

CONSTRUCTION LENDING AGENCY

I hereby affirm under penalty of perjury that there is a construction lending agency for the performance of the work for which this permit is issued (Sec. 3097, Civ.C).

Lenders Name _____

Lenders Address _____

LICENSED CONTRACTORS DECLARATION

I hereby affirm under penalty of perjury that I am licensed under provisions of Chapter 9 (commencing with Section 7000) of Division 3 of the Business and Professions Code and my license is in full force and effect.

License Class C-39 Lic. Number 420375

Date 7-8-97 Contractor Larry Peer
(Signature)

OWNER - BUILDER DECLARATION

I hereby affirm under penalty of perjury that I am exempt from the Contractors License Law for the following reason (Sec. 7031.5, Business and Professions Code: Any city or county which requires a permit to construct, alter, improve, demolish, or repair any structure, prior to its issuance, also requires the applicant for such permit to file a signed statement that he or she is licensed pursuant to the provisions of the Contractors License Law (Chapter 9 (commencing with Section 7000) of Division 8 of the Business and Professions Code) or that he or she is exempt therefrom and the basis for the alleged exemption. Any violation of Section 7031.5 by any applicant for a permit subjects the applicant to a civil penalty of not more than five hundred dollars (\$500.00):

I, as owner of the property, or my employees with wages as their sole compensation, will do the work, and the structure is not intended or offered for sale (Sec. 7044, Business and Professions Code: The Contractors License Law does not apply to an owner of property who builds or improves thereon, and who does such work himself or herself or through his or her own employees, provided that such improvements are not intended or offered for sale. If, however, the building or improvement is sold within one year of completion, the owner-builder will have the burden of proving that he or she did not build or improve for the purpose of sale.)

I, as owner of the property, am exclusively contracting with licensed contractors to construct the project (Sec. 7044, Business and Professions Code: The Contractors License Law does not apply to an owner of property who builds or improves thereon, and who contracts for such projects with a contractor(s) licensed pursuant to the Contractors License Law).

I am exempt under Sec. _____ B & P C for this reason _____

Date _____ Owner _____
(Signature)

In issuing this building permit, the applicant represents, and the City relies on the representation of the applicant, that the applicant verified all measurements and locations shown on the application or accompanying drawings and that the improvement to be constructed does not violate any law or private agreement relating to permissible or prohibited locations for such improvements. This building permit does not authorize any illegal location of any improvement or the violation of any private agreement relating to location of improvements.

I certify that I have read this application and state that all information is correct. I agree to comply with all city and county ordinances and state laws relating to building construction and hereby authorize representative of this city to enter upon the abovementioned property for inspection purposes.

Date 7/8/97 Signature of Applicant or Agent Carolyn Peer

BUILDING SITE ADDRESS

1276 Nevis Ct

SUITE

INSP. AREA 2R

ASSESSOR PARCEL NO. 016-0305-019

COMMUNITY PLAN NO.

PLAN CHECK NO. PC

NAME OF APPLICANT

ADDRESS

ZIP CODE

PHONE NO.

LICENSED CONTRACTOR

Weather tight Roofing 4601 Summer Creek 95642 672-1113

PROPERTY OWNER

Kiyoshi Mori 1276 Nevis Ct

ARCH. ENGR.

LICENSE NO.

NO. OF STORIES

NO. OF ROOMS

ROOF COVERING

AREA 1ST FLOOR

TOTAL AREA

GARAGE AREA

PATIO AREA

USE ZONE

STREET WIDTH

THIS PERMIT IS FOR:

BUILDING MECHANICAL PLUMBING ELECTRICAL SITE FIRE

OCCUP. GROUP

NATURE OF WORK IN DETAIL

Re-roof 33 # of Tile

CONSTR. TYPE R3

Light Weight

FIRE SP. 1

FLOOD STATUS

(roof)

SPECIAL CONDITIONS ATTACHMENTS:

FED CODE 1A

CITY OF SACRAMENTO

PERMIT SERVICES

BUILDING INSPECTION DIVISION

264-7619

VALUATION \$ 1999

WORKER'S COMPENSATION DECLARATION

I hereby affirm under penalty of perjury one of the following declarations:

I have and will maintain a certificate of consent to self-insure for workers' compensation as provided for by Section 3700 of the Labor Code, for the performance of work for which the permit is issued.

I have and will maintain workers' compensation insurance, as required by Section 3700 of the Labor Code, for the performance of the work for which this permit is issued. My workers' compensation insurance carrier and policy number are:

Carrier

State Fund

Policy Number

1271896-96

10-1-97

(This section need not be completed if the permit is for one hundred dollars (\$100) or less) I certify that in the performance of the work for which this permit is issued, I shall not employ any person in any manner so as to become subject to the workers' compensation laws of California and agree that if I should become subject to the workers' compensation provisions of Section 3700 of the Labor Code, I shall forthwith comply with those provisions.

Date: 7-8-97

Applicant: Larry Peer

(Signature)

WARNING: FAILURE TO SECURE WORKERS' COMPENSATION COVERAGE IS UNLAWFUL AND SHALL SUBJECT AN EMPLOYER TO CRIMINAL PENALTIES AND CIVIL FINES UP TO ONE HUNDRED THOUSAND DOLLARS (\$100,000) IN ADDITION TO THE COST OF COMPENSATION. DAMAGES AS PROVIDED FOR IN SECTION 3706 OF THE LABOR CODE, INTEREST AND ATTORNEY'S FEES.

ISSUED BY

DATE ISSUED 7-8-97

BUILDING PERMIT FEE \$ 89

PLAN CHECK/PROC. FEE \$ 1900

S.M.I. FEE \$.50

CONST. EXCISE TAX \$

CITY BUS LICENSE \$.80

TECH. FEE \$ 432

WATER DEV. FEE \$

CITY SEWER DEV. FEE \$

REG. SEWER FEE \$

RESIDENTIAL CONST. TAX \$

TOTAL FEES \$ 11862

THIS PERMIT SHALL EXPIRE BY LIMITATION IF WORK AUTHORIZED IS NOT COMMENCED WITHIN 180 DAYS.

SCHOEN ENGINEERING

9524 BEDINGTON WAY
SACRAMENTO, CA 95827
(916) 369 6866
Lic. # C042913

July 7, 1997

Larry Peer
Weather-Tite Roofing Company
P.O. Box 6068
Folsom, CA 95673

SUBJECT: Reroof at 1276 Nevis Ct., Sacramento, CA

Dear Larry:

On July 2nd, 1997 I observed the roof structure of the residence at the above mentioned address. I found the roof of the main house and garage made up mostly of 2x6 Douglas fir No. 2 rafters at 2' o.c. with a few 2x4s in some of the shorter spans. The max. span in the house was 11'-4" plate to purlin. The max span in the garage was 14' for the 2x6's and 9'-6" for the 2x4's. The back porch had 2x6 rafters 2' o.c. spanning 11'-6".

Prior to reroofing the following modification should be made:

* In the garage reinforce all 2x6 rafters over 12'-4" long with 2x6s and all 2x4 rafters over 8' long with 2x4's. The reinforcing rafters should run from plate to the hip or ridge and be attached to the existing rafters with 16d common nails at 16" o.c..

* In the house the existing purlins are 2x4's. These should be upgraded to 2x6's. Where it is possible to install the new 2x6 up against the existing 2x4 purlin this may be done by laminating them together with 16d common nails @ 6" o.c. staggered. This will allow the utilization of existing braces. Where lap joints in the purlin or brace framing does not allow this a new 2x6 purlin should be installed with new braces off of the load bearing walls @ 6' o.c.. In no case should the braces support the purlin more than 6' apart or should the brace angles exceed 45 degrees from the vertical. All braces should be framed to load bearing walls and all braces should provide direct bearing for the purlins.

* At the main ridge of the house provide 2x4 rafter ties at the purlin elevation on each opposing pair of common rafters. Attach the ties to the rafters with 5-16d common nails.

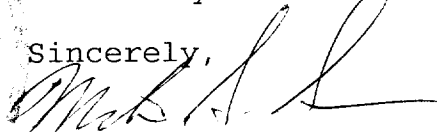
It is my finding that with the above mentioned modification this structure is adequate for the proposed reroof system which is comprised of: 1/2" plywood installed over the existing skip sheathing; 30 lb. tarred felt; 1x2 batts; lightweight concrete tile weighing 7.4 lbs./sq.ft..

NOTE: It is possible when reroofing that the increased load to structural elements also supporting wall, ceiling and floor finishes could cause some minor cosmetic cracking of these finishes. This is not untypical of a wood framed house and does not necessarily constitute structural inadequacy of these members.

This report deals with the structural adequacy of roof supporting members that were readily observable. It does not address any structure that was covered by wall finishes, buried in the ground or was otherwise not directly observable. These structures were assume to be of standard construction as called for in the Uniform Building Code. Also, it does not address any existing deflection or warping of roof members. The repair of such deflections to improve architectural appearance, is at the option of the home owner and the roofing contractor.

I would like to thank you for allowing me to provide my services in this matter. Please let me know if I may be of further assistance.

Sincerely,



Mark S. Schoen P.E.

MSS:mss

C:\WP51\3-ENG97\WTRF004.001

ISSUED

JUL 06 1997

Structural Building Division

SCHOEN ENGINEERING

Client: Weather-Tite Roofing

Date: 6/10/97

MSB
Exp. 3/00

Job No.: WTRF004

Page: _____

Job Title: _____

Check for adequacy of existing rafters for reroofing with material heavier than the original installation. Lumber grade is Douglas fir no. 2

$$E_w = 1700000 \quad F_b = 875 \quad C_d = 1.25 \quad C_f = 1.5 \quad C_r = 1.15$$

$$F_{bp} = F_b \cdot C_d \cdot C_f \cdot C_r \quad F_{bp} = 1886.719$$

The Existing garage rafters are 2x4 douglas fir 24" o.c. spanning 9'6" from wall plate to the ridge. These are to be doubled up with 16d common nails 16" o.c.

$$b = 1.5 \quad d = 3.5 \quad I_{2x4} = b \cdot \frac{d^3}{12} \quad S_{2x4} = b \cdot \frac{d^2}{6} \quad \text{Stiff}_{2x4} = I_{2x4} \cdot E_w$$

Superimposed dead and live loads:

$$\text{Tile dead load:} \quad D_{Lt} = 7.4 \quad sp = 1$$

$$\text{Live load:} \quad LL = 16$$

$$\text{Structure roof dead load:} \quad D_{Lext} = 4$$

Check maximum span based on deflection limit of L/240 for rafters w/ceiling

L/180 for rafters w/o ceiling:

(Note: The formula used to calculate allowable span for deflections is based on a two span continuous beam where rafters are continuous over a midspan purlin with live load on one span only and dead load on both spans otherwise it is based on a simple span condition)

$$\text{Load:} \quad wd = sp \cdot \frac{1}{12} \cdot \left(\frac{D_{Lext} + D_{Lt}}{1} + \frac{LL}{1} \right) \quad ws = sp \cdot \frac{1}{12} \cdot ((D_{Lext} + D_{Lt}) + LL)$$

$$L_{maxd} = \sqrt[3]{\frac{77 \cdot \text{Stiff}_{2x4}}{1 \cdot 180 \cdot wd}} \cdot \frac{1}{12} \quad L_{maxd} = 9.959 \quad > 9.5' \text{ therefore O.K.}$$

Check for maximum span based on stresses:

$$L_{maxs} = \sqrt[3]{\frac{F_{bp} \cdot 8 \cdot S_{2x4}}{ws}} \cdot \frac{1}{12} \quad L_{maxs} = 11.857 \quad > 9.5' \text{ therefore O.K.}$$

ISSUED
JUL 08 1997
Schoen Engineering

2x6 Douglas fir No. 2 rafter:

$E_w = 1600000$ $F_b = 875$ $C_d = 1.25$ $C_f = 1.3$
 $C_r = 1.15$ $F_{bp} = F_b \cdot C_d \cdot C_f \cdot C_r$ $F_{bp} = 1635.156$
 $b = 1.5$ $d = 5.5$ $I_{x2x6} = b \cdot \frac{d^3}{12}$ $S_{x2x6} = b \cdot \frac{d^2}{6}$ $Stiff_{2x6} = I_{x2x6} \cdot E_w$

The existing garage rafters are 2x6's 2' o.c. spanning 14' from plate to ridge. These are to be doubled up with 16d common nails 16" o.c.

Superimposed dead and live loads:

Tile dead load: $DL_t = 10.5$ Live load: $LL = 16$

Structure roof dead load: $DL_{ext} = 1 + 1 + 1.5 + 1$ Spacing: $sp = \frac{12}{12}$

Check maximum span based on deflection limit of L/240 for rafters w/ceiling

L/180 for rafters w/o ceiling:

(Note: The formula used to calculate allowable span for deflections is based on a two span continuous beam where rafters are continuous over a midspan purlin with live load on one span only and dead load on both spans otherwise it is based on a simple span condition)

Load: $w_d = sp \cdot \frac{1}{12} \cdot ((DL_{ext} + DL_t) + LL)$ $w_s = sp \cdot \frac{1}{12} \cdot ((DL_{ext} + DL_t) + LL)$

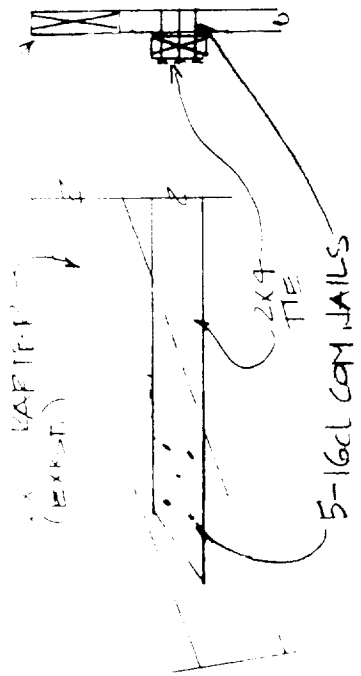
$L_{maxd} = \frac{77 \cdot Stiff_{2x6}}{1 \cdot 180 \cdot w_d} \cdot \frac{1}{12}$ $L_{maxd} = 14.719 > 14$ therefore O.K.

Check for maximum span based on stresses:

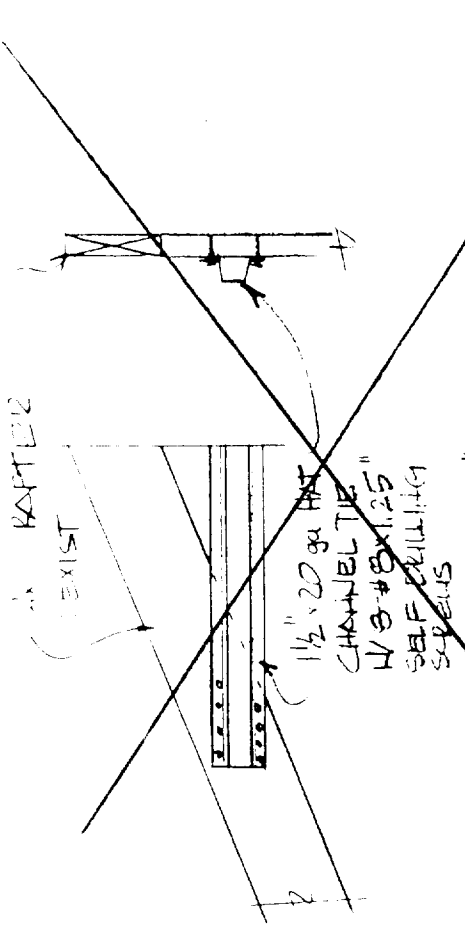
$L_{maxs} = \frac{F_{bp} \cdot 8 \cdot S_{x2x6}}{ws} \cdot \frac{1}{12}$ $L_{maxs} = 16.307 > 14$ therefore O.K.

ISSUED
 JUL 06 1997
 Commercial Building Division

TYPICAL CONNECTIONS

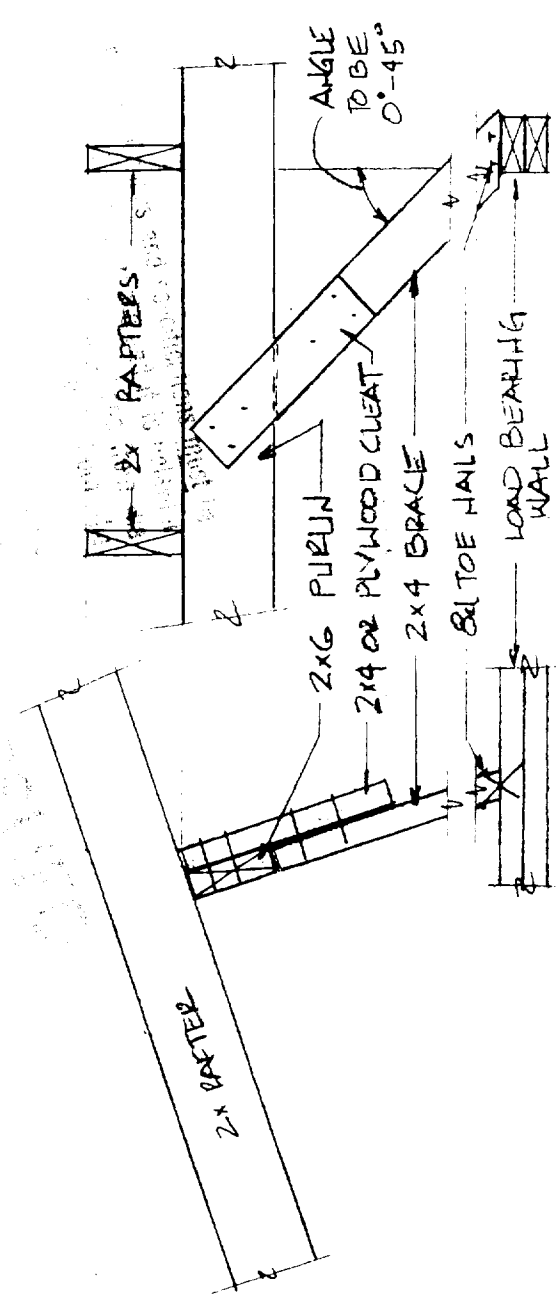


ATTACHMENT OF 2x4 TIE



ATTACHMENT OF 1/2 x 20ga HAT SECTION TIE

M. J. H.
 6/23/00



2x4 BRACE TO PURLIN CONNECTIONS

