146
BCLL 0.0	Rep Stress Incr NO	WB 0.77	Horz(TL) 0.05 11 n/a n/a	Weight: 171 lb	
BCDL 8.0	Code UBC97/ANSI95	(Matrix)			

LUMBER
TOP CHORD 2 X 4 DF No.1&Btr G
BOT CHORD 2 X 4 DF No.1&Btr G *Except*
2-17 2 X 6 DF No.2 G
WEBS 2 X 4 DF Std G

BRACING
TOP CHORD Sheathed or 4-1-13 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing. Except:
5-4-1 oc bracing: 16-17.
WEBS 1 Row at midpt 5-17

REACTIONS (lb/size) 2=1176/0-3-8, 11=814/0-3-8, 17=3362/0-3-9 (input: 0-3-8)
Max Horz 2=8 (load case 4)
Max Uplift 2=-68 (load case 4), 11=-52 (load case 3), 17=-149 (load case 5)
Max Grav 2=1184 (load case 6), 11=819 (load case 7), 17=3362 (load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/36, 2-3=-2446/123, 3-21=-2225/144, 21-22=-2225/144, 4-22=-2225/144, 4-5=-2225/144, 5-6=-58/2327, 6-7=-58/2327,
7-8=-20/1271, 8-23=-1457/106, 23-24=-1457/106, 9-24=-1457/106, 9-10=-1547/95, 10-11=-1939/0, 11-12=0/30
BOT CHORD 2-20=-69/2230, 20-25=-76/2220, 25-26=-76/2220, 19-26=-76/2220, 18-19=-12/685, 17-18=-12/685, 16-17=-1355/85,
15-16=-17/767, 15-27=-17/767, 27-28=-17/767, 14-28=-17/767, 13-14=0/1653, 11-13=0/1728
WEBS 3-20=0/354, 3-19=-18/84, 4-19=-800/119, 5-19=-81/1712, 5-18=0/294, 5-17=-3269/141, 6-17=-454/82, 7-17=-1523/62,
7-16=0/855, 8-16=-2241/109, 8-15=0/267, 8-14=-29/774, 9-14=-137/98, 10-14=-297/0, 10-13=0/356

- NOTES**
- Unbalanced roof live loads have been considered for this design.
 - This truss has been designed for the wind loads generated by 80 mph winds at 25 ft above ground level, using 5.0 psf top chord dead load and 5.0 psf bottom chord dead load, 100 mi from hurricane oceanline, on an occupancy category I, condition I enclosed building, of dimensions 45 ft by 35 ft with exposure B ASCE 7-93 per UBC97/ANSI95 if end verticals or cantilevers exist, they are exposed to wind. If porches exist, they are not exposed to wind. The lumber DOL increase is 1.33, and the plate grip increase is 1.33
 - Provide adequate drainage to prevent water ponding.
 - All plates are MT20 plates unless otherwise indicated.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - A plate rating reduction of 20% has been applied for the green lumber members.
 - WARNING: Required bearing size at joint(s) 17 greater than input bearing size.
 - Bearing at joint(s) 11 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
 - Girder carries hip end with 6-0-0 end setback and tie-in span of 2-0-0 from subgirder.
 - Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 319 lb down and 72 lb up at 28-8-0, and 319 lb down and 72 lb up at 6-0-0 on top chord. The design/selection of such connection device(s) is the responsibility of others.
 - In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
August 22, 2005



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Job	Truss	Truss Type	Qty	Ply	MEEKS E.G. / PRESIDIO	R17952415
J5-277	A12	ROOF TRUSS	1	1	Job Reference (optional)	

LATHAM TRUSS, VALLEJO, CA, 94592

6.200 s Jul 13 2005 MiTek Industries, Inc. Fri Aug 19 15:19:28 2005 Page 2

LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-3=-60, 3-21=-60, 22-23=-125, 9-24=-60, 9-12=-60, 2-17=-29(F=-13), 16-17=-29(F=-13), 13-16=-29(F=-13), 11-13=-29(F=-13)

Concentrated Loads (lb)

Vert: 21=-319 24=-319

Trapezoidal Loads (plf)

Vert: 21=-110-to-22=-125, 23=-125-to-24=-110

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.
 Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information** available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

7777 Greenback Lane
 Suite 109
 Citrus Heights, CA, 95610



J66	Truss	Truss Type	City	Ply	MEEKS E.G. / PRESIDIO	R17952418
J5-277	A3	ROOF TRUSS	7	1	Job Reference (optional)	
LATHAM TRUSS, VALLEJO, CA, 94592					6,200 s Jul 13 2005 Mitek Industries, Inc. Fri Aug 19 15:19:28 2005 Page 1	

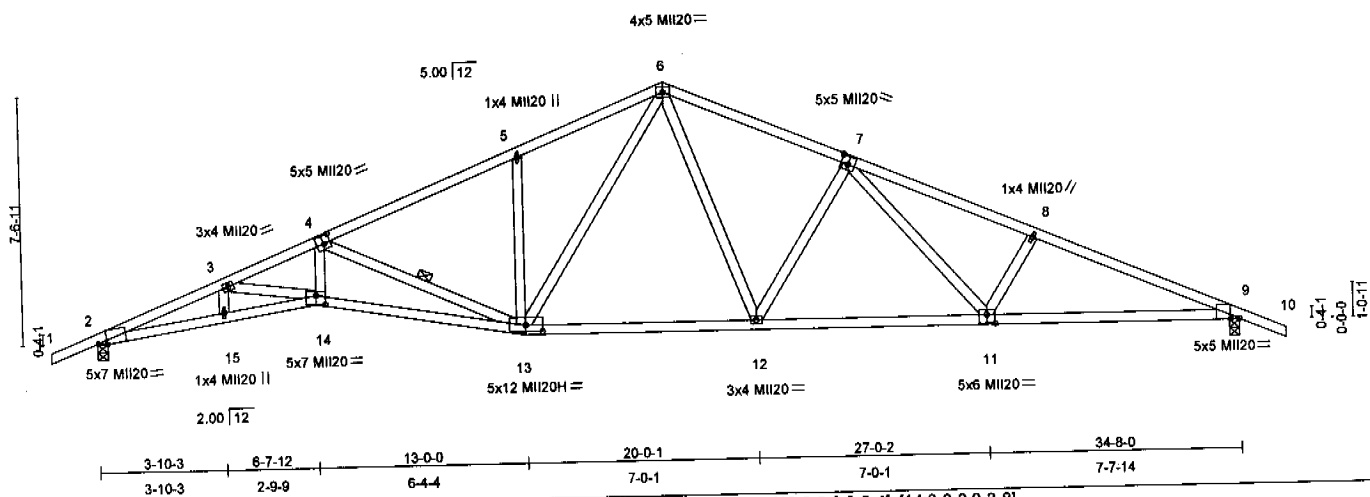
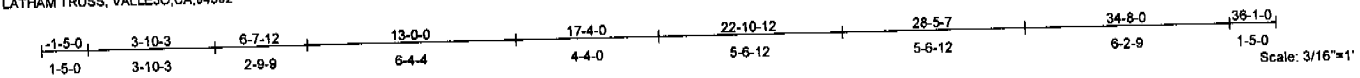


Plate Offsets (X, Y): [2:0-3-8,0-0-11], [4:0-2-8,0-3-0], [7:0-2-8,0-3-0], [9:0-3-2,0-0-2], [11:0-3-0,0-3-4], [13:0-6-0,0-2-4], [14:0-3-8,0-3-8]									
LOADING (psf)	SPACING	2-0-0	CSI	DEFL	In (loc)	l/def	L/d	PLATES	GRIP
TCLL 16.0	Plates Increase	1.25	TC 0.35	Vert(LL)	-0.19 13-14	>999	360	MI120	220/195
TCDL 14.0	Lumber Increase	1.25	BC 0.67	Vert(TL)	-0.47 13-14	>869	240	MI120H	165/146
BCLL 0.0	Rep Stress Incr	YES	WB 0.63	Horz(TL)	0.20 9	n/a	n/a		
BCDL 8.0	Code	UBC97/ANSI95	(Matrix)						Weight: 166 lb

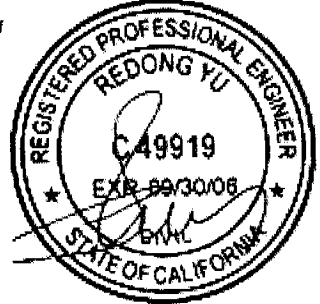
LUMBER	BRACING
TOP CHORD 2 X 4 DF No.1&Btr G	TOP CHORD Sheathed or 2-11-15 oc purlins.
BOT CHORD 2 X 4 DF No.1&Btr G	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 2 X 4 DF Std G	WEBS 1 Row at midpt 4-13

REACTIONS (lb/size) 2=1400/0-3-8, 9=1400/0-3-8
 Max Horz 2=-17(load case 3)
 Max Uplift 2=-41(load case 5), 9=-41(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/30, 2-3=-4317/0, 3-4=-4151/0, 4-5=-2211/37, 5-6=-2198/82, 6-7=-1979/83, 7-8=-2629/30, 8-9=-2811/17, 9-10=0/31
 BOT CHORD 2-15=0/3936, 14-15=0/3934, 13-14=0/3826, 12-13=0/1553, 11-12=0/2049, 9-11=0/2519
 WEBS 4-14=0/1308, 4-13=-1986/1, 5-13=-341/73, 6-13=-20/826, 6-12=0/624, 7-12=-559/64, 7-11=0/516, 8-11=-303/68, 3-15=0/111, 3-14=-91/0

- NOTES**
- Unbalanced roof live loads have been considered for this design.
 - This truss has been designed for the wind loads generated by 80 mph winds at 25 ft above ground level, using 5.0 psf top chord dead load and 5.0 psf bottom chord dead load, 100 mi from hurricane oceanline, on an occupancy category I, condition I enclosed building, of dimensions 45 ft by 35 ft with exposure B ASCE 7-93 per UBC97/ANSI95. If end verticals or cantilevers exist, they are exposed to wind. If porches exist, they are not exposed to wind. The lumber DOL increase is 1.33, and the plate grip increase is 1.33.
 - All plates are MT20 plates unless otherwise indicated.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - A plate rating reduction of 20% has been applied for the green lumber members.
 - Bearing at joint(s) 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

LOAD CASE(S) Standard



August 22, 2005

<p>WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 BEFORE USE.</p> <p>Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI Quality Criteria, D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.</p>	<p>7777 Greenback Lane Suite 109 Citrus Heights, CA, 95610</p>
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Job	Truss	Truss Type	City	Ply	MEEKS E.G./PRESIDIO	R17952417
J5-277	A4	ROOF TRUSS	5	1	Job Reference (optional)	
LATHAM TRUSS, VALLEJO, CA, 94582						6.200 s Jul 13 2005 MiTek Industries, Inc. Fri Aug 19 15:19:28 2005 Page 1

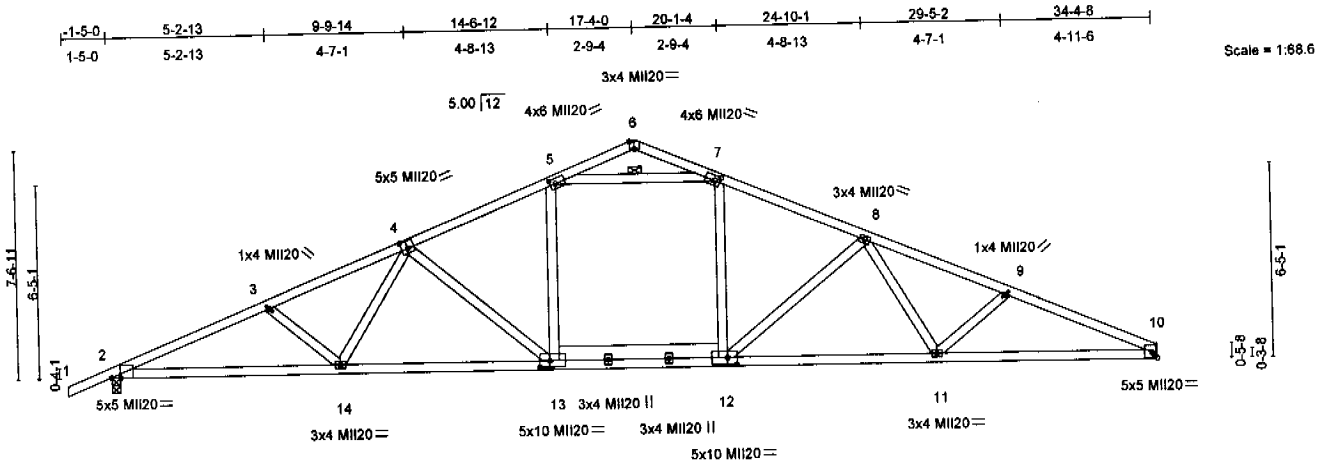


Plate Offsets (X, Y): [2:0-3-6,0-0-2], [4:0-2-8,0-3-0], [5:0-2-0,0-2-0], [6:0-2-0,Edge], [7:0-2-0,0-2-0], [12:0-3-12,0-2-12], [13:0-4-0,0-2-8]									
LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 16.0	Plates Increase	1.25	TC 0.37	Vert(LL)	-0.23	13-14	>999	MII20	220/195
TCDL 14.0	Lumber Increase	1.25	BC 0.54	Vert(TL)	-0.41	13-14	>991		
BCLL 0.0	Rep Stress Incr	YES	WB 0.36	Horz(TL)	0.10	10	n/a		
BCDL 8.0	Code	UBC97/ANSI95	(Matrix)						Weight: 170 lb

LUMBER
 TOP CHORD 2 X 4 DF No.1&Btr G
 BOT CHORD 2 X 4 DF No.1&Btr G *Except*
 12-13 2 X 6 DF No.2 G
 WEBS 2 X 4 DF Std G

BRACING
 TOP CHORD Sheathed or 3-8-13 oc purlins. Except:
 1 Row at midpt 5-7
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=1468/0-3-8, 10=1371/Mechanical
 Max Horz 2=35(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/31, 2-3=-3004/0, 3-4=-2784/0, 4-5=-2290/0, 5-6=-160/11, 6-7=-162/10, 7-8=-2286/0, 8-9=-2735/0, 9-10=-2921/0,
 5-7=-1930/0
 BOT CHORD 2-14=0/2701, 13-14=0/2416, 12-13=0/2042, 11-12=0/2384, 10-11=0/2620
 WEBS 5-13=0/565, 7-12=0/552, 4-13=-526/61, 4-14=0/320, 3-14=-238/60, 8-12=-495/70, 8-11=0/283, 9-11=-192/69

- NOTES**
- 1) Unbalanced roof live loads have been considered for this design.
 - 2) This truss has been designed for the wind loads generated by 80 mph winds at 25 ft above ground level, using 5.0 psf top chord dead load and 5.0 psf bottom chord dead load, 100 mi from hurricane oceanline, on an occupancy category I, condition I enclosed building, of dimensions 45 ft by 34 ft with exposure B ASCE 7-83 per UBC97/ANSI95. If end verticals or cantilevers exist, they are exposed to wind. If porches exist, they are not exposed to wind. The lumber DOL increase is 1.33, and the plate grip increase is 1.33.
 - 3) 150.0lb AC unit load placed on the bottom chord, 17-4-0 from left end, supported at two points, 5-0-0 apart.
 - 4) Provide adequate drainage to prevent water ponding.
 - 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 6) A plate rating reduction of 20% has been applied for the green lumber members.
 - 7) Refer to girder(s) for truss to truss connections.
 - 8) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.
 - 9) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 75 lb down at 14-10-0, and 75 lb down at 19-10-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard



August 22, 2005

<p>WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 BEFORE USE. Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.</p>	<p>7777 Greenback Lane Suite 109 Citrus Heights, CA, 95610</p>
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JOB	Truss	Truss Type	Qty	Ply	MEEKS E.G. / PRESIDIO	R17052418
J5-277	A5	ROOF TRUSS	1	1	Job Reference (optional)	
LATHAM TRUSS, VALLEJO, CA, 94592					6.200 5 Jul 13 2005 MITEK Industries, Inc. Fri Aug 19 15:19:29 2005 Page 1	

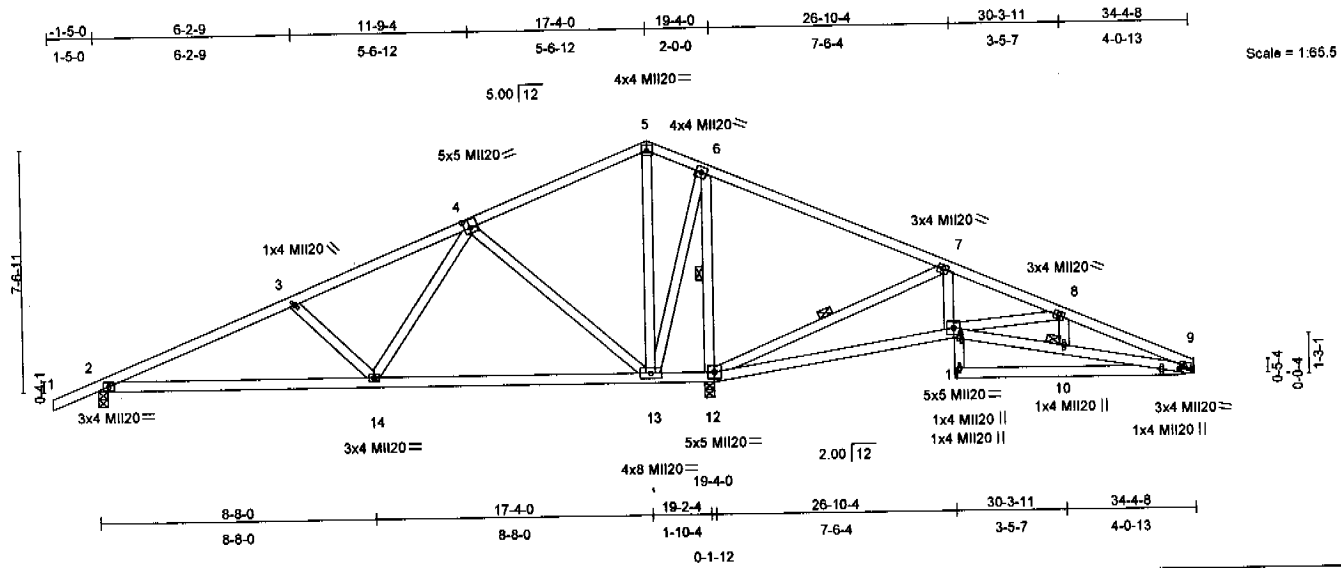


Plate Offsets (X, Y): [4:0-2-4,0-3-0], [9:0-1-12,0-1-8], [11:0-2-0,0-0-8]						
LOADING (psf)	SPACING	CSI	DEFL	in (loc)	L/def	L/d
TCLL 16.0	Plates Increase 1.25	TC 0.48	Vert(LL) -0.16	11-12	>999	360
TCDL 14.0	Lumber Increase 1.25	BC 0.55	Vert(TL) -0.29	11-12	>629	240
BCLL 0.0	Rep Stress Incr YES	WB 0.61	Horz(TL) 0.03	9	n/a	n/a
BCDL 8.0	Code UBC97/ANSI95	(Simplified)				
						PLATES MI120
						GRIP 220/195
						Weight: 178 lb

LUMBER
TOP CHORD 2 X 4 DF No.1&Btr G
BOT CHORD 2 X 4 DF No.1&Btr G *Except*
9-15 2 X 4 DF Std G
WEBS 2 X 4 DF Std G *Except*
11-15 2 X 4 DF No.1&Btr G

BRACING
TOP CHORD Sheathed or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing. Except:
10-0-0 oc bracing: 2-14.
WEBS 1 Row at midpt 6-12, 7-12
JOINTS 1 Brace at Jt(s): 10

REACTIONS (lb/size) 9=310/Mechanical, 2=807/0-3-8, 12=1772/0-3-8
Max Horz2=36(load case 4)
Max Uplift9=-3(load case 3), 2=-42(load case 4), 12=-1(load case 5)
Max Grav9=338(load case 7), 2=871(load case 6), 12=1772(load case 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/16, 2-3=-832/39, 3-4=-572/33, 4-5=0/371, 5-6=0/371, 6-7=0/668, 7-8=-191/60, 8-9=-700/0
BOT CHORD 2-14=-28/64, 13-14=-77/295, 12-13=-617/63, 11-12=-56/178, 10-11=0/644, 9-10=0/645
WEBS 3-14=-337/79, 4-14=0/493, 4-13=-633/56, 5-13=-499/0, 6-13=0/1041, 6-12=-1401/33, 7-12=-797/18, 7-11=0/346,
8-11=-472/25, 8-10=0/143

NOTES

- Unbalanced roof live loads have been considered for this design.
- This truss has been designed for the wind loads generated by 80 mph winds at 25 ft above ground level, using 5.0 psf top chord dead load and 5.0 psf bottom chord dead load, 100 mi from hurricane oceanline, on an occupancy category I, condition I enclosed building, of dimensions 45 ft by 34 ft with exposure B ASCE 7-93 per UBC97/ANSI95. If end verticals or cantilevers exist, they are exposed to wind. If porches exist, they are not exposed to wind. The lumber DOL increase is 1.33, and the plate grip increase is 1.33.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- A plate rating reduction of 20% has been applied for the green lumber members.
- Refer to girder(s) for truss to truss connections.

LOAD CASE(S) Standard



August 22, 2005

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JOB	Truss	Truss Type	Qty	Ply	MEEKS E.G./PRESIDIO	R17852419
J5-277	A8	ROOF TRUSS	2	1	Job Reference (optional)	
LATHAM TRUSS, VALLEJO, CA, 94592						6.200 s Jul 13 2005 Mitek Industries, Inc. Fri Aug 19 15:19:29 2005 Page 1

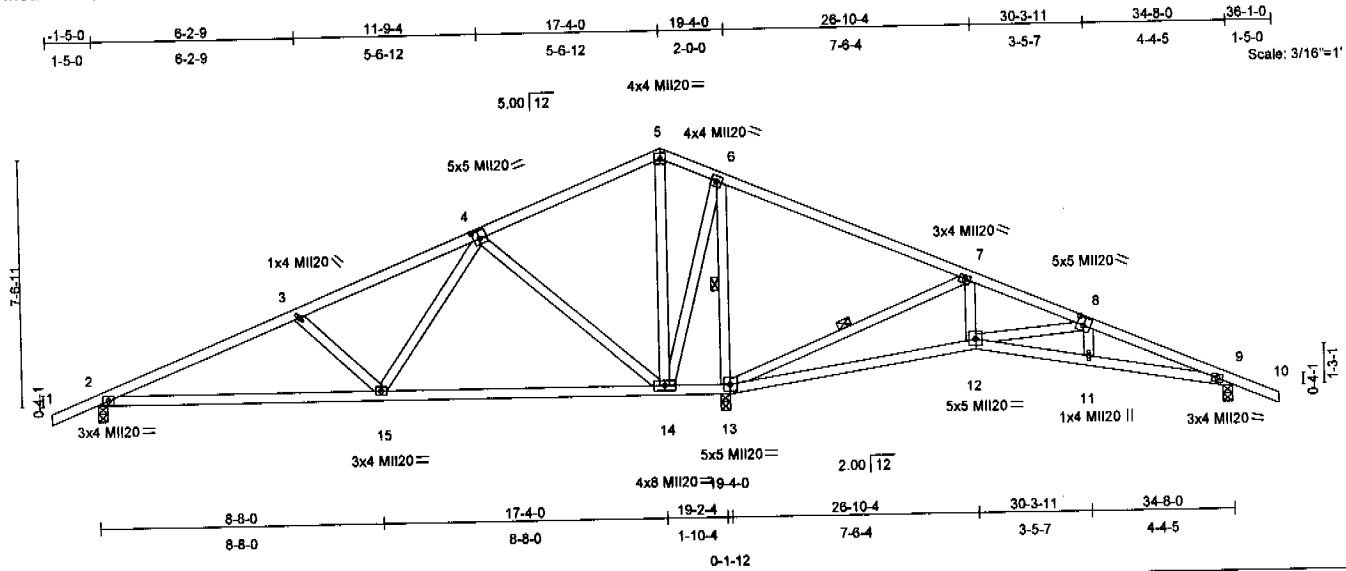


Plate Offsets (X, Y): [4:0-2-4,0-3-0], [8:0-2-8,0-3-0]

LOADING (psf)	SPACING 2-0-0	CSI	DEFL	PLATES	GRIP
TCLL 16.0	Plates Increase 1.25	TC 0.49	Vert(LL) -0.16 12-13 >999 360	MI120	220/195
TCDL 14.0	Lumber Increase 1.25	BC 0.55	Vert(TL) -0.29 12-13 >636 240		
BCLL 0.0	Rep Stress Incr YES	WB 0.61	Horz(TL) 0.03 9 n/a n/a		
BCDL 8.0	Code UBC97/ANSI95	(Simplified)			Weight: 170 lb

LUMBER
TOP CHORD 2 X 4 DF No.1&Btr G
BOT CHORD 2 X 4 DF No.1&Btr G
WEBS 2 X 4 DF Std G

BRACING
TOP CHORD Sheathed or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 6-13, 7-13

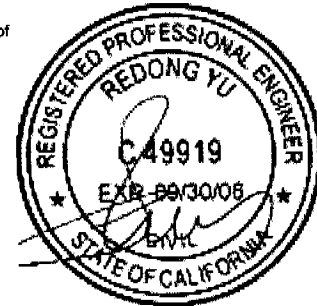
REACTIONS (lb/size) 2=601/0-3-8, 13=1791/0-3-8, 9=408/0-3-8
Max Horz2=17(load case 4)
Max Uplift2=-46(load case 4), 9=-47(load case 3)
Max Grav2=668(load case 6), 13=1791(load case 1), 9=434(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/16, 2-3=-825/49, 3-4=-564/43, 4-5=0/389, 5-6=0/389, 6-7=0/688, 7-8=-173/66, 8-9=-701/5, 9-10=0/15
BOT CHORD 2-15=-17/757, 14-15=-94/289, 13-14=-635/71, 12-13=-62/162, 11-12=0/642, 9-11=0/649
WEBS 3-15=-337/78, 4-15=0/493, 4-14=-634/55, 5-14=-512/0, 6-14=0/1054, 6-13=-1415/24, 7-13=-803/13, 7-12=0/351,
8-12=-486/25, 8-11=0/149

NOTES

- Unbalanced roof live loads have been considered for this design.
- This truss has been designed for the wind loads generated by 80 mph winds at 25 ft above ground level, using 5.0 psf top chord dead load and 5.0 psf bottom chord dead load, 100 ml from hurricane oceanline, on an occupancy category I, condition I enclosed building, of dimensions 45 ft by 35 ft with exposure B ASCE 7-93 per UBC97/ANSI95. If end verticals or cantilevers exist, they are exposed to wind. If porches exist, they are not exposed to wind. The lumber DOL increase is 1.33, and the plate grip increase is 1.33.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- A plate rating reduction of 20% has been applied for the green lumber members.
- Bearing at joint(s) 9 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

LOAD CASE(S) Standard



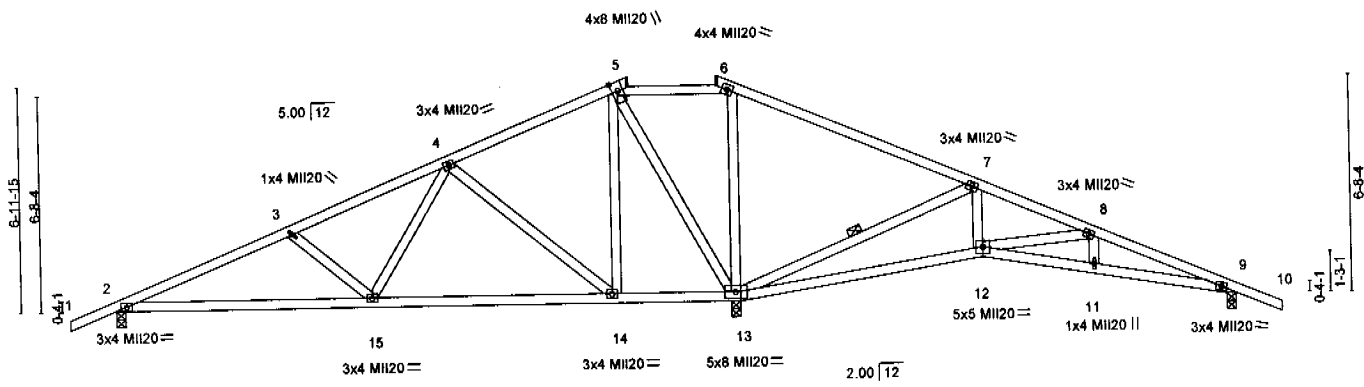
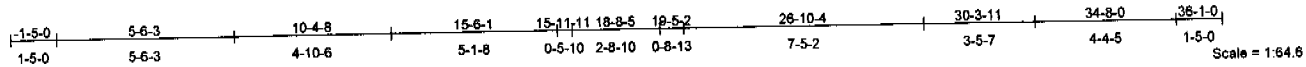
August 22, 2005

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 BEFORE USE.
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Citrus Heights, CA, 95610



JOB	Truss	Truss Type	Qty	Ply	MEEKS E.G./PRESIDIO	R17952420
J5-277	A7	ROOF TRUSS	1	1	Job Reference (optional)	
LATHAM TRUSS, VALLEJO, CA, 94592						6.200 s Jul 13 2005 MITEK Industries, Inc. Fri Aug 19 15:19:29 2005 Page 1



LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/def	L/d	PLATES	GRIP
TCLL 16.0	Plates Increase	1.25	TC 0.53	Vert(LL)	-0.14	12-13	>999	360	220/195
TCDL 14.0	Lumber Increase	1.25	BC 0.45	Vert(TL)	-0.26	12-13	>693	240	
BCLL 0.0	Rep Stress Incr	YES	WB 0.89	Horz(TL)	0.02	9	n/a	n/a	
BCDL 8.0	Code	UBC97/ANSI95	(Simplified)						Weight: 168 lb

LUMBER
TOP CHORD 2 X 4 DF No.1&Btr G
BOT CHORD 2 X 4 DF No.1&Btr G
WEBS 2 X 4 DF Std G

BRACING
TOP CHORD Sheathed or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 7-13

REACTIONS (lb/size) 2=570/0-3-8, 13=1879/0-3-8, 9=351/0-3-8
Max Horz 2=16(load case 4)
Max Uplift 2=-48(load case 4), 9=-50(load case 3)
Max Grav 2=646(load case 6), 13=1879(load case 1), 9=388(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/16, 2-3=-835/54, 3-4=-588/44, 4-5=0/336, 5-6=0/754, 6-7=0/845, 7-8=0/244, 8-9=-527/15, 9-10=0/15
BOT CHORD 2-15=-23/765, 14-15=-60/361, 13-14=-320/74, 12-13=-228/57, 11-12=0/487, 9-11=0/488
WEBS 3-15=-304/71, 4-15=0/422, 4-14=-585/46, 5-14=0/496, 5-13=-960/0, 6-13=-644/37, 7-13=-753/16, 7-12=0/295, 8-12=-503/24, 8-11=0/145

NOTES

- Unbalanced roof live loads have been considered for this design.
- This truss has been designed for the wind loads generated by 80 mph winds at 25 ft above ground level, using 5.0 psf top chord dead load and 5.0 psf bottom chord dead load, 100 mi from hurricane oceanline, on an occupancy category I, condition I enclosed building, of dimensions 45 ft by 35 ft with exposure B ASCE 7-93 per UBC97/ANSI95. If end verticals or cantilevers exist, they are exposed to wind. If porches exist, they are not exposed to wind. The lumber DOL increase is 1.33, and the plate grip increase is 1.33
- Provide adequate drainage to prevent water ponding.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- A plate rating reduction of 20% has been applied for the green lumber members.
- Bearing at joint(s) 9 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

LOAD CASE(S) Standard



August 22, 2005

<p>WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 BEFORE USE. Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component's responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and SCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.</p>	<p>7777 Greenback Lane Suite 109 Citrus Heights, CA, 95610</p>
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JOB	Truss	Truss Type	Qty	Ply	WEEKS E.G./PRESIDIO	R17952421
J5-277	A8	ROOF TRUSS	1	1		
LATHAM TRUSS, VALLEJO, CA, 94592					Job Reference (optional)	
					6.200 s Jul 13 2005 MITek Industries, Inc. Fri Aug 19 15:19:30 2005 Page 1	

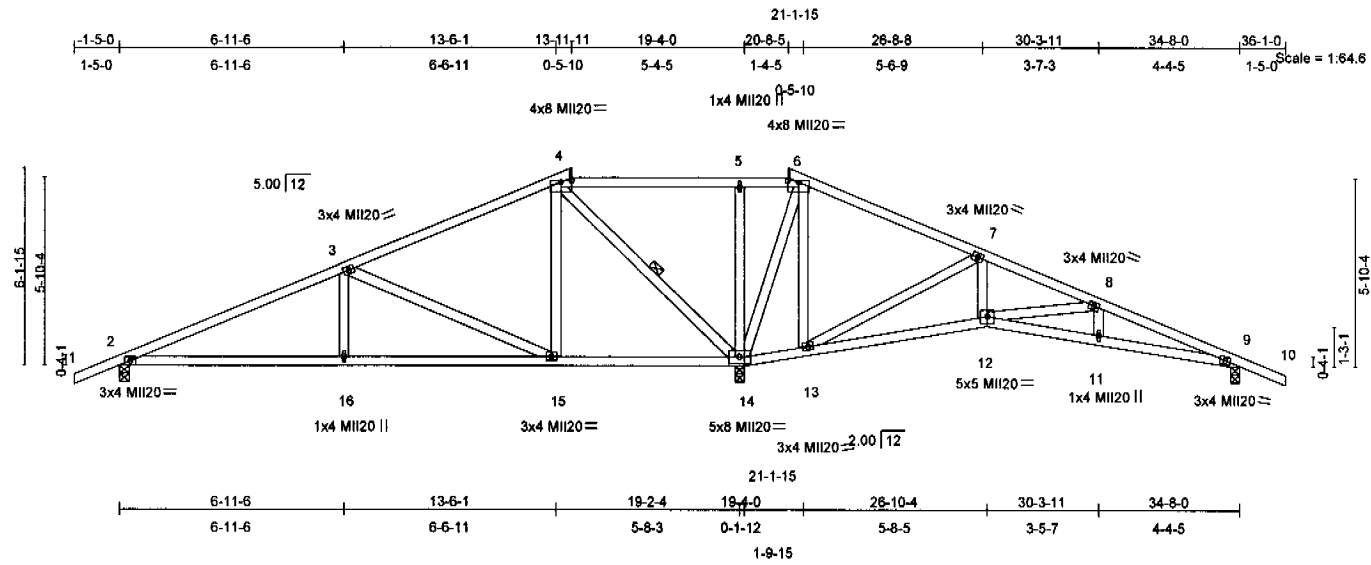


Plate Offsets (X, Y): [4:0-4-0,0-0-8], [6:0-4-0,0-0-8]					
LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 16.0	2-0-0 Plates Increase 1.25	TC 0.40	in (loc) l/defl L/d	MI120	220/195
TCDL 14.0	Lumber Increase 1.25	BC 0.36	Vert(LL) -0.05 15-16 >999 360		
BCLL 0.0	Rep Stress Incr YES	WB 0.57	Vert(TL) -0.11 15-16 >999 240		
BCDL 8.0	Code UBC97/ANSI95	(Simplified)	Horz(TL) 0.02 9 n/a n/a		
					Weight: 171 lb

LUMBER
TOP CHORD 2 X 4 DF No.1&Btr G
BOT CHORD 2 X 4 DF No.1&Btr G
WEBS 2 X 4 DF Std G

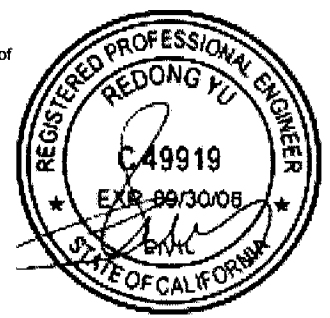
BRACING
TOP CHORD Sheathed or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 4-14

REACTIONS (lb/size) 2=586/0-3-8, 14=1843/0-3-8, 9=371/0-3-8
Max Horz2=14(load case 4)
Max Uplift2=-49(load case 4), 9=-50(load case 3)
Max Grav2=656(load case 6), 14=1843(load case 1), 9=401(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/16, 2-3=-768/24, 3-4=-137/154, 4-5=0/813, 5-6=0/819, 6-7=0/639, 7-8=-43/183, 8-9=-581/16, 9-10=0/15
BOT CHORD 2-16=0/706, 15-16=0/706, 14-15=-150/118, 13-14=-811/60, 12-13=-171/56, 11-12=0/537, 9-11=0/539
WEBS 3-16=0/240, 3-15=-641/30, 4-15=0/442, 4-14=-1065/0, 5-14=-221/37, 6-14=-747/0, 6-13=0/299, 7-13=-636/10, 7-12=0/280, 8-11=0/145, 8-12=-502/24

- NOTES**
- 1) Unbalanced roof live loads have been considered for this design.
 - 2) This truss has been designed for the wind loads generated by 80 mph winds at 25 ft above ground level, using 5.0 psf top chord dead load and 5.0 psf bottom chord dead load, 100 mi from hurricane oceanline, on an occupancy category I, condition I enclosed building, of dimensions 45 ft by 35 ft with exposure B ASCE 7-93 per UBC97/ANSI95 if end verticals or cantilevers exist, they are exposed to wind. If porches exist, they are not exposed to wind. The lumber DOL increase is 1.33, and the plate grip increase is 1.33
 - 3) Provide adequate drainage to prevent water ponding.
 - 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 5) A plate rating reduction of 20% has been applied for the green lumber members.
 - 6) Bearing at joint(s) 9 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

LOAD CASE(S) Standard



August 22, 2005

<p>WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE. Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, DSB-89 and BCS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.</p>	<p>7777 Greenback Lane Suite 109 Citrus Heights, CA, 95610</p>
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Job	Truss	Truss Type	Qty	Ply	MEEKS E.G. / PRESIDIO	R17952422
JS-277	A9	ROOF TRUSS	1	1	Job Reference (optional)	
LATHAM TRUSS, VALLEJO, CA, 94592						6:200 s Jul 13 2005 MITek Industries, Inc. Fri Aug 19 15:19:30 2005 Page 1

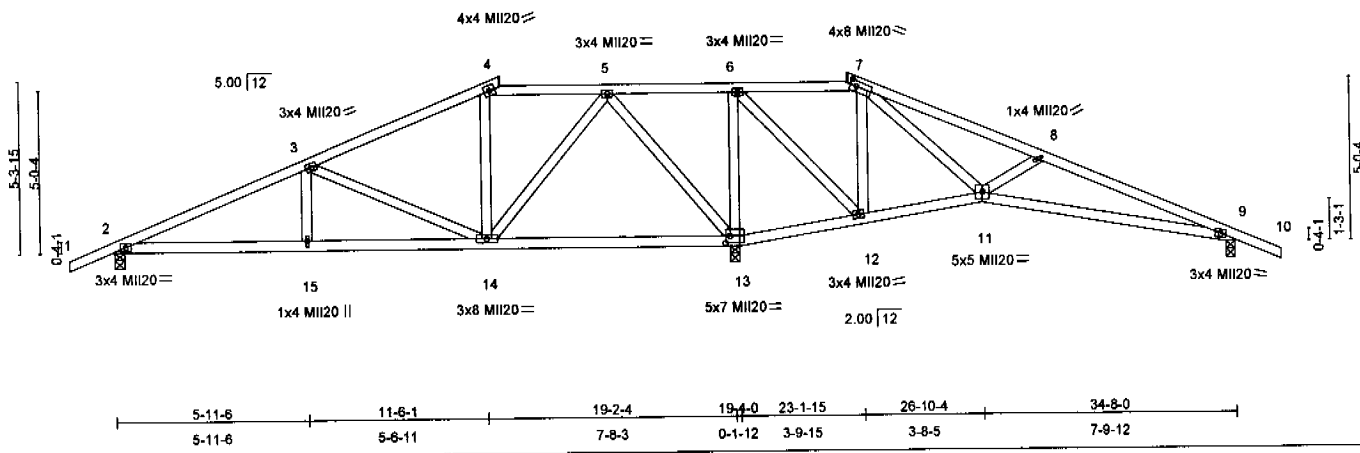
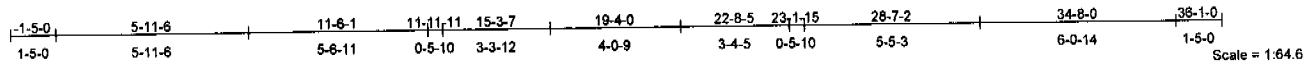


Plate Offsets (X, Y): [7-0-2-4, 0-2-0], [13-0-1-12, 0-2-8]									
LOADING (psf)	SPACING	CSI	DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL 16.0	Plates Increase 1.25	TC 0.28	Vert(LL)	-0.11	9-11	>999	360	MI20	220/195
TCDL 14.0	Lumber Increase 1.25	BC 0.42	Vert(TL)	-0.21	9-11	>888	240		
BCLL 0.0	Rep Stress Incr YES	WB 0.64	Horz(TL)	0.02	9	n/a	n/a		
BCDL 8.0	Code UBC97/ANSI95	(Simplified)							Weight: 163 lb

LUMBER
 TOP CHORD 2 X 4 DF No. 1&Btr G
 BOT CHORD 2 X 4 DF No. 1&Btr G
 WEBS 2 X 4 DF Std G

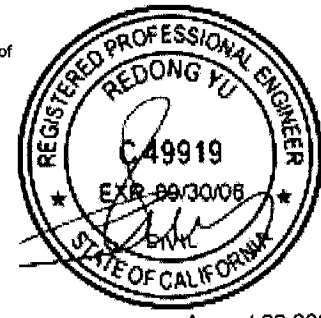
BRACING
 TOP CHORD Sheathed or 6-0-0 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 2=623/0-3-8, 13=1758/0-3-8, 9=419/0-3-8
 Max Horz 2=12(load case 4)
 Max Uplift 2=-46(load case 4), 9=-47(load case 3)
 Max Grav 2=674(load case 6), 13=1758(load case 1), 9=444(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/16, 2-3=-805/17, 3-4=-382/43, 4-5=-360/64, 5-6=0/807, 6-7=0/290, 7-8=-127/83, 8-9=-466/41, 9-10=0/15
 BOT CHORD 2-15=0/831, 14-15=0/831, 13-14=-204/39, 12-13=-843/50, 11-12=-294/63, 9-11=0/433
 WEBS 3-15=0/204, 3-14=-528/25, 4-14=-190/33, 5-14=0/659, 5-13=-975/19, 6-13=-786/28, 6-12=0/776, 7-12=-587/22, 7-11=0/472, 8-11=-373/86

- NOTES**
- 1) Unbalanced roof live loads have been considered for this design.
 - 2) This truss has been designed for the wind loads generated by 80 mph winds at 25 ft above ground level, using 5.0 psf top chord dead load and 5.0 psf bottom chord dead load, 100 mi from hurricane oceanline, on an occupancy category I, condition I enclosed building, of dimensions 45 ft by 35 ft with exposure B ASCE 7-93 per UBC97/ANSI95. If end verticals or cantilevers exist, they are exposed to wind. If porches exist, they are not exposed to wind. The lumber DOL increase is 1.33, and the plate grip increase is 1.33
 - 3) Provide adequate drainage to prevent water ponding.
 - 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 5) A plate rating reduction of 20% has been applied for the green lumber members.
 - 6) Bearing at joint(s) 9 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

LOAD CASE(S) Standard



August 22, 2005

<p>WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 BEFORE USE.</p> <p>Design valid for use only with Mittek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.</p>	<p>7777 Greenback Lane Suite 109 Citrus Heights, CA, 95810</p> <p>MITEK</p>
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JOB J5-277	Truss B1	Truss Type CAL HIP	Qty 1	Ply 3	MEEKS E.G. / PRESIDIO	R17952423
LATHAM TRUSS, VALLEJO, CA, 94592					Job Reference (optional) 6/2005 Jul 13 2005 Mitek Industries, Inc. Fri Aug 19 15:19:30 2005 Page 1	

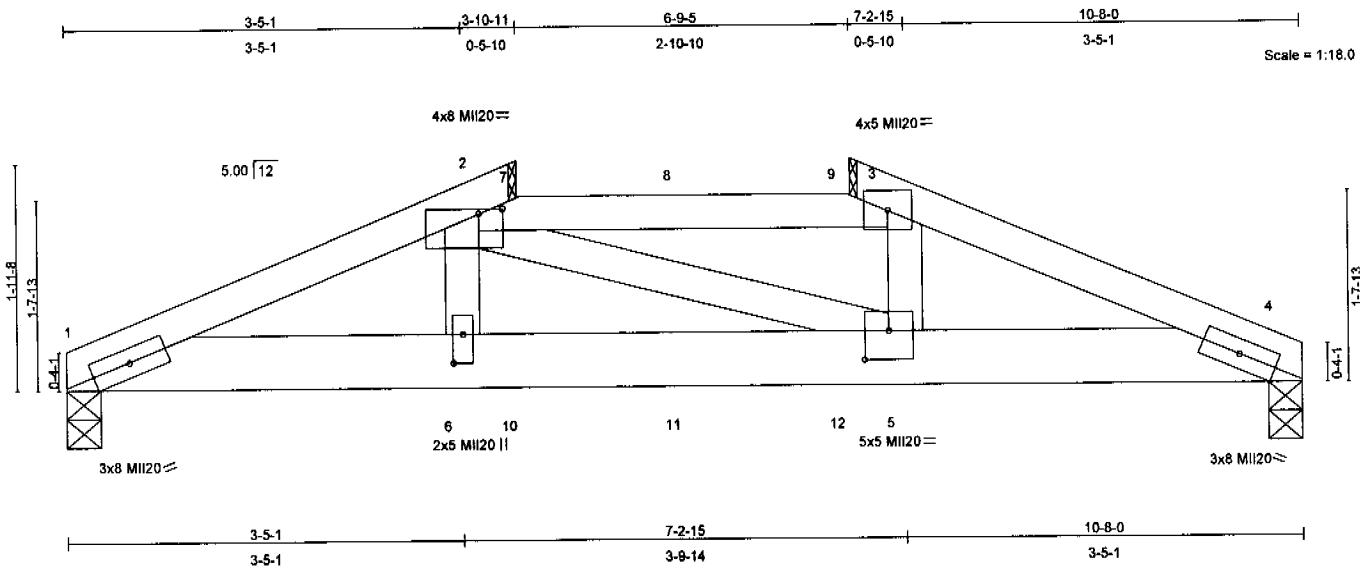


Plate Offsets (X, Y): [2-0-2-8,0-0-8], [5-0-2-8,0-3-0], [6-0-3-0,0-1-0]						
LOADING (psf)	SPACING	CSI	DEFL	in (loc)	l/def	L/d
TCLL 16.0	Plates Increase 2-0-0 Lumber Increase 1.25	TC 0.18	Vert(LL) -0.05	5-6	>999	360
TCDL 14.0	Rep Stress Incr NO	BC 0.60	Vert(TL) -0.11	5-6	>999	240
BCLL 0.0	Code UBC97/ANSI95	WB 0.36	Horz(TL) 0.03	4	n/a	n/a
BCDL 8.0		(Matrix)				
						PLATES MII20
						GRIP 220/195
						Weight: 143 lb

LUMBER
TOP CHORD 2 X 4 DF No.1&Btr G
BOT CHORD 2 X 6 DF No.2 G
WEBS 2 X 4 DF Std G

BRACING
TOP CHORD Sheathed or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=4258/0-3-8, 4=4258/0-3-8
Max Horz 1=2(load case 4)
Max Uplift 1=-25(load case 5), 4=-25(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-8881/93, 2-7=-7960/100, 7-8=-7960/100, 8-9=-7960/100, 3-9=-7960/100, 3-4=-8831/91
BOT CHORD 1-8=-70/8311, 6-10=-76/8006, 10-11=-76/8006, 11-12=-76/8006, 5-12=-76/8006, 4-5=-68/8262
WEBS 2-6=0/2672, 2-5=-114/19, 3-5=0/2668

- NOTES**
- 3-ply truss to be connected together with 10d Common(.148"x3") Nails as follows:
Top chords connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 6 - 2 rows at 0-4-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
 - All loads are considered equally applied to all piles, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
 - Unbalanced roof live loads have been considered for this design.
 - This truss has been designed for the wind loads generated by 80 mph winds at 25 ft above ground level, using 5.0 psf top chord dead load and 5.0 psf bottom chord dead load, 100 mi from hurricane oceanline, on an occupancy category I, condition I enclosed building, of dimensions 45 ft by 24 ft with exposure B ASCE 7-93 per UBC97/ANSI95. If end verticals or cantilevers exist, they are exposed to wind. If porches exist, they are not exposed to wind. The lumber DOL increase is 1.33, and the plate grip increase is 1.33.
 - Provide adequate drainage to prevent water ponding.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - A plate rafter reduction of 20% has been applied for the green lumber members.
 - Girder carries hip end with 3-11-0 end setback and tie-in span of 2-0-0 from subgirder.
 - Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 153 lb down and 35 lb up at 6-9-0, and 153 lb down and 35 lb up at 3-11-0 on top chord. The design/selection of such connection device(s) is the responsibility of others.

LOAD CASE(S) Standard

Continued on page 2



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WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 BEFORE USE.
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Citrus Heights, CA, 95610



Job	Truss	Truss Type	Qty	Ply	MEEKS E.G. / PRESIDIO	R17052423
J5-277	B1	CAL HIP	1	3	Job Reference (optional)	

LATHAM TRUSS, VALLEJO, CA, 94592

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LOAD CASE(S) Standard

1) Regular: Lumber Increase=1.25, Plate Increase=1.25

Uniform Loads (plf)

Vert: 1-2=-60, 2-7=-60, 3-9=-60, 3-4=-60, 1-4=-722(F=-6, B=700)

Concentrated Loads (lb)

Vert: 7=-153 9=-153

Trapezoidal Loads (plf)

Vert: 7=-84-to-8=-99, 8=-99-to-9=-84

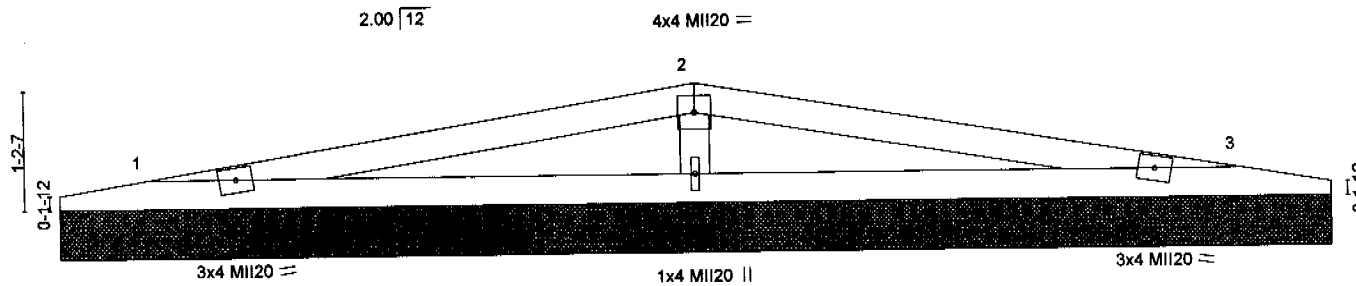
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7473 BEFORE USE.
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Job	Truss	Truss Type	Qty	Ply	MEEKS E.G. / PRESIDIO	R17952424
J5-277	8G1	ROOF TRUSS	2	1	Job Reference (optional)	

LATHAM TRUSS, VALLEJO, CA, 94592 6.200 s Feb 11 2005 MiTek Industries, Inc. Mon Aug 22 14:00:22 2005 Page 1



LOADING (psf)		SPACING 2-0-0		CSI		DEFL in (loc) l/def L/d		PLATES MII20		GRIP 220/195	
TCLL	20.0	Plates Increase	1.25	TC	0.16	Vert(LL)	n/a	-	n/a	999	
TCDL	14.0	Lumber Increase	1.25	BC	0.11	Vert(TL)	n/a	-	n/a	999	
BCLL	0.0	Rep Stress Incr	NO	WB	0.04	Horz(TL)	0.00	3	n/a	n/a	
BCDL	8.0	Code	UBC97/ANSI95	(Simplified)						Weight: 33 lb	

LUMBER
TOP CHORD 2 X 4 DF No.1&Btr G
BOT CHORD 2 X 4 DF No.1&Btr G
WEBS 2 X 4 DF Std G

BRACING
TOP CHORD Sheathed or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=231/12-8-8, 3=231/12-8-8, 4=457/12-8-8
Max Uplift 1=-1(load case 3), 3=-1(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-7/8, 2-3=-7/8
BOT CHORD 1-4=0/6, 3-4=0/6
WEBS 2-4=-370/54

NOTES

- Unbalanced roof live loads have been considered for this design.
- This truss has been designed for the wind loads generated by 80 mph winds at 25 ft above ground level, using 5.0 psf top chord dead load and 5.0 psf bottom chord dead load, 100 mi from hurricane oceanline, on an occupancy category I, condition I enclosed building, of dimensions 45 ft by 24 ft with exposure B ASCE 7-93 per UBC97/ANSI95. If end verticals or cantilevers exist, they are exposed to wind. If porches exist, they are not exposed to wind. The lumber DOL increase is 1.33, and the plate grip increase is 1.33.
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail".
- Gable requires continuous bottom chord bearing.
- Gable studs spaced at 1-4-0 oc.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- A plate rating reduction of 20% has been applied for the green lumber members.

LOAD CASE(S) Standard



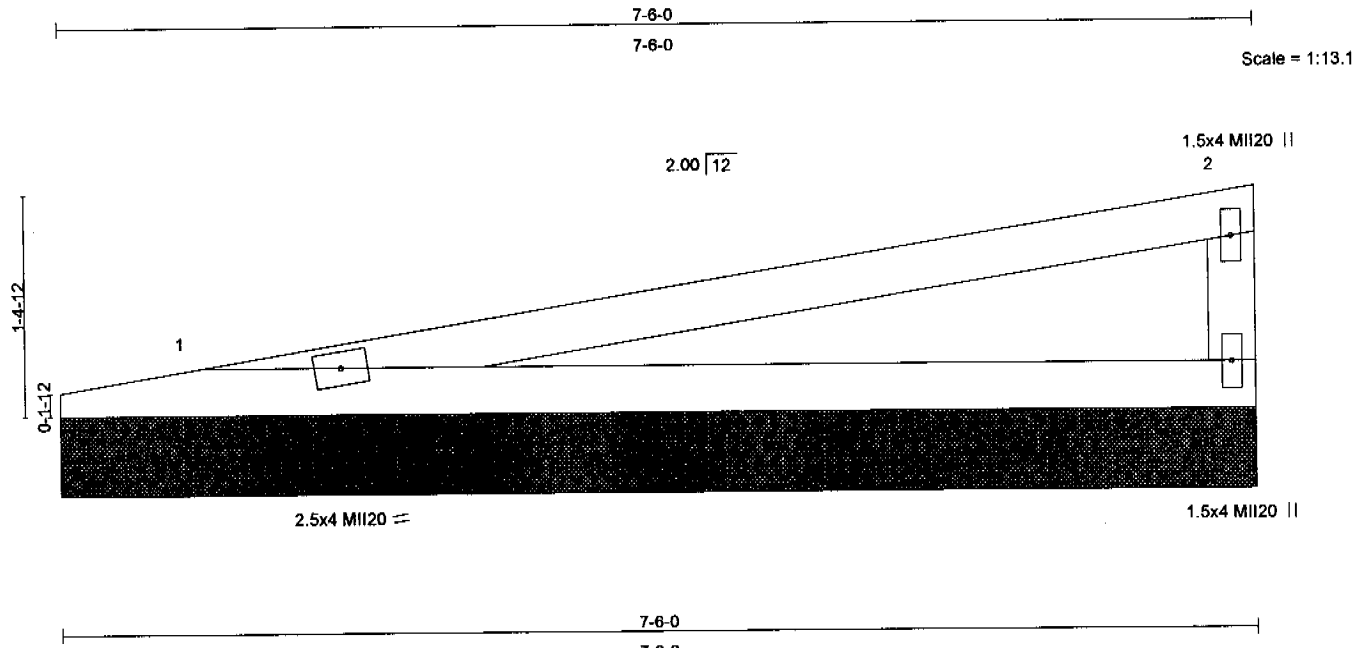
August 22, 2005

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Job	Truss	Truss Type	Qty	Ply	MEEKS E.G. / PRESIDIO	R17852425
J5-277	BG2	ROOF TRUSS	2	1	Job Reference (optional)	

LATHAM TRUSS, VALLEJO, CA, 94592

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LOADING (psf)	SPACING	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 20.0	Plates Increase 1.25	TC 0.33	Vert(LL)	n/a	n/a	999	MI120	220/195
TCDL 14.0	Lumber Increase 1.25	BC 0.17	Vert(TL)	n/a	n/a	999		
BCLL 0.0	Rep Stress Incr NO	WB 0.00	Horz(TL)	0.00	3	n/a		
BCDL 8.0	Code UBC97/ANSI95	(Simplified)						Weight: 20 lb

LUMBER
 TOP CHORD 2 X 4 DF No.1&Btr G
 BOT CHORD 2 X 4 DF No.1&Btr G
 WEBS 2 X 4 DF Std G

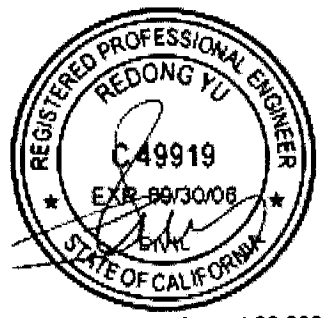
BRACING
 TOP CHORD Sheathed or 7-6-0 oc purlins, except end verticals.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=272/7-6-0, 3=272/7-6-0
 Max Horz 1=38(load case 4)
 Max Uplift 3=-4(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=-19/0, 2-3=-220/36
 BOT CHORD 1-3=-8/11

- NOTES**
- 1) This truss has been designed for the wind loads generated by 80 mph winds at 25 ft above ground level, using 5.0 psf top chord dead load and 5.0 psf bottom chord dead load, 100 mi from hurricane oceanline, on an occupancy category I, condition I enclosed building, of dimensions 45 ft by 24 ft with exposure B ASCE 7-93 per UBC97/ANSI95. If end verticals or cantilevers exist, they are exposed to wind. If porches exist, they are not exposed to wind. The lumber DOL increase is 1.33, and the plate grip increase is 1.33.
 - 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail".
 - 3) Gable requires continuous bottom chord bearing.
 - 4) Gable studs spaced at 1-4-0 oc.
 - 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 6) A plate rating reduction of 20% has been applied for the green lumber members.

LOAD CASE(S) Standard



August 22, 2005

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Job	Truss	Truss Type	Qty	Ply	MEEKS E.G. / PRESIDIO	R17952426
J5-277	C1	ROOF TRUSS	1	1	Job Reference (optional)	
LATHAM TRUSS, VALLEJO, CA, 94592					6.200 s Jul 13 2005 MITek Industries, Inc. Fri Aug 19 15:19:31 2005 Page 1	

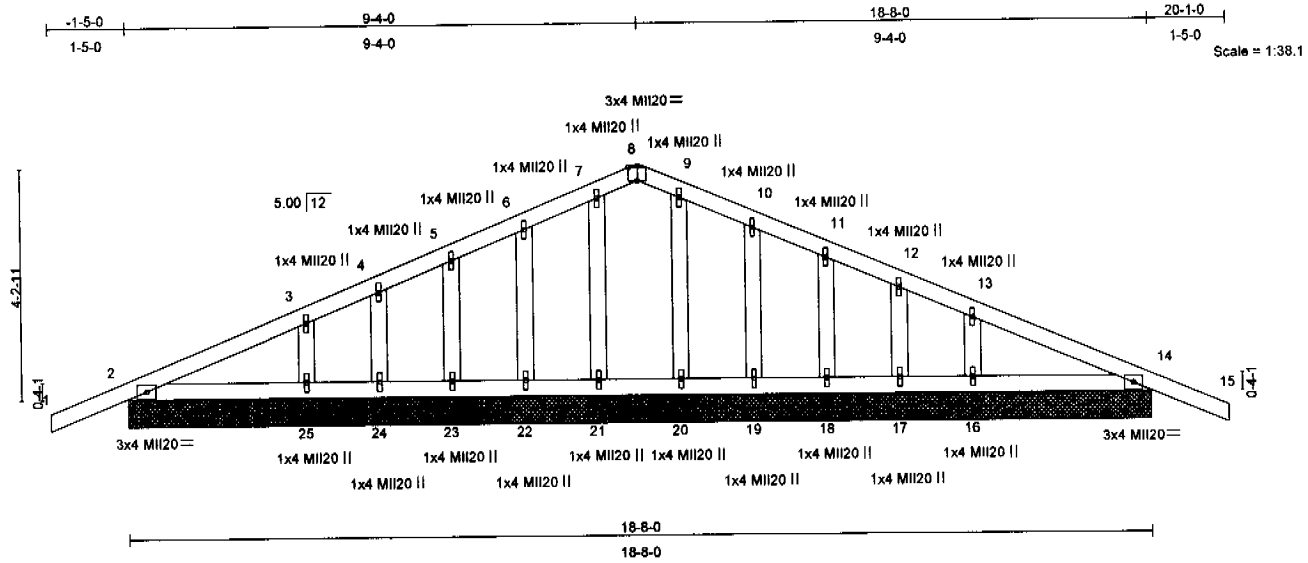


Plate Offsets (X,Y): [8:0-2-0,Edge]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL 16.0	Plates Increase	1.25	TC 0.11	Vert(LL)	-0.00	15	n/r	240	MII20
TCDL 14.0	Lumber Increase	1.25	BC 0.05	Vert(TL)	-0.00	15	n/r	180	
BCLL 0.0	Rep Stress Incr	NO	WB 0.02	Horz(TL)	0.00	14	n/a	n/a	
BCDL 8.0	Code	UBC97/ANSI95	(Matrix)	Wind(LL)	0.01	15	n/r	180	Weight: 88 lb

LUMBER
 TOP CHORD 2 X 4 DF No.1&Btr G
 BOT CHORD 2 X 4 DF No.1&Btr G
 OTHERS 2 X 4 DF Std G

BRACING
 TOP CHORD Sheathed or 6-0-0 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

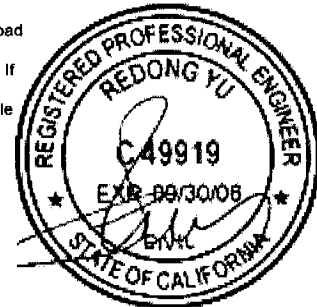
REACTIONS (lb/size) 2=214/18-8-0, 14=214/18-8-0, 21=105/18-8-0, 22=99/18-8-0, 23=111/18-8-0, 24=55/18-8-0, 25=211/18-8-0, 20=105/18-8-0, 19=99/18-8-0, 18=111/18-8-0, 17=55/18-8-0, 16=211/18-8-0
 Max Horz2=7(load case 4)
 Max Uplift2=-45(load case 5), 14=-45(load case 5), 22=-10(load case 5), 23=-2(load case 4), 24=-15(load case 4), 19=-10(load case 3), 18=-2(load case 5), 17=-15(load case 3)
 Max Grav2=214(load case 1), 14=214(load case 1), 21=105(load case 1), 22=100(load case 6), 23=111(load case 1), 24=55(load case 6), 25=211(load case 6), 20=105(load case 1), 19=100(load case 7), 18=111(load case 1), 17=55(load case 7), 16=211(load case 7)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/31, 2-3=-50/30, 3-4=-36/21, 4-5=-23/32, 5-6=-25/43, 6-7=-25/55, 7-8=-26/55, 8-9=-26/55, 9-10=-25/55, 10-11=-25/42, 11-12=-23/29, 12-13=-36/18, 13-14=-50/30, 14-15=0/31
 BOT CHORD 2-25=0/40, 24-25=0/40, 23-24=0/40, 22-23=0/40, 21-22=0/40, 20-21=0/40, 19-20=0/40, 18-19=0/40, 17-18=0/40, 16-17=0/40, 14-16=0/40
 WEBS 7-21=-82/0, 6-22=-80/22, 5-23=-86/18, 4-24=-47/16, 3-25=-161/26, 9-20=-82/0, 10-19=-80/22, 11-18=-88/18, 12-17=-47/16, 13-16=-161/26

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) This truss has been designed for the wind loads generated by 80 mph winds at 25 ft above ground level, using 5.0 psf top chord dead load and 5.0 psf bottom chord dead load, 100 mi from hurricane oceanline, on an occupancy category I, condition I enclosed building, of dimensions 45 ft by 24 ft with exposure B ASCE 7-93 per UBC97/ANSI95 if end verticals or cantilevers exist, they are exposed to wind. If porches exist, they are not exposed to wind. The lumber DOL increase is 1.33, and the plate grip increase is 1.33
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
- 4) Gable requires continuous bottom chord bearing.
- 5) Gable studs spaced at 1-4-0 oc.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) A plate rating reduction of 20% has been applied for the green lumber members.

LOAD CASE(S) Standard



August 22, 2005

<p>WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE. Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, D58-89 and BC311 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.</p>	<p>7777 Greenback Lane Suite 109 Citrus Heights, CA, 95610</p>
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Job J5-277	Truss C2	Truss Type FINK	Qty 4	Ply 1	MEEKS E.G. / PRESIDIO	R17952427
LATHAM TRUSS, VALLEJO, CA, 94582					Job Reference (optional)	

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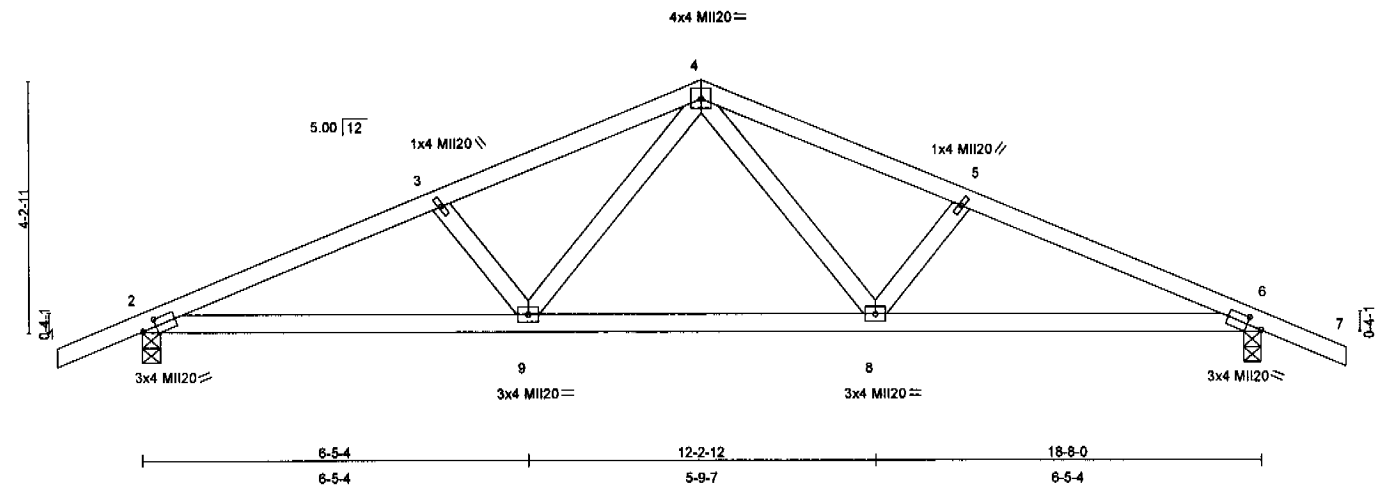
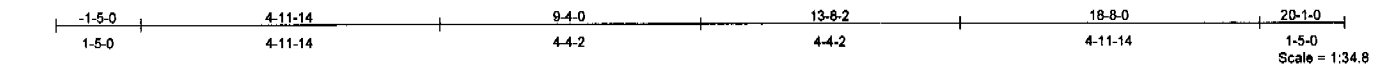


Plate Offsets (X,Y): [2:0-3-0,0-1-8], [6:0-3-0,0-1-8]									
LOADING (psf)	SPACING	CSI	DEFL	in	(loc)	l/def	L/d	PLATES	GRIP
TCLL 16.0	Plates Increase 2-0-0 1.25	TC 0.19	Vert(LL) -0.04	8-9	>999	360		MII20	220/195
TCDL 14.0	Lumber Increase 1.25	BC 0.36	Vert(TL) -0.10	8-9	>999	240			
BCLL 0.0	Rep Stress Incr YES	WB 0.15	Horz(TL) 0.02	6	n/a	n/a			
BCDL 8.0	Code UBC97/ANSI95	(Simplified)							Weight: 76 lb

LUMBER
TOP CHORD 2 X 4 DF No.1&Btr G
BOT CHORD 2 X 4 DF No.1&Btr G
WEBS 2 X 4 DF Std G

BRACING
TOP CHORD Sheathed or 5-9-8 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 2=792/0-3-8, 6=792/0-3-8
Max Horz 2=7 (load case 4)
Max Uplift 2=-40 (load case 5), 6=-40 (load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/16, 2-3=-1264/21, 3-4=-1099/26, 4-5=-1099/26, 5-6=-1264/21, 6-7=0/16
BOT CHORD 2-9=0/1158, 8-9=0/801, 6-8=0/1158
WEBS 3-9=-244/60, 4-9=0/375, 4-8=0/375, 5-8=-244/60

- NOTES**
- Unbalanced roof live loads have been considered for this design.
 - This truss has been designed for the wind loads generated by 80 mph winds at 25 ft above ground level, using 5.0 psf top chord dead load and 5.0 psf bottom chord dead load, 100 mi from hurricane oceanline, on an occupancy category I, condition I enclosed building, of dimensions 45 ft by 24 ft with exposure B ASCE 7-93 per UBC97/ANSI95. If end verticals or cantilevers exist, they are exposed to wind. If porches exist, they are not exposed to wind. The lumber DOL increase is 1.33, and the plate grip increase is 1.33.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - A plate rating reduction of 20% has been applied for the green lumber members.

LOAD CASE(S) Standard



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Job J5-277	Truss V1	Truss Type ROOF TRUSS	3x4 MII20	Qty 1	Ply 1	MEEKS E.G. / PRESIDIO	R17952428
LATHAM TRUSS, VALLEJO, CA, 94592						Job Reference (optional) 6.200 s Jul 13 2005 MiTek Industries, Inc. Fri Aug 19 15:19:32 2005 Page 1	

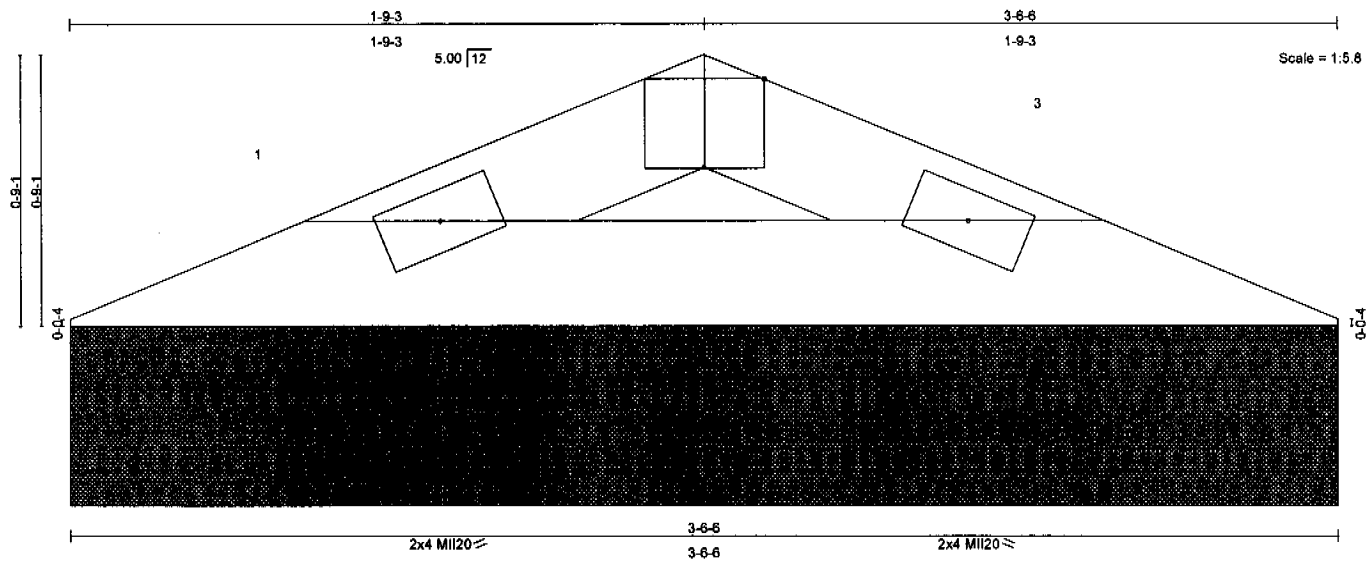


Plate Offsets (X,Y): [2:0-2:0,Edge]

LOADING (psf)	SPACING	CSI	DEFL	in (loc)	I/defl	L/d	PLATES	GRIP
TCLL 16.0	Plates Increase 2-0-0 1.25	TC 0.00	Vert(LL)	n/a	-	n/a	MI120	220/195
TCDL 14.0	Lumber Increase 1.25	BC 0.01	Vert(TL)	n/a	-	n/a		
BCLL 0.0	Rep Stress Incr YES	WB 0.00	Horz(TL)	0.00	3	n/a		
BCDL 8.0	Code UBC97/ANSI95	(Simplified)						Weight: 8 lb

LUMBER

TOP CHORD 2 X 4 DF No.1&Btr G
BOT CHORD 2 X 4 DF No.1&Btr G

BRACING

TOP CHORD Sheathed or 3-6-6 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=83/3-6-6, 3=83/3-6-6
Max Horz 1=1(load case 4)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-85/14, 2-3=-85/14
BOT CHORD 1-3=-9/78

NOTES

- Unbalanced roof live loads have been considered for this design.
- This truss has been designed for the wind loads generated by 80 mph winds at 25 ft above ground level, using 5.0 psf top chord dead load and 5.0 psf bottom chord dead load, 100 mi from hurricane oceanline, on an occupancy category I, condition I enclosed building, of dimensions 45 ft by 24 ft with exposure B ASCE 7-93 per UBC97/ANSI95. If end verticals or cantilevers exist, they are exposed to wind. If porches exist, they are not exposed to wind. The lumber DOL increase is 1.33, and the plate grip increase is 1.33.
- Gable requires continuous bottom chord bearing.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- A plate rating reduction of 20% has been applied for the green lumber members.

LOAD CASE(S) Standard



August 22, 2005

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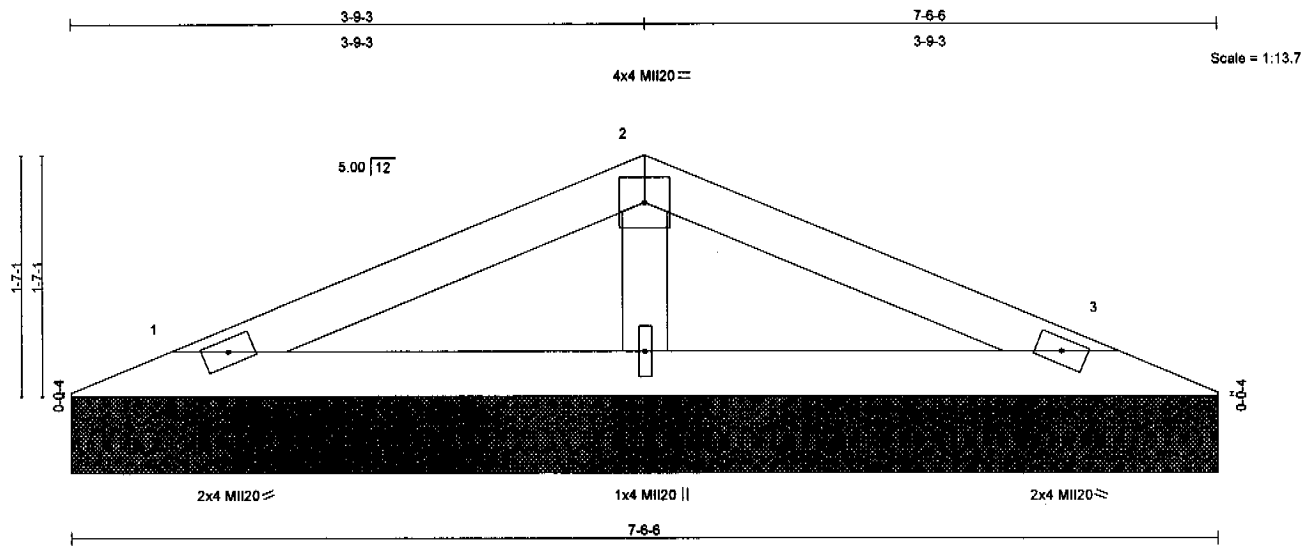
7777 Greenback Lane
Suite 109
Citrus Heights, CA, 95610



JOB	Truss	Truss Type	Qty	Ply	MEEKS E.G./PRESIDIO	R17952429
J5-277	V2	ROOF TRUSS	1	1	Job Reference (optional)	

LATHAM TRUSS, VALLEJO, CA, 94592

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LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 16.0	2-0-0	TC 0.05	in (loc) l/defl L/d	II20	220/195
TGDL 14.0	Plates Increase 1.25	BC 0.04	Vert(LL) n/a - n/a 999		
BCLL 0.0	Lumber Increase 1.25	WB 0.02	Vert(TL) n/a - n/a 999		
BCDL 8.0	Rep Stress Incr YES	(Simplified)	Horz(TL) 0.00 3 n/a n/a		
	Code UBC97/ANSI95				Weight: 21 lb

LUMBER

TOP CHORD 2 X 4 DF No.1&Btr G
 BOT CHORD 2 X 4 DF No.1&Btr G
 OTHERS 2 X 4 DF Std G

BRACING

TOP CHORD Sheathed or 6-0-0 oc purlins.
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 1=124/7-6-6, 3=124/7-6-6, 4=221/7-6-6

Max Horz 1=2(load case 4)
 Max Uplift 1=-4(load case 5), 3=-4(load case 5)

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD 1-2=-18/9, 2-3=-18/9
 BOT CHORD 1-4=0/17, 3-4=0/17
 WEBS 2-4=-171/25

NOTES

- Unbalanced roof live loads have been considered for this design.
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- Gable requires continuous bottom chord bearing.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- A plate rating reduction of 20% has been applied for the green lumber members.

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