

CITY OF SACRAMENTO

Permit No: 9800758

1231 I Street, Sacramento, CA 95814

Insp Area: 4

Site Address: 2460 WEST EL CAMINO AV SAC

Sub-Type: NCOM

Parcel No: 2251010012

Housing (Y/N): N

CONTRACTOR

OWNER

ARCHITECT

TEACHERS RET SYSTEM STATE OF
1201 N CLARK ST 300
CHICAGO IL 60610

Nature of Work: CONSTRUCTION OF NEW CARL'S JR RESTAURANT

CONSTRUCTION LENDING AGENCY: I hereby affirm under penalty of perjury that there is a construction lending agency for the performance of the work for which this permit is issued (Sec. 3097, Civ. C).

Lender's Name _____ Lender's Address _____

LICENSED CONTRACTORS DECLARATION: I hereby affirm under penalty of perjury that I am licensed under provisions of Chapter 9 (commencing with section 7000) of Division 3 of the Business and Professions Code and my license is in full force and effect.

License Class C36B License Number 570943 Date 5/18/98 Contractor Signature Douglas Van Dyke

OWNER-BUILDER DECLARATION: I hereby affirm under penalty of perjury that I am exempt from the contractors License Law for the following reason (Sec. 7031.5, Business and Professions Code; any city or county which requires a permit to construct, alter, improve, demolish, or repair any structure, prior to its issuance, also requires the applicant for such permit to file a signed statement that he or she is licensed pursuant to the provisions of the Contractors License Law (Chapter 9 (commencing with Section 7000) of Division 8 of the Business and Professions Code) or that he or she is exempt therefrom and the basis for the alleged exemption. Any violation of Section 7031.5 by any applicant for a permit subjects the applicant to a civil penalty of not more than five hundred dollars (\$500.00);

I, as a owner of the property, or my employees with wages as their sole compensation, will do the work, and the structure is not intended or offered for sale (Sec. 7044, Business and Professional Code: The Contractors License Law does not apply to an owner of property who builds or improves thereon, and who does such work himself or herself or through his/her own employees, provided that such improvements are not intended or offered for sale. If, however, the building or improvement is sold within one year of completion, the owner-builder will have the burden of proving that he/she did not build or improve for the purpose of sale.)

I, as owner of the property, am exclusively contracting with licensed contractors to construct the project (Sec. 7044, Business and Professions Code: The Contractors License Law does not apply to an owner of property who builds or improves thereon, and who contracts for such projects with a contractor(s) licensed pursuant to the Contractors License Law).

I am exempt under Sec. _____ B & PC for this reason: _____

Date _____ Owner Signature _____

IN ISSUING THIS BUILDING PERMIT, the applicant represents, and the city relies on the representation of the applicant, that the applicant verified all measurements and locations shown on the application or accompanying drawings and that the improvement to be constructed does not violate any law or private agreement relating to permissible or prohibited locations for such improvements. This building permit does not authorize any illegal location of any improvement or the violation of any private agreement relating to location of improvements.

I certify that I have read this application and state that all information is correct. I agree to comply with all city and county ordinances and state laws relating to building construction and hereby authorize representative(s) of this city to enter upon the abovementioned property for inspection purposes.

Date 5/18/98 Applicant/Agent Signature Douglas Van Dyke

WORKER'S COMPENSATION DECLARATION: I hereby affirm under penalty of perjury one of the following declarations:

I have and will maintain a certificate of consent to self-insure for workers' compensation as provided for by Section 3700 of the Labor Code, for the performance of work for which the permit is issued.

I have and will maintain workers' compensation insurance, as required by Section 3700 of the Labor Code, for the performance of the work for which this permit is issued. My workers' compensation insurance carrier and policy number are:

Carrier Unicare Insurance Company Policy Number SC50099713649

(This section need not be completed if the permit is for \$100 or less) I certify that in the performance of the work for which this permit is issued, I shall not employ any person in any manner so as to become subject to the workers' compensation laws of California and agree that if I should become subject to the workers' compensation provisions of Section 3700 of the Labor Code, I shall forthwith comply with those provisions.

Date 5/18/98 Applicant Signature Douglas Van Dyke

WARNING: FAILURE TO SECURE WORKER'S COMPENSATION COVERAGE IS UNLAWFUL AND SHALL SUBJECT AN EMPLOYER TO CRIMINAL PENALTIES AND CIVIL FINES UP TO ONE HUNDRED THOUSAND DOLLARS (\$100,000) IN ADDITION TO THE COST OF COMPENSATION, DAMAGES AS PROVIDED FOR IN SECTION 3706 OF THE LABOR CODE, INTEREST AND ATTORNEY'S FEE.

THIS PERMIT SHALL EXPIRE BY LIMITATION IF WORK IS NOT COMMENCED WITHIN 180 DAYS.

CITY OF SACRAMENTO
BUILDING INSPECTIONS DIVISION
PERMIT SERVICES SECTION

PC # 5710

Address: 2440 West El Camino Av.

This application will need one or more of the following items before it can be issued.

Owner/Builder form

Current Certificate of Worker's Comp. (City of Sacramento listed as Certificate Holder.)

Letter of Authorization for employees to sign (Contractor).

School Impact Fee Form

Driveway Permit - 1231 I Street, #200, 264-7995

County Regional Sanitation Permit (Howard Richmond)
827 - 7th Street, Rm. 105, Window 10 - Ph: 875-6679

Special Inspection Forms

PERMIT FEES \$ _____

A-99 Flood Waiver Form

Hazardous Materials Form

SCAT X-11, X-12, Y-12

Other: _____

Date Notified

5/19/98

Plans in Bin #

5801

Initials:

K.G.

"D" By:

[Signature]

TEMPORARY C of O

CITY OF SACRAMENTO

** TEMPORARY **

2-weeks

Expires:

August 4, 1998

CERTIFICATE OF OCCUPANCY

For information contact (916) 264-5716

Building Address 2415 14th St. Lincoln Ave. Permit No. 98-00758

Building Use Restaurant Occupancy A-3

Building Owner CHE Motel Construction Type VN

Owner Address 3840 North F St., Sacramento, CA 95834 Sprinkled Yes () No

Portion of Building Occupied Entire Building Area 3,050 Sq. Ft.

This Temporary Certificate of Occupancy is for training and stocking only

This TCO expires August 4, 1998

07/21/98

Bradford J. Boehm, P.E.

Date Issued By: Print Sign City Building Official

Henry/Verga/Green/Malvar/Woodson
This Certificate, issued pursuant to the provisions of Section 99 of the Uniform Building Code, certifies that at time of issuance the described portion of the building has been inspected for compliance with the Uniform Building Code as adopted per Title 9 of the Sacramento City Code for the grade and division of occupancy and use for which the proposed occupancy is classified. Issuance of this certificate shall not be construed as an approval of a violation of any Codes, or Federal, State and City Laws or Ordinances. Certificates purporting to give authority to such violation shall not be valid. This certificate shall be posted in a conspicuous place on the premises and shall not be removed except by the City Building Official. No changes shall be made in the character of occupancy or use without approval of the City Building Official.

POST IN A CONSPICUOUS PLACE

CITY OF SACRAMENTO
CERTIFICATE OF OCCUPANCY

For Information Contact (916) 264-5716

Permit No. 98-00758

Occupancy A-3

Construction Type Y-N

Building Address 2640 West El Camino Avenue

Building Use Restaurant

Building Owner CHE Zeston

Owner Address 384 North Fwy Blvd., Sacramento, CA 95834

Portion of Building Occupied Entire building

Date Issued 02 03 98 By Print Bradford J. Boehm, P.E. City Building Official

Henry/Verga/Green/Mel...
This Certificate, issued pursuant to the requirements of Section 109 of the Uniform Building Code as adopted per Title 9 of the Sacramento City Code for the group and division of occupancy and use for which the proposed occupancy is classified. Issuance of this certificate shall not be construed as an approval of a violation shall not be valid. This certificate shall be posted in a conspicuous place on the premises and shall not be removed except by the City Building Official. No changes shall be made in the character of occupancy or use without approval of the City Building Official.

POST IN A CONSPICUOUS PLACE

NOTE: COVER OR CONCEAL ANY BUILDING, ELECTRICAL, PLUMBING OR MECHANICAL WORK WITHOUT INSPECTOR'S SIGNATURE IN PROPER PLACE.

INSPECTION	INSPECTOR	DATE
FOUNDATION FORMS		
EXTERIOR GROUND		
CONCRETE SLAB FORMS		
UMB. UNDERFLOOR/SLAB		
EXCH/UNDERFLOOR/SLAB		
ECT. UNDERGROUND		
ECT. CONDUIT-SLAB		
BEAR TO REFERENCE		
DO NOT COVER UNTIL INSTALLATION ABOVE HAS BEEN SIGNED		
DOOR JOISTS OR GIRDERS		
DO NOT INSTALL SUB FLOOR UNTIL ABOVE HAS BEEN SIGNED		
SULATION/WALL/FLOOR		
JP PLUMBING		
JP MECHANICAL WALL/CEIL.		
ROUGH ELECTRICAL WALL/CEIL.		
FRAME		
JOIST PLYWOOD NAIL, COMM. & APTS.		
CEILING LATH/SIDING		
SHAKE NAIL		
DO NOT COVER UNTIL INSTALLATION ABOVE HAS BEEN SIGNED		
LATH OR WALL BD. NAILING		
DO NOT TAPE PLASTER OR TOP UNTIL ABOVE HAS BEEN SIGNED		
SERVICE UNDERGRD CONDUIT		
SEWER SERVICE		
WATER SERVICE		
DRINKER SYSTEM		
DO NOT COVER UNTIL INSTALLATION ABOVE HAS BEEN SIGNED		
AS TEST		
EMP GAS ISSUED		
POWER POLE		
EMP. POWER #		
SWIMMING POOLS ONLY		
AS TEST		
LUMBER PRE-GUMITE		
LUMBER PRE-DECK		
ELECTRICAL PRE-GUNIT		
ELECTRICAL PRE-DECK		
ELECTRICAL UNDERGRD		
DO NOT COVER UNTIL INSTALLATION ABOVE HAS BEEN SIGNED		
NEEDY COMPLIANCE CERTIFICATE TO BE ON FILE PRIOR TO FINAL APPROVAL.		

SIGNED: _____ DATE: _____

FINAL APPROVALS

BUILDING: _____

ELECTRICAL: _____

PLUMBING: _____

MECHANICAL: _____

DO NOT OCCUPY BUILDING UNTIL ALL OF THE ABOVE HAVE BEEN SIGNED AND CERTIFICATE OF OCCUPANCY ISSUED

2460 West El Camino
 225-1010-012
 3841 North Fwy Blvd.
 Sacramento, CA
 43 Corporate Park #106
 Sacramento, CA
 95834
 92714
 (714) 862-9333
 SC-PUD
 30509
 30509
 MECHANICAL
 PLUMBING
 ELECTRICAL
 SITE
 FIRE
 A-3
 CONST
 TYP
 VN
 FIRE
 SP.
 N
 FED
 CODE
 18
 PERMIT
 NO.
 988
 D
 D
 7
 5
 8
 1
 VALUATION \$ 226,767.50
 ISSUED BY: [Signature]
 DATE ISSUED: 5/19/98
 BUILDING PERMIT FEE \$
 PLAN CHECK/PROC. FEE \$
 S.M.I. FEE \$
 CONST. EXCISE TAX \$
 CITY BUS LICENSE \$
 TECH. FEE \$
 WATER DEV. FEE \$
 CITY SEWER DEV. FEE \$
 REG. SEWER FEE \$
 RESIDENTIAL CONST. TAX \$
 TOTAL \$

NAME OF APPLICANT: Carl's Jr.
 ADDRESS: 3841 North Fwy Blvd., Sacramento, CA
 LICENSE NO.: 30509
 SPECIAL CONDITIONS ATTACHMENTS: SCU, CPP
 INSPECTIONS: 264-5191
 CITY OF SACRAMENTO
 WORKER'S COMPENSATION DECLARATION
 I hereby affirm under penalty of perjury one of the following declarations:
 I have and will maintain a certificate of consent to self-insure for workers' compensation as provided for by Section 3700 of the Labor Code, for the performance of work for which the permit is issued.
 I have and will maintain workers' compensation insurance, as required by Section 3700 of the Labor Code, for the performance of the work for which this permit is issued. My workers' compensation insurance carrier and policy number are:
 Carrier: _____
 Policy Number: _____
 (This section need not be completed if the permit is for one hundred dollars (\$100) or less) I certify that in the performance of the work for which this permit is issued, I shall not employ any person in any manner so as to become subject to the workers' compensation laws of California and agree that if I should become subject to the workers' compensation provisions of Section 3700 of the Labor Code, I shall forthwith comply with those provisions.
 Date: _____ Applicant: _____ (Signature)

1321

9800758c

CITY OF SACRAMENTO
APPLICATION FOR BUILDING PERMIT
DEPARTMENT OF PLANNING AND DEVELOPMENT
BUILDING INSPECTION DIVISION

1231 I Street, Room 200
Sacramento, CA 95814
(916) 264-7619 FAX 264-7046

RECEIVED

JAN 29 1998

ADDRESS 2460 West El Camino
 PARCELS # 295-1010-012
Building Inspection Division

P.C. # 5710
SUITE # -
AREA # -

CONTACT
NAME Linda Budge / CommTech Services
ADDRESS 9571 Miral del Rio
Sacramento ZIP 95827
PHONE (916) 363-4164

LICENSED CONTRACTOR
NAME _____
ADDRESS _____
PHONE _____ ZIP _____

ARCH./ENG.
NAME JES Partnership
ADDRESS 43 Corporate Park # 106
Irvine CA ZIP 92714
PHONE (714) 752-9333

OWNER/TENANT
NAME CKE Region #3
ADDRESS 384 North Fwy Blvd.
Sacramento ZIP 95834
PHONE (916) 561-4700

WILL THE PERMITEE HAVE ANY EMPLOYEE'S ON THE JOBSITE? YES NO
 NATURE OF WORK IN DETAIL: construction of a new Carl's Jr. Restaurant

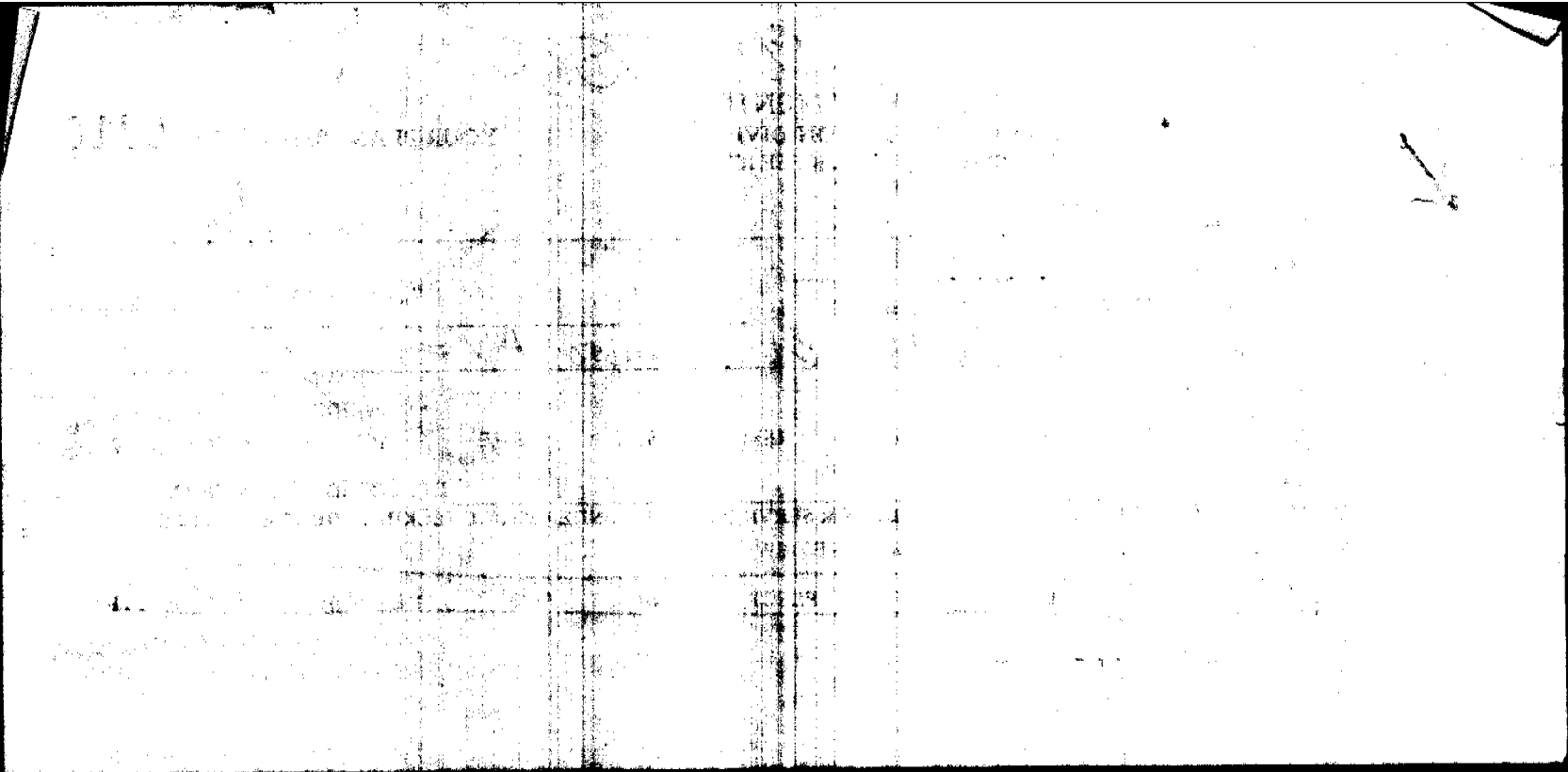
D.B.A. Carl's Jr. VALUATION 226,767.50
BELOW THIS LINE FOR BLDG. DEPT. USE ONLY

FLOOD STATUS None S.C.A.T. X-11, 12, 13

JOB DESCR. BLDG SHEL APT TI () REM () SW FIRE ADD OTH
INSP. DISCIPLINES BLDG MECH PLUMB ELEC SITE FIRE

# OF STORIES	AREA 1ST FL.	TOTAL AREA	USE ZONE	OCCUP. GROUP	CONST. TYPE	FIRE SPRINK.	FED CODE	VIO. FILE
1	3050	3050	SC-PAD	A-3	IN	uo	18	
	<u>B</u>	<u>L</u>	<u>P</u>	<u>M</u>	<u>E</u>	<u>F</u>	<u>S</u>	<u>D</u>

COMMENTS:
- contact H. Richmond - (called in - 1-30-98)
- Health Dept Receipt enclosed
- All fees need to be recalculated



RECEIPT
 ENVIRONMENTAL MANAGEMENT DEPARTMENT
 ENVIRONMENTAL HEALTH DIVISION

RECEIVED FROM: Paul Kwickler
 ADDRESS: 1200 NW Harbor DATE: 1/27/98
 AMOUNT RECEIVED: \$ 883 CHECK NO.: _____
 FACILITY NAME: Paul Kwickler CASH
 FAC. ADDRESS: Environmental Health Division, 1100 81 Avenue

REPT 03 ENVR 01/27/98
 15 1684 01/27/98
 RECEIPT 000007 001
 2001.00

REVENUE DESCRIPTION: (KEY 33)
 PLAN CHECK - FOOD
 PLAN CHECK - POOLS
 PUBLIC POOL FEE (CONSTRUCTION INSPECTIONS)
 PLAN CHECK - NOISE

INDEX	REV DETAIL	PCA	AMOUNT
6202	8817	32142	\$
6202	8818	32142	\$
6202	8710	32131	\$
6202	8823	32233	\$

SIGNATURE: [Signature]

City of Sacramento
Water and Sewer Service Quotation

Date: 04-10-1998 Time: 11:25 hrs Building Permit No.: 898-08 Plan Check No.: 5710
 Address: 2460 W EL CAMINO AVE Parcel No.: 225-1010-012
 Description: CARLS JR
 Subdivision Map: GATEWAY OAKS CENTER Water Plan No.: 51-1840
 Estimate by: FAVILLA Bldg. Insp. Reviewer: SPROSS
 Engineering Firm: TRUXAW & ASSOC
 Sewer Jurisdiction: County S.D. #1
 Comment No. 1 - EXISTING 2in TAP PCL E WATER PALN 51-1840

TOTAL WATER DEV. FEES: ~~\$8,428.43~~ TOTAL ON-SITE GRADING AND DRAINAGE REVIEW FEE: ^{1,050.00} \$1,350.00
 TOTAL SEWER DEV. FEES: \$0.00 ^{300.00}

Water Services Quotations

Main Service Size	Service Size	Description	Qty	Tap Fee-ea.	Meter Fee-ea.	Tot. Tap Cost	Dev. Fees	
12 in	2 in	Meter Only	1	\$0.00	\$657.00	\$657.00		
	2 in	Dev. Fee Only	1	\$0.00			\$5,872.0	
Total for Water:						\$657.00		
Parcel Area: .835 acres							Acresage Charge:	\$2,553.4

1,050.00
300.00
Charged to bldg perm.

Sewer Services Quotations

Main Service Size	Service Size	Description	Qty	St. (ft)	MH Fee/ea.	Tap Fee/ft.	Total Cost	Dev. Fees	
NOTE: TOT. COST=QNTY X ST/2 X TAP FEE + MH FEE								Total for Sewer:	\$0.00

Water Main Construction Charge: \$0.00
 Total for Address: \$657.00

WATER DEVELOPMENT FEES ARE BASED ON THE SIZE OF DOMESTIC SERVICE.
 TOTAL WATER DEVELOPMENT FOR COMMERCIAL PROPERTY INCLUDES A \$3,058.00 PER ACRE CHARGE IN ADDITION TO THE STANDARD FEE.

SRCSD

FACSIMILE TRANSMITTAL: PLEASE

DELIVER IMMEDIATELY

NUMBER OF PAGES: one

DATE: January 30, 1998

RECEIVING FAX : 264-7046

SENDING FAX : (916) 875-6253

TO: **Greg Johnson**
CITY OF SACRAMENTO

FROM: **CLAIR DAUGHERTY** PHONE NUMBER: (916) 875-6369
CUSTOMER SERVICE, WATER QUALITY DIVISION ENGINEERING

SUBJECT: **SEWER FACILITY IMPACT FEES** 2460 W EL CAMINO AVE.
APN: 225-1010-012

Plan Check - 5710

The following Sewer Facility Impact fees are for a Carl's Jr. on the above site.

Fee Paid prior to March 1st, 1998 On or After March 1st

Inspection Fee	\$0	\$0
CSD-1 Fee	\$468	\$552
SRCSD Fee	\$32,741	\$33,487
Construction	\$0	\$0
In-lieu Fee	\$0	\$0
	<hr/>	<hr/>
	\$33,209	\$34,039

cc: **Dolores**

Jan 30 '98 17:25 P. 02/03

9168756253
Fax: 9168756253

WATER QUALITY ENG

ADDRESS: 2400 W. 20th Avenue

SPECIAL CONDITIONS ATTACHMENT

PROJECT TITLE

PERMIT #

PC 1 5710

Minimum Monitoring Plan

Yes

No

Page 1 of 1

Timing Range

PERMIT ISSUANCE

Start

Item #	Description	Orig. Dept.	Timing	Follow up Req.	Verification Due Date	Code	Remarks
X1	Special inspections in accordance with section 106 of the U.B.C. (all)	SPI	Prior to final, C of C, or C of O				
X2	Verify floor elevation taking per elevation certificate.	LFA	Prior to foundation inspection	BLI			Inspection required not necessary
X3	A survey/engineer shall re-survey the finished floor elevation and provide an elevation certificate based on finished floor elevation.	LFA	Prior to framing inspection	SCU			(1)
X4	Life Safety testing is required for highrises	CPC	Prior to final, C of C, or C of O	EU FRI			(1), (2)
X5	Smoke control/Alarm system testing is required for malls and atriums	CPC	Prior to final, C of C, or C of O	EU FRI			(1), (2)
X6	Verify that the C of C has been issued	CPC	Prior to final, C of C, or C of O	FLC			(1), (2)
X7	Site improvement/24 houring	SCU	Prior to final, C of C, or C of O	SCU			(1)
X8	Site improvement/24 houring	SCU	Prior to final, C of C, or C of O	SCU			(1)
X9	Special permit conditions	GRP	Prior to final, C of C, or C of O	GRP			(1)
X10	Noise and Dust Abatement Program	ESD	Random	BLI			(1) 264-7720
X11	Archaeological Findings	ESD	Verify all foundation inspection	BLI			Random inspections will be arranged by Building Inspections Division
X12	Design Review	GRP	Prior to final, C of C, or C of O	GRP			Notify building inspector at foundation inspection
X13	Haban system	FRI	Prior to final, C of C, or C of O	FRI			(1)
X14	Kitchen hood fire suppression system	FRI	Prior to final, C of C, or C of O	FRI			(2)
X15	Fire alarm	FRI	Prior to final, C of C, or C of O	FRI			(2)
X16	Medical Gas Lines	FRI	Prior to final, C of C, or C of O	FRI			(2)
X17	Underground fuel tanks	FRI	Prior to covering with sheet rock	FRI			(2)
X18	Underground fuel tanks	FRI	Prior to covering with sheet rock	FRI			(2)
X19	Underground fire sprinkler mains	FRI	Prior to covering mains	FRI			(2)
X20	Inverted fire sprinkler system	FRI	Prior to final, C of C, or C of O	FRI			(2)

Approved: _____
 Construction Name: _____
 1 - 1 checked by owner or builder by phone
 4 - Posting/Sign work site posted
 5 - Project ready for inspection
 6 - Violation posted on sign
 7 - permit closed w/o and inspection
 Attachment A

PLAN CHECK ROUTING PROCEDURE

Date Received: _____ Plan Check #: 5710
 Project: 2460 W EL CAMINO AV
 Address: _____
 Legal Description: 225-1010-012 Fire Zone: 363-4164
 Contractor: LINDA BUOGE Telephone: _____
 Address: _____ City License: _____
 Architect: _____ Telephone: _____

PUBLIC WORKS - ENGINEERING TRANSPORTATION:
 927 - 10th Street, Room 100, Ron Perry

CIVIL ENGINEERING Date Received: _____
 Approved: P. Failla 4-10-98 Disapproved _____
 Total frontage length of New Street Improvements: _____ LF
 Comments: _____

Right of Way Dedication : Approved _____ Disapprove _____
 Public Improvement Agreement: Approved _____ Disapprove _____
 Surety Bond, etc. : Approved _____ Disapprove _____
 Staking and Inspection Fee : \$ _____

TRAFFIC ENGINEERING
 927 - 10th Street, Room 100, Paul Favilla

Approved: P. Failla Date Received: _____
 Disapproved _____
 Need new driveway permit No
 No driveway permit needed _____
 Removal of abandoned driveway _____
 Comments: EXISTING DWY

PUBLIC WORKS - WATER & SEWER, PLANNING, ARCH, REVIEW COMMITTEE,
 927 - 10th Street, Room 100, Ron Perry

Approved: P. Failla Date Received: _____
 Disapproved _____
 Comments: 20 meter ONLY

SITE CONDITIONS UNIT (264-7619)
 Steve Reed, Gary Spross, Wes Jigour

Approved _____ Date Received: _____
 Approved with Changes _____ Disapproved _____
 Review Zone: _____ Special Permit: _____ Variances: _____
 Parking Spaces Furnished: _____ Parking Spaces Required: _____
 Comments: _____

ARCHITECTURAL ADVISORY COMMITTEE
 (264-5604) Dick Hastings

Date Received: _____
 Is property located in a Civic Improvement District _____
 Meeting Approved _____ Approved with Changes _____ Disapproved _____
 Item# _____ Comments _____
 P# _____

**CITY OF SACRAMENTO
CASHIER'S WORKSHEET**

RECEIPT NUMBER: R9807622
 TRANSACTION DATE: 7/15/98
 TRANSACTION AMOUNT: 85.00
 NOTATION:

APD #: **9800758**
 SITE ADDRESS: 2460 WEST EL CAMINO AV SAC
 PARCEL: 225-1010-012

TYPE: Bldg Commercial
 SUB-TYPE: NCOM
 HOUSING: N
 STATUS: ISSUED

TRANSACTION LIST

Type	Method	Description	Pymt Amount
Payment	Cash		85.00

RECEIPT ACCOUNT ITEM LIST

Item #	Description	Class #	Total Fee	Prev Pymt	Current Pymt
1100	Permit--Building	200/201/222	2,397.46	2,397.46	.00
1120	Permit--Inspections	200/201/244	85.00	.00	85.00
1200	Plan Ck--Building Com	205/225/266	1,961.11	1,961.11	.00
1500	Process--Flood	219	17.00	17.00	.00
1510	Process--Addressing	219	17.00	17.00	.00
1520	Process--Haz Material	219	17.00	17.00	.00
1530	Process--School Impact	219	17.00	17.00	.00
1540	Review--Engineering	217	300.00	300.00	.00
1560	Review--Landscape	263	50.00	50.00	.00
1570	Review--Grading	221	150.00	150.00	.00
1600	Strong Motion (SMI)	207	41.41	41.41	.00
1710	Construction Excise Tax	208	1,577.71	1,577.71	.00
1730	City Business Oper Tax	206	118.88	118.88	.00
1750	Technology Surcharge	259	174.34	174.34	.00
1940	Hsg Trust Fund-Regular	246	2,464.24	2,464.24	.00
1960	SAFCA CIEF Fee	277	498.95	498.95	.00
2000	FBA--South Natomas	299	11,490.56	11,490.56	.00

CITY OF SACRAMENTO
ELECTRICAL INSPECTION DIVISION
1231 - I Street, Room 200
Sacramento, CA 95814

36444C

REQUEST FOR TEMPORARY ELECTRIC SERVICE

Job Address: 2400 West EL Camino

Permit Number: 9800758

Occupancy: A3

Issue Date: 7/7/98 Expiration Date: 8/7/98

Temporary electric service is requested for the above address for a period of 30 days for the following purpose(s) only:

TESTING OF ELECTRICAL

It is understood and agreed that the granting of this request for temporary electric service does not constitute approval to occupy or use the building except for the purpose(s) stated above. It is further understood and agreed that if final approval of the building is not granted by the City of Sacramento Building Inspection Division in the number of days specified above, or if the provisions of the National Electrical Code, Local ordinances and all applicable codes and regulations are not complied with the temporary electric service will be disconnected without further notice given by the City.

Owner's Signature _____ Phone No. _____ Date _____
Douglas Van Dyke 916 464 0801 7/7/98
Contractor's (or Agent's) _____ Phone No. _____ Date _____

7026 Turtle Cove Way, Elk Grove, Ca. 95758
Address

This Division will not authorize temporary electric service unless all required permits have been obtained and the following has been inspected and approved:

- 1- All related service equipment.
- 2- All wiring, panelboards, devices, etc. that may be energized.

Approved by: D. Verga Date: 7/7/98

William F. Clark, Chief Electrical Inspector

Electrical Inspections 449-5671

Building Inspections 449-5716

White - Office Copy

Yellow - Smud Copy

Pink - Owner Copy

CITY OF SACRAMENTO
1231 I Street, Sacramento, CA 95814

FEE SUMMARY
FOR PERMIT #9803449
as of 04-28-1998 Permit Status: **APPLIED**

Site Address: **2460 WEST EL CAMINO AV SAC**
Parcel No: **2251010012**

CONTRACTOR

Phone:

OWNER

TEACHERS RET SYSTEM STATE OF
1201 N CLARK ST 300
CHICAGO IL 60610
Phone:

ARCHITECT

Phone:

Nature of Work: FOUNDATION FOR NEW CARL'S JR RESTAURANT

Permit Valuation: **\$30,000.00**
Square Footage: **3056**

Building Permit.....:	\$511.00	Water Development Fee:	\$8,425.43
Plan Review/Processing:	\$731.60	Sewer Development Fee:	\$0.00
Strong Motion Fee.....:	\$6.30	Regional Sanitation Fee:	\$0.00
Coach Recording Fee...:	\$0.00	Bell Avenue Sewer.....:	\$0.00
Manuf Housing Fee.....:	\$0.00	Granite Park Fee.....:	\$0.00
Auth to Start Work.....:	\$0.00	Pocket Area Bridge.....:	\$0.00
Penalty Fee.....:	\$0.00	Pocket Area Road.....:	\$0.00
Inspections.....:	\$0.00	Quimby Park Fee.....:	\$0.00
Cert of Occupancy.....:	\$0.00	Housing Trust Fund.....:	\$0.00
Replace Cards/Plans....:	\$0.00	Natomas Dev Fees.....:	\$0.00
Hsg Process/Surcharge:	\$0.00	FBA-South Natomas....:	\$0.00
Technology Fee.....:	\$49.70	FBA-Jacinto Creek.....:	\$0.00
City Bus Oper Tax.....:	\$12.00	Amount Deferred.....:	\$0.00
Const Excise Tax.....:	\$240.00	Refund.....:	\$0.00
Res Const Tax.....:	\$0.00		
Processing Fees.....:	\$0.00		
Review Fees.....:	\$1,050.00		
		Subtotal.....:	\$11,026.03
		Additional Fees.....:	\$0.00
		TOTAL FEES.....:	\$11,026.03
		Payments.....:	\$11,026.03
		BALANCE DUE.....:	\$0.00

SPECIAL CONDITIONS ATTACHMENT

ADDRESS 2640 West El Camino PERMIT # 98-00758 Mitigation Monitoring Plan Yes No Pg. 1 of 3

PROJECT TITLE _____ P.C. # _____ Timing Range _____ PERMIT ISSUANCE _____ thru _____

Item #	Description	Orig. Dept.	Timing	Follow Up Resp.	Verification Date/Int.	Code	Remarks
X1	Special inspections in accordance with section 306 of the U.D.C. (all)	SPI	Prior to final, C of C, or C of O	SPI			Inspection request not necessary
X2	Verify floor elevation staking per elevation certificate.	LFA	Prior to foundation inspection	BLI			(1)
X3	A surveyor/engineer shall re-survey the finished floor elevation and provide an elevation certificate based on finished floor elevation.	LFA	Prior to framing inspection	SCU			(1)
X4	Life-Safety testing is required for highrises	CPC	Prior to final, C of C, or C of O	ELI FRI			(1),(2)
X5	Smoke control/Alarm systems testing is required for malls and atriums	CPC	Prior to final, C of C, or C of O	ELI FRI			(1),(2)
X6	Verify that the C of C has been issued	CPC	Prior to final, C of C, or C of O	FLC			(1)
X11	Site improvements/T-24 handicap	SCU	Prior to final, C of C, or C of O	SCU			(1)
X12	Site improvements/zoning code	SCU	Prior to final, C of C, or C of O	SCU			(1)
X13	Special permit conditions	CRP	Prior to final, C of C, or C of O	CRP			(1)
X14	Noise and Dust Abatement Program	ESD	Random	BLI BLI			Random inspections will be arranged by Building Inspections Division
X15	Archaeological Findings	ESD	Verify at foundation inspection	BLI			Notify building inspector at foundation inspection
X16	Design Review	CRP	Prior to final, C of C, or C of O	CRP			(1)
X24	Halon system	FRI	Prior to final, C of C or C of O	FRI			(2)
X25	Kitchen hood fire suppression system	FRI	Prior to final, C of C or C of O	FRI			(2)
X26	Fire alarm	FRI	Prior to final, C of C or C of O	FRI			(2)
X27	Medical Gas Lines	FRI	Prior to covering with sheet rock	FRI			(2)
X28	Underground fuel tanks	FRI	Prior to covering tanks	FRI			(2)
X29	Underground fire sprinkler mains	FRI	Prior to covering mains	FRI			(2)
X30	Overhead fire sprinkler system	FRI	Prior to final, C of C, or C of O	FRI			(2)

Codes
 1 = Approved
 2 = Correction Notice
 3 = Canceled by owner or builder by phone
 4 = Posting/Stop work etc. posted
 5 = Project not ready for inspection
 6 = Violation posted or set
 7 = permit closed w/o and inspection

(1) Call 264-5191 for inspections
 (2) Call 264-5191 for inspections

Attachment A

2-WK TCO

73.34

Date: 7/15/98

REQUEST FOR CERTIFICATE OF OCCUPANCY

- Final Certificate of Occupancy
- Temporary Certificate of Occupancy*
- Extension - Temporary Certificate of Occupancy expires _____
- Certificate of Occupancy Questions

Project Address: 2400 West El Camino, Sacramento

Type of Project (Remodel, etc., New Construction, etc.): NEW construction

Permit No(s): 08 00 758

Contact Person: Douglas Van Dyke or Jim Williams

Contact Phone No: 916 206 6341 or 916 920 3866 FAX 916 648 9936

Have ALL disciplines (INCLUDING Fire Dept. Requirements) been signed off by the inspector? Yes No N/A

*Temporary Certificate of Occupancy and extension requests are issued at the discretion of the Building Official. If it is a request for a temporary Certificate of Occupancy or an extension of a Temporary Certificate of Occupancy, explain the reason below.

all disciplines have noted that they are OK for a temp certificate of occupancy because there are no significant items remaining - our training program for employees needs to begin this is the need for the temp. coz.

COMMENTS: OK 7-16-98 [Signature]

ALL INFORMATION ON THIS FORM IS CONFIDENTIAL AND NOT TO BE RELEASED TO THE PUBLIC

CITY OF SACRAMENTO
1231 I ST. ROOM 200
BUILDING INSPECTIONS DIVISION

PERMIT NO.

98-00758

AREA NO.

40

CORRECTION NOTICE

WHEN CORRECTIONS HAVE BEEN MADE, CALL 264-5191 FOR REINSPECTION OF WORK.

JOB LOCATION 2460 W El Camino Ave

INSPECTION REQUESTED Final

THE UNDERSIGNED BUILDING PLUMBING MECHANICAL ELECTRICAL
INSPECTOR THIS DAY INSPECTED THIS STRUCTURE FOR THE REQUESTED INSPECTION AND FOUND THE FOLLOWING VIOLATIONS OF CITY AND/OR STATE LAWS GOVERNING SAME:

Plumbing O.K.

Mechanical requires outside
air intake.

O.K. for Temp Occ.
for Straining & Stocking

INSPECTOR Tim N. Jan

DATE 7/16/98

BUILDING INSPECTIONS 264-5716

JOB COPY

DO NOT REMOVE THIS TAG

PERMIT NO.

9800758

AREA NO.

4C

CORRECTION NOTICE

WHEN CORRECTIONS HAVE BEEN MADE, CALL 264-5191 FOR REINSPECTION OF WORK.

JOB LOCATION 2400 West Fl Camino Ave

INSPECTION REQUESTED Final

THE UNDERSIGNED BUILDING PLUMBING MECHANICAL ELECTRICAL
INSPECTOR THIS DAY INSPECTED THIS STRUCTURE FOR THE REQUESTED INSPECTION AND FOUND THE FOLLOWING VIOLATIONS OF CITY AND/OR STATE LAWS GOVERNING SAME:

- ① Previous Item #2 Signs not connected
- ② Previous Item #8 Sinal Case to be hard wired as per Plan + Note 46
- ③ It would be OK to apply for training of 0

INSPECTOR

[Signature]

DATE

7/15/98

BUILDING INSPECTIONS 264-5716

JOB COPY

DO NOT REMOVE THIS TAG

PERMIT NO.

98 00758 e

CITY OF SACRAMENTO
1231 I ST. ROOM 200
BUILDING INSPECTIONS DIVISION

AREA NO.

4

CORRECTION NOTICE

WHEN CORRECTIONS HAVE BEEN MADE, CALL 264-5191 FOR REINSPECTION OF WORK.

JOB LOCATION 2460 W. EL Camino

INSPECTION REQUESTED FINAL

THE UNDERSIGNED BUILDING PLUMBING MECHANICAL ELECTRICAL
INSPECTOR THIS DAY INSPECTED THIS STRUCTURE FOR THE REQUESTED INSPECTION AND FOUND THE
FOLLOWING VIOLATIONS OF CITY AND/OR STATE LAWS GOVERNING SAME:

F 01192

- ① Provide Ramps for Handicapped
- ② Provide Handicap Signs on Toilet room Doors
- ③ Completely Special Condition site work inspection

INSPECTOR D. H.

DATE 7/13/98

BUILDING INSPECTIONS 264-5716

JOB COPY

DO NOT REMOVE THIS TAG

PERMIT NO
9800758C

AREA NO.
4C

CORRECTION NOTICE

WHEN CORRECTIONS HAVE BEEN MADE CALL 264-5191 FOR REINSPECTION OF WORK.

JOB LOCATION 2460 West El Camino

INSPECTION REQUESTED Site insp.

THE UNDERSIGNED BUILDING PLUMBING MECHANICAL ELECTRICAL
INSPECTOR THIS DAY INSPECTED THIS STRUCTURE FOR THE REQUESTED INSPECTION AND FOUND THE FOLLOWING VIOLATIONS OF CITY AND/OR STATE LAWS GOVERNING SAME: **D 17050**

1. Provide 20' long concrete apron in front of trash enclosure.
2. Provide 2" shredded mulch in all planters. Provide sample.

phone 264-7720

OK for temp C of O.

INSPECTOR Greg R. Spess

DATE 7/14/98

BUILDING INSPECTOR 264-5716
ELECTRICAL INSPECTOR 264-5671

PLUMBING INSPECTOR 264-5661
MECHANICAL INSPECTOR 264-5661

JOB COPY

DO NOT REMOVE THIS TAG

POST THIS CARD IN A CONSPICUOUS PLACE!

SACRAMENTO CITY FIRE DEPARTMENT

1231 I STREET, SUITE 401
SACRAMENTO, CA 95814-2979

FIRE PREVENTION DIVISION
INSPECTION SERVICES

FOR INSPECTIONS CALL (916) 264-5400 **5791**
NOTICE OF 48 HOURS REQUIRED FOR INSPECTIONS

PERMIT # 9805580 APPROVED BY [Signature]
PROJECT Kitchen Hood
ADDRESS 2460 W. El Camino
OWNER _____

NOTE

- 1) DO NOT COVER WALLS, CEILINGS OR PIPING UNTIL THE FOLLOWING ITEMS ARE SIGNED OFF.
- 2) ALL-WEATHER EMERGENCY ACCESS ROADWAYS AND FIRE HYDRANTS (IN SERVICE) SHALL BE PROVIDED PRIOR TO ANY COMBUSTIBLE CONSTRUCTION OR STORAGE ON SITE.

INSPECTIONS	SITE	INITIALS	DATE
→ UNDERGROUND WATER MAINS/THRUST BLOCKS			
→ HYDROSTATIC TEST OF WATER MAINS			
→ FLUSHING OF WATER MAINS			
→ REFLECTIVE MARKERS			
→ PAINTING OF EQUIPMENT			
→ MARKING OF FIRE LANES			
→ ADDRESS POSTED			
→ EMERGENCY ACCESS KEY BOX (KNOX)			
→ FIRE HYDRANTS			

FIRE & LIFE SAFETY

EXTING SYSTEMS		
FIRE DOORS		
SMOKE VENTING		
HIGH PILED STOCK		
FLAMMABLE LIQUIDS		
HAZARDOUS MATERIALS		
SPECIAL HAZARDS		
INTERIOR FINISH		
POSTED SIGNS FOR OCCUPANT LOAD		

EQUIPMENT

→ FIRE SPRINKLER WELDED OUTLETS		
→ FIRE SPRINKLER SYSTEM PIPING		
→ FIRE SPRINKLER HYDROSTATIC TEST		
→ STANDPIPES		
→ FIRE ALARMS/TESTING CONTRACT		
KITCHEN HOOD & DUCT SYSTEM		
SPECIAL EXTINGUISHING SYSTEM		
FIRE EXTINGUISHERS		
PRIVATE WATER SYSTEM		

SPECIAL REQUIREMENTS

FINAL APPROVAL

APPROVED FIRE FLOW [Signature]
OCCUPANCY GRANTED/F.D. APPROVAL [Signature]

NOTICE:
FAILURE TO COMPLY WITH AN ORDER OF THE FIRE DEPARTMENT MAY RESULT IN THE
ISSUANCE OF A CITATION AND/OR DISCONTINUED USE OF THE BUILDING OR PREMISES.
1994 UPC SECTIONS 103.4.3.1, 103.4.3.2, 103.4.4.

KEEP THIS CARD FOR REFERENCE
THIS IS YOUR RECORD OF FIELD INSPECTIONS



FIELD TEST

ORDER DATE
July 16, 1998

Grill Hood

CUSTOMER INFO		SERVICE INFO	
NAME <i>Con's Jr.</i>	DATE <i>7-16-98</i>	<input checked="" type="checkbox"/> AM <input type="checkbox"/> PM	TECHNICIAN <i>Gary Meloy</i>
ADDRESS <i>2960 W. El Camino</i>	CITY <i>San Jose</i>	TYPE OF SYSTEM <i>Amul R-102 3 Gal.</i>	
STATE <i>CA</i>	ZIP <i>95131</i>	SYSTEM SERIAL NO. <i>R 460332</i>	
CITY <i>San Jose</i>	PHONE <i>910 264-7120</i>	INSPECTOR NAME <i>Jim Kowitz</i>	

GENERAL SERVICE INSTRUCTIONS

Bulb test on Amul R-102 3 Gallon System

New Install

APPROVALS

DATE: *7-16-98*

INSPECTOR: *[Signature]*

TECHNICIAN: *[Signature]*

DATE: *7-16-98*

TECHNICIAN: *[Signature]*

FIELD TEST

ORDER DATE
July 16, 98

FRYER HOOD

CUSTOMER INFO		SERVICE INFO	
NAME	Carl S Jr.	DATE	7-16-98
ADDRESS	2460 W 21 CANNING	TIME	<input checked="" type="checkbox"/> AM <input type="checkbox"/> PM
CITY	San Jose	TECHNICIAN	Gary Melby
STATE	CA	DESCRIPTION	Inst R-102-3 Caller
ZIP	95033	PHONE	R 460326
CITY	()	TECHNICIAN	Jim Krentz
		PHONE	916 264-7125

COMMERCIAL INSTRUCTIONS

Ballen Test ANSI R-102-3 Caller System
New install

APPROVALS		DATE
TECHNICIAN	<i>[Signature]</i>	7-16-98
OWNER	<i>[Signature]</i>	7-16-98

WHITE -- Job Order CANARY -- Branch PINK -- Customer

FIELD TEST

ORDER DATE
July 16, 98

Brooks Hood

CUSTOMER INFO		SERVICE INFO	
NAME <i>Curtis Jr.</i>	DATE <i>7-16-98</i>	<input checked="" type="checkbox"/> AM <input type="checkbox"/> PM	SERVICE MAN <i>Gary McKay</i>
ADDRESS <i>2460 W. EL CAMINO</i>	CITY <i>Antelope</i>	STATE <i>CA</i>	ZIP <i>95833</i>
CITY <i>Sacramento</i>	STATE <i>CA</i>	ZIP <i>95833</i>	PHONE <i>916 264-7120</i>
CONTACT <i>()</i>	PHONE <i>916 264-7120</i>		

COMMENTS/SPECIAL INSTRUCTIONS

*Baller test on Ansul R-102-3 Guller system.
New Trussell.*

APPROVALS

DATE	BY	DATE	BY
<i>7-16-98</i>	<i>[Signature]</i>	<i>7-16-98</i>	<i>[Signature]</i>

WHITE - Job Field CASHIER GIBSON () - Customer



International Fire Equipment Co.

133 OTTO CIRCLE • SACRAMENTO, CA 95822

FAX COVER LETTER

DATE: July 17, 1998

TOTAL PAGES SENT INCLUDING THIS COVER SHEET: 6

FROM: Nikkie

TO: City of Sacramento Fire Department

FAX #: (916) 264-7046

ATTN: Val Brown

Approved Deferred

2460 W. El Camino

98-00758

Submitted Truss Cables



WESTERN WOOD



FABRICATORS

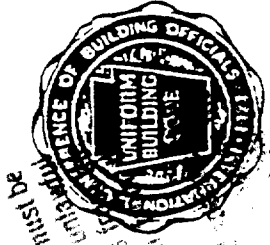
3700 RIEGO ROAD
ELVERTA, CA 95626
(916) 991-4400
(916) 991-4461 FAX

ENGINEERING DRAWING PACKAGE

CUSTOMER: CLARK CONTRACTORS
PROJECT: CARLS JR
PLAN: _____ ELEV: _____
SALESMAN: FRED _____ ENG TECH: ERIC C. _____

RECEIVED AT FINAL

RECEIVED



JUN 24 1998

NOTE: ALL LATERAL BRACING SPECIFIED IS FOR BRACING COMPRESSION MEMBERS AND MUST BE INSTALLED. TOP CHORDS ARE ASSUMED TO BE INHERENTLY RESTRAINED BY PLYWOOD OR SPACED SHEATHING. WHERE RIGID CEILING IS APPLIED DIRECTLY TO THE BOTTOM CHORDS, BRACING SHALL BE BRACED AT INTERVALS NOT EXCEEDING 10'-0"

NOTE: VERIFICATION OF LOADING, DEFLECTION LIMITATIONS, FRAMING METHODS, WIND BRACING OR OTHER LATERAL BRACING THAT IS ALWAYS REQUIRED, IS THE RESPONSIBILITY OF THE PROJECT ARCHITECT OR ENGINEER.

NOTE: ALL FLOOR TRUSSES RECOMMEND 2X6 STRONGBACKS AT 10'-0" O.C.

NOTE: DUE TO THE VARIATIONS IN WEATHER, LUMBER DIMENSIONS AND MOISTURE CONTENT, AT THE TIME OF FABRICATION WESTERN WOOD FABRICATORS CANNOT BE RESPONSIBLE FOR ANY TRUSS DIMENSION VARIANCE OF + - 1/4" OR LESS

WARNING
(DO NOT CUT OR ALTER TRUSSES)



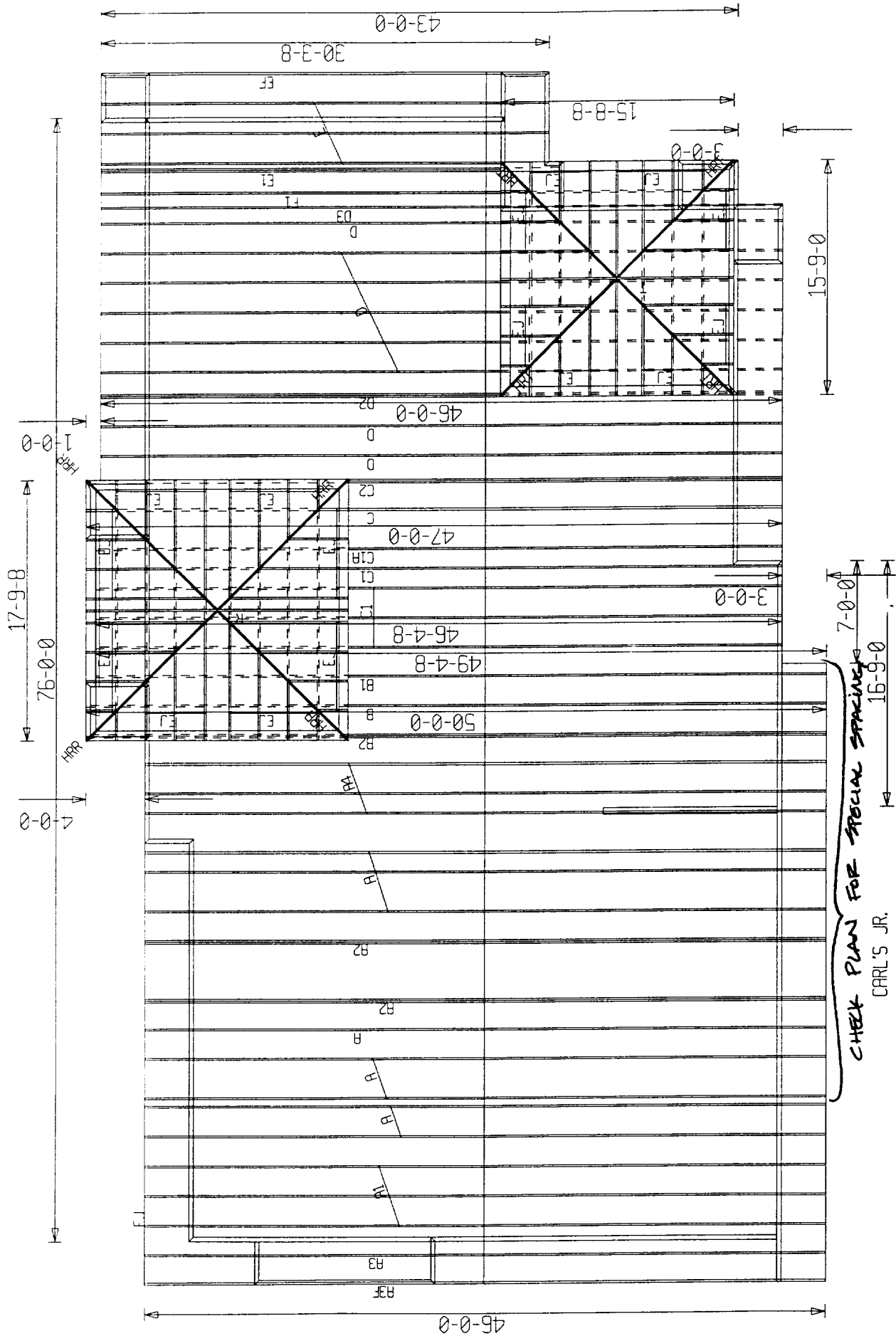
TRUSS PLATE INSTITUTE

F:\APPS\FORHTOOL\AMF\DRGPACK



WESTERN WOOD
CERTIFIED INSPECTION
IN STRICT ACCORDANCE
WITH U.B.C. 2311.6
PREFABRICATED





CHECK PLAN FOR SPECIAL BRACING

CARL'S JR.
 PLAN #140C
 SACRAMENTO, CA.
 5/13/98
 #J8140

Job Name: CARLS JR. 14DC

Truss ID: A

Drwg: A

ERG	X-LOC	PERCT	SIZE	REV'D	TOP CHRD	2x6 FL #2	2x4 EL #2	2-14, 10-20
1	3-1-12	2007	3-50"	2-14"	WEB			
2	42-10-4	1861	3-50"	1-98"				

Plating spec : ANSI/TPI - 1995
 THIS DESIGN IS THE COMPOSITE RESULT OF MULTIPLE LOAD CASES.
 PLATE VALUES PER ICBO RESEARCH REPORT #16007.
 Loaded for 10 PSF non-concurrent B.L.L.
 Permanent bracing is required (by others) to prevent rotation/toppling. See HIB-91 and ANSI/TPI 1-1995, 10-3.4.5 and 10-3.4.6.
 Drainage must be provided to avoid ponding.
 FINISHES BASED ON GREEN LUMBER VALUES.
 End verticals that are extended above or below the truss profile (if any) may require additional design consideration (by others) for lateral forces due to wind or seismic loads on the building.

ERG	X-LOC	PERCT	SIZE	REV'D	TOP CHRD	2x6 FL #2	2x4 EL #2	2-14, 10-20
1	3-1-12	2007	3-50"	2-14"	WEB			
2	42-10-4	1861	3-50"	1-98"				

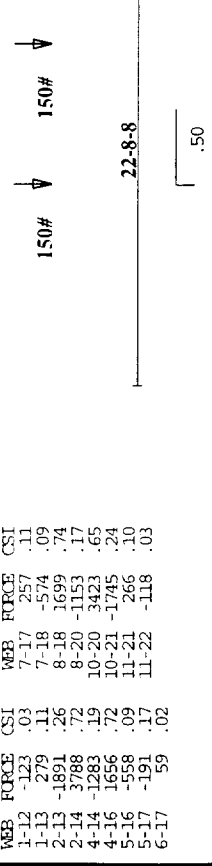
TC	FORCE	AXL	END	CSI	MAX DEFLECTION (span)
1	-229	.00	.36	.36	L/949 IN MEM 16-17 (LIVE)
2	-3981	.14	.36	.51	L = .50" D = .32" T = .82"
3	-3965	.15	.18	.33	MAX DEFLECTION (cant)
4	-5718	.32	.32	.64	L/351 IN MEM 12-13 (LIVE)
5	-5649	.31	.34	.65	L = .10" D = .07" T = .17"
6	-5649	.31	.33	.64	
7	-5362	.28	.30	.58	
8	-3594	.12	.20	.32	
9	-3610	.12	.35	.46	
10	-218	.00	.35	.35	

TC	FORCE	AXL	END	CSI
1	0	.00	.21	.21
2	428	.04	.19	.23
3	4216	.58	.15	.73
4	4216	.58	.14	.72
5	5788	.80	.12	.92
6	5435	.75	.11	.86
7	3820	.52	.12	.65
8	3820	.52	.14	.66
9	401	.04	.17	.21
10	0	.00	.19	.19

WEB	FORCE	CSI	WEB	FORCE	CSI
1-12	-123	.03	7-17	257	.11
1-13	279	.11	7-18	-574	.09
2-13	-1891	.26	8-18	1699	.74
2-14	3788	.72	8-20	-1153	.17
4-14	-1283	.19	10-20	3423	.65
4-16	1656	.72	10-21	-1745	.24
5-16	-558	.09	11-21	266	.10
5-17	-191	.17	11-22	-118	.03
6-17	59				

Joint Locations	1	2	3	4	5	6	7	8	9	10	11
0-0-0	12	0	0	0	0	0	0	0	0	0	0
1-12	13	3	1-12	14	9	9	3	3	9	9	3
9-0-6	15	14	14	14	14	14	14	14	14	14	14
16-4-9	16	16	16	16	16	16	16	16	16	16	16
23-0-0	17	23	23	23	23	23	23	23	23	23	23
29-7-7	18	29	29	29	29	29	29	29	29	29	29
36-2-13	19	36	36	36	36	36	36	36	36	36	36
42-10-4	20	42	42	42	42	42	42	42	42	42	42
46-0-0	21	46	46	46	46	46	46	46	46	46	46
46-0-0	22	46	46	46	46	46	46	46	46	46	46

Joint Locations	1	2	3	4	5	6	7	8	9	10	11
0-0-0	12	0	0	0	0	0	0	0	0	0	0
1-12	13	3	1-12	14	9	9	3	3	9	9	3
9-0-6	15	14	14	14	14	14	14	14	14	14	14
16-4-9	16	16	16	16	16	16	16	16	16	16	16
23-0-0	17	23	23	23	23	23	23	23	23	23	23
29-7-7	18	29	29	29	29	29	29	29	29	29	29
36-2-13	19	36	36	36	36	36	36	36	36	36	36
42-10-4	20	42	42	42	42	42	42	42	42	42	42
46-0-0	21	46	46	46	46	46	46	46	46	46	46
46-0-0	22	46	46	46	46	46	46	46	46	46	46



MAX DEFLECTION (span) :
 L/949 IN MEM 16-17 (LIVE)
 L = .50" D = .32" T = .82"
 MAX DEFLECTION (cant) :
 L/351 IN MEM 12-13 (LIVE)
 L = .10" D = .07" T = .17"

Joint Locations
 1 0-0-0 12 0 0 0
 2 3 1-12 13 3 1-12
 3 9 0-6 14 9 9-3
 4 9 9-3 15 14 0-0
 5 16-4-9 16 16 16-4-9
 6 23-0-0 17 23-0-0
 7 29-7-7 18 29-7-7
 8 36-2-13 19 32-0-0
 9 36-11-10 20 36-2-13
 10 42-10-4 21 42-10-4
 11 46-0-0 22 46-0-0

Joint Locations
 1 0-0-0 12 0 0 0
 2 3 1-12 13 3 1-12
 3 9 0-6 14 9 9-3
 4 9 9-3 15 14 0-0
 5 16-4-9 16 16 16-4-9
 6 23-0-0 17 23-0-0
 7 29-7-7 18 29-7-7
 8 36-2-13 19 32-0-0
 9 36-11-10 20 36-2-13
 10 42-10-4 21 42-10-4
 11 46-0-0 22 46-0-0

Eng. Job:	.EJ.
Chk: CM	
Dsgnr: EC	
TC Live	20.0 psf
TC Dead	9.0 psf
BC Live	.0 psf
BC Dead	4.0 psf
TOTAL	33.0 psf

NO:	J8140
Date:	5/12/98
DurFace	L=1.15 P=1.15
Rep Mbr	Bnd 1.00
O.C.Spacing	2-4-0
Design Spec	UBC-94
Seqn	03.20.98-4428

Western Wood Fabricators
 3700 Riego Road, Elverta, CA 95626

WARNING Read all notes on this sheet and give a copy of it to the Erecting Contractor.
 This design is for an individual building component. It has been based on specifications provided by the component manufacturer and done in accordance with the current versions of TPI and AFPA design standards. No responsibility is assumed for dimensional accuracy. Dimensions are to be verified by the component manufacturer and/or building designer prior to fabrication. The building design shall ascertain that the loads utilized on this design meet or exceed the loading imposed by the local building code. It is assumed that the top chord is laterally braced by the roof or floor sheathing and the bottom chord is laterally braced by a rigid sheathing material directly attached, unless otherwise noted. Bracing shown is for lateral support of components members only to reduce buckling length. This component shall not be placed in any environment that will cause the moisture content of the wood exceed 19% and/or cause connector plate corrosion. Fabricate, handle, install and brace this truss in accordance with the following standards: "TRUSCOM MANUAL"; by Truswal, "QUALITY CONTROL STANDARD FOR METAL PLATE CONNECTED WOOD TRUSSES" - (HIB-88), "HANDLING INSTALLING AND BRACING METAL PLATE CONNECTED WOOD TRUSSES" - (HIB-91) and "HIB-91 SUMMARY SHEET" by TPI. The Truss Plate Institute (TPI) is located at 583 D'Onofrio Drive, Madison, Wisconsin 53719. The American Forest and Paper Association (AFPA) is located at 1250 Connecticut Ave, NW, Ste 200, Washington, DC 20036.

All plates are 20 gauge Truswal Connectors unless preceded by "18" for 18 gauge or "H" for 16 gauge.

Job Name: CARLS JR. 14DC

Truss ID: B

Drwg: B

FRG	X-LOC	REACT	SIZE	REQ'D	TOP CHFD	#2	EL	SS	6-9	13-16	2-15, 4-17	21-10, 23-12
1	3-1-12	2090	3.50"	2.23"								
2	49-10-4	1970	3.50"	2.10"								
TC	FORCE	AXL	END	CSI								
1	-220	.00	.31	.31								
2	-4362	.08	.31	.39								
3	-4348	.08	.12	.20								
4	-6956	.29	.30	.59								
5	-8113	.40	.31	.71								
6	-8114	.22	.13	.34								
7	-9259	.31	.57	.89								
8	-8313	.19	.15	.34								
9	-8327	.40	.26	.66								
10	-5495	.14	.07	.21								
11	-3337	.06	.16	.22								
BC	FORCE	AXL	END	CSI								
1	0	.00	.19	.19								
2	432	.04	.18	.22								
3	4627	.59	.14	.73								
4	4627	.34	.08	.42								
5	7081	.52	.08	.60								
6	9276	.69	.10	.79								
7	8492	.63	.12	.75								
8	8492	.63	.12	.75								
9	5722	.43	.10	.53								
10	3605	.27	.08	.35								
11	0	.00	.08	.08								

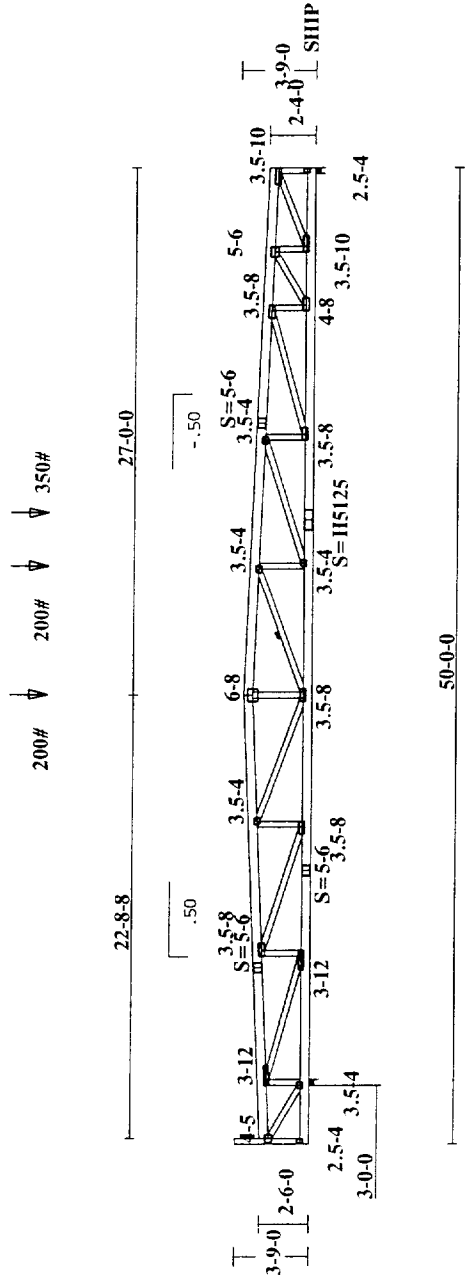
Plating spec: ANSI/TPI - 1995
 THIS DESIGN IS THE COMPOSITE RESULT OF MULTIPLE LOAD CASES.
 Loads for 10 PSF non-concurrent RLL.
 Permanent bracing is required (by others) to prevent rotation/torsion. See HB-91 and ANSI/TPI 1-1995; 10.3.4.1.5 and 10.3.4.1.6.

2x4 continuous lateral WEB bracing (L1B), nailed w/2-10d, located for equal segments, OR 1x4 w/1" brace nailed flat to edge of web w/ 8d at 8" o.c. OR a scab (same as web) nailed to face of web w/10d at 8" o.c. If 2 are required, attach w/1" to both edges or scab to both faces. If 3 or more are req., use CLB. w/1" or scab must be 90% of web length, and 2x6 if web is > 14'-0". Use stress-graded lumber & box or column nails. PLATE VALUES PER LCO RESEARCH REPORT #1607. DRAINAGE MUST BE PROVIDED TO AVOID PONDING. PLATING BASED ON GREEN LUMBER VALUES. END verticals that are extended above or below the truss profile (if any) may require additional design consideration (by others) for lateral forces due to wind or seismic loads on the building.

Dir	TC Vert	BC Vert	Type	Lbs	X Loc	L/TL
Dir	TC Vert	BC Vert	Type	Lbs	X Loc	L/TL
	58.0	8.0	0-0-0	58.0	50-0-0	.69
	200.0	200.0	29-7-7	200.0	29-7-7	.61
	350.0	350.0	32-4-4	350.0	32-4-4	.61

Joint Locations	1	2	3	4	5	6	7	8	9	10	11	12
1	0-0-0	13-0-0	14-3-1-12	15-9-9-3	16-14-0-0	17-16-4-9	18-23-0-0	19-29-7-7	20-36-2-13	21-42-10-4	22-48-10-4	23-45-10-4
2	0-0-0	13-0-0	14-3-1-12	15-9-9-3	16-14-0-0	17-16-4-9	18-23-0-0	19-29-7-7	20-36-2-13	21-42-10-4	22-48-10-4	23-45-10-4
3	0-0-0	13-0-0	14-3-1-12	15-9-9-3	16-14-0-0	17-16-4-9	18-23-0-0	19-29-7-7	20-36-2-13	21-42-10-4	22-48-10-4	23-45-10-4
4	0-0-0	13-0-0	14-3-1-12	15-9-9-3	16-14-0-0	17-16-4-9	18-23-0-0	19-29-7-7	20-36-2-13	21-42-10-4	22-48-10-4	23-45-10-4
5	0-0-0	13-0-0	14-3-1-12	15-9-9-3	16-14-0-0	17-16-4-9	18-23-0-0	19-29-7-7	20-36-2-13	21-42-10-4	22-48-10-4	23-45-10-4
6	0-0-0	13-0-0	14-3-1-12	15-9-9-3	16-14-0-0	17-16-4-9	18-23-0-0	19-29-7-7	20-36-2-13	21-42-10-4	22-48-10-4	23-45-10-4
7	0-0-0	13-0-0	14-3-1-12	15-9-9-3	16-14-0-0	17-16-4-9	18-23-0-0	19-29-7-7	20-36-2-13	21-42-10-4	22-48-10-4	23-45-10-4
8	0-0-0	13-0-0	14-3-1-12	15-9-9-3	16-14-0-0	17-16-4-9	18-23-0-0	19-29-7-7	20-36-2-13	21-42-10-4	22-48-10-4	23-45-10-4
9	0-0-0	13-0-0	14-3-1-12	15-9-9-3	16-14-0-0	17-16-4-9	18-23-0-0	19-29-7-7	20-36-2-13	21-42-10-4	22-48-10-4	23-45-10-4
10	0-0-0	13-0-0	14-3-1-12	15-9-9-3	16-14-0-0	17-16-4-9	18-23-0-0	19-29-7-7	20-36-2-13	21-42-10-4	22-48-10-4	23-45-10-4
11	0-0-0	13-0-0	14-3-1-12	15-9-9-3	16-14-0-0	17-16-4-9	18-23-0-0	19-29-7-7	20-36-2-13	21-42-10-4	22-48-10-4	23-45-10-4
12	0-0-0	13-0-0	14-3-1-12	15-9-9-3	16-14-0-0	17-16-4-9	18-23-0-0	19-29-7-7	20-36-2-13	21-42-10-4	22-48-10-4	23-45-10-4

MAX DEFLECTION (span) :
 L/623 IN MEM 18-19 (LIVE)
 L/F = .89" D = .58" T = 1.47"
 MAX DEFLECTION (cant) :
 L/235 IN MEM 13-14 (LIVE)
 L/F = .15" D = .10" T = .25"



All plates are 20 gauge Truswal Connectors unless preceded by "18" for 18 gauge or "H" for 16 gauge.

Scale: 3/32" = 1'

Eng. Job: .EJ.	WO: J8140
Chk: CM	Date: 5/13/98
Design: BC	Dur/Fac L=1.25 P=1.25
TC Live 20.0 psf	Rep Mbr Bnd 1.00
TC Dead 9.0 psf	O.C.-Spacing 2-0-0
BC Live .0 psf	Design Spec UBC
BC Dead 4.0 psf	
TOTAL 33.0 psf	Seqn 03.20.98- 4674

WARNING Read all notes on this sheet and give a copy of it to the Erecting Contractor.
 This design is for an individual building component. It has been based on specifications provided by the component manufacturer and done in accordance with the current versions of TPI and AFPA design standards. No responsibility is assumed for dimensional accuracy. Dimensions are to be verified by the component manufacturer and/or building designer prior to fabrication. The building designer shall ascertain that the loads utilized on this design meet or exceed the loading imposed by the local building code. It is assumed that the top chord is laterally braced by the roof or floor sheathing and the bottom chord is laterally braced by a rigid sheathing material directly attached, unless otherwise noted. Bracing shown is for lateral support of components members only to reduce buckling length. This component shall not be placed in any environment that will cause the moisture content of the wood exceed 19% and/or cause connector plate corrosion. Fabricate, handle, install and brace this truss in accordance with the following standards: "TRUSCOM MANUAL" by Truswal, "QUALITY CONTROL STANDARD FOR METAL PLATE CONNECTED WOOD TRUSSES" (QST-38), "HANDLING INSTALLING AND BRACING METAL PLATE CONNECTED WOOD TRUSSES" (HIB-91) and "HIB-91 SUMMARY SHEET" by TPI. The Truss Plate Institute (TPI) is located at 583 D'Onofrio Drive, Madison, Wisconsin 53719. The American Forest and Paper Association (AFPA) is located at 1250 Connecticut Ave, NW, Ste 200, Washington, DC 20036.

Western Wood Fabricators
 3700 Riego Road, Elverta, CA 95626

EG	X-LOC	RFCT	SIZE	RDY/D	TOP CHRD	2x6	EL #2
1	3-1-12	1.762	3.50"	1.88"	2x6 FL #2	2-15, 10-21	
2	45-10-4	4.245	3.50"	4.53"	2x4 FL #2	11-22	
3	49-2-12	-727	3.50"	1.50"	2x6 FL #2		

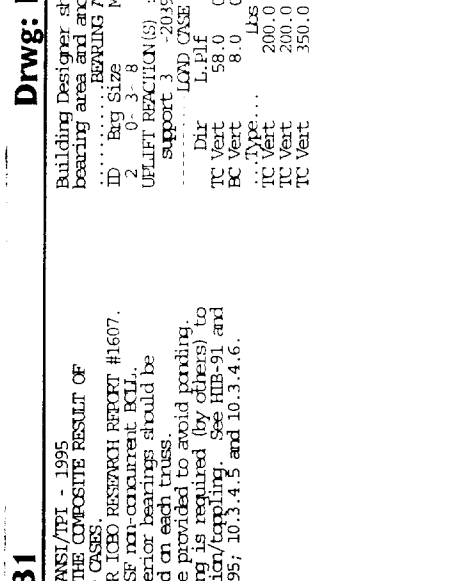
Plating spec : ANSI/TPI - 1995
 THIS DESIGN IS THE COMPOSITE RESULT OF
 MULTIPLE LOAD CASES.
 PLATE VALUES PER ICBO RESEARCH REPORT #1607.
 Loaded for 10 FSF non-concurrent B.C.L.
 Location of interior bearings should be
 clearly marked on each truss.
 Drainage must be provided to avoid ponding.
 Permanent bracing is required (by others) to
 prevent rotation/torquing. See HIB-91 and
 ANSI/TPI 1-1995, 10.3.4.5 and 10.3.4.6.

Interior support or temporary shoring must
 be in place before erecting this truss.
 PLATING BASED ON GREEN LUMBER VALUES.
 End verticals designed for axial loads only.
 End verticals that are extended above or
 below the truss profile (if any) may require
 additional design consideration (by others)
 for lateral forces due to wind or seismic
 loads on the building.

Building Designer shall provide adequate
 bearing area and anchorage.
 BEARING AREA REQUIRED
 ID Bag Size Min Size -or- Min Area
 2 0-3-8 0-4-8 6.79 sq
 UPLIFT REACTION(S)
 support 3 -2039#
 LxHf LxLc R.Df R.Loc Lx/IL
 58.0 0-0-0 58.0 49-3-0 .69
 8.0 0-0-0 8.0 49-3-0 .00
 Lx/IL
 Type... Lbs X.Loc Lx/IL
 TC Vert 200.0 23-0-0 .61
 TC Vert 200.0 29-7-7 .61
 TC Vert 350.0 32-4-4 .61

MAX DEFLECTION (span) :
 L/881 IN MEM 18-19 (LIVE)
 L = .58" D = .38" T = .95"
 MAX DEFLECTION (cant) :
 L/332 IN MEM 13-14 (LIVE)
 L = .11" D = .07" T = .18"

Joint Locations	1	2	3	4	5	6	7	8	9	10	11	12
0-0-0	13	0	0	0	0	0	0	0	0	0	0	0
3-1-12	14	3	1	12								
9-0-6	15	9	9	3								
9-9-3	16	14	4	0	0							
16-4-9	17	16	4	9								
23-0-0	18	23	0	0	0							
29-7-7	19	29	7	7								
36-2-13	20	36	2	13								
42-10-4	21	42	10	4								
45-10-4	22	45	10	4								
48-11-0	23	48	11	0								
48-11-0	24	48	11	0								



200# 200# 200# 350#
 22-8-8 26-4-8 49-4-8 0-7-8
 STUB

MEMBER	FORCE	CS1	CS2
12-24	-80	.62	-42 .01
13	2039	.37	7-18 -256 .22
1-13	-106	.02	7-19 -720 .11
1-14	241	.09	8-19 2039 .82
2-14	-1663	.22	8-21 -1432 .20
2-15	3357	.59	10-22 4263 .75
4-15	-1137	.16	10-22 -1906 .25
4-17	1819	.74	11-22 3107 .55
5-17	-627	.09	11-23 -2078 .26
5-18	527	.21	12-23 -3604 .75

Scale: 3/32" = 1'
 OVER 3 SUPPORTS
 Eng. Job: .EJ.
 Chk: CM
 Dgmt: EC
 Date: 5/13/98
 DurFacs L=1.25 P=1.25
 Rep Mbr Bnd 1.00
 O.C.Spacing 2-0-0
 Design Spec UBC
 Seqn 03.20.98- 4677

All plates are 20 gauge Truswal Connectors unless preceded by "18" for 18 gauge or "H" for 16 gauge.

WARNING Read all notes on this sheet and give a copy of it to the Erecting Contractor.
 This design is for an individual building component. It has been based on specifications provided by the component manufacturer and done in accordance with the current versions of TPI and AFPA design standards. No responsibility is assumed for dimensional accuracy. Dimensions are to be verified by the component manufacturer and/or building designer prior to fabrication. The building designer shall ascertain that the loads utilized on this design meet or exceed the loading imposed by the local building code. It is assumed that the top chord is laterally braced by the roof or floor sheathing and the bottom chord is laterally braced by a rigid sheathing material directly attached, unless otherwise noted. Bracing shown is for lateral support of components members only to reduce buckling length. This component shall not be placed in any environment that will cause the moisture content of the wood exceed 19% and/or cause connector plate corrosion. Fabricate, handle, install and brace this truss in accordance with the following standards: 'TRUSCOM MANUAL', by Truswal, 'QUALITY CONTROL STANDARD FOR METAL PLATE CONNECTED WOOD TRUSSES' - (QST-88), 'HANDLING INSTALLING AND BRACING METAL PLATE CONNECTED WOOD TRUSSES' - (HIB-91) and 'HIB-91 SUMMARY SHEET' by TPI. The Truss Plate Institute (TPI) is located at 583 D'Onofrio Drive, Madison, Wisconsin 53719. The American Forest and Paper Association (AFPA) is located at 1250 Connecticut Ave, N.W., Ste 200, Washington, DC 20036.

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 3700 Riego Road, Elverta, CA 95626

Building Designer shall provide adequate bearing area and anchorage.
 ... BEARING AREA REQUIRED ...
 ID Brg Size Min Size -or- Min Area
 2 0- 3- 8 0- 5- 0 7.48 sq
 UPLIFT REACTION(S)
 support 3 -2220#
 ---LOAD CASE #1 DESIGN LOADS
 Dir L.R.F. L.Loc R.P.F. R.Loc LL/TL
 TC Vert 58.0 3- 0- 0 58.0 49- 3- 0 .69
 EC Vert 8.0 3- 0- 0 8.0 49- 3- 0 .69
 ...Type... X.Loc LL/TL
 TC Vert 200.0 23- 0- 0 .61
 TC Vert 200.0 29- 7- 7 .61
 TC Vert 700.0 32- 4- 4 .61

Plating spec : ANSI/TPI - 1995
 THIS DESIGN IS THE COMPOSITE RESULT OF MULTIPLE LOAD CASES.
 PLATE VALUES PER ICBO RESEARCH REPORT #1607.
 Loaded for 10 PSF non-concurrent ROLL.
 Location of interior bearings should be clearly marked on each truss.
 Interior support or temporary shoring must be in place before erecting this truss.
 Drainage must be provided to avoid ponding.
 PLATING BASED ON GREEN LUMBER VALUES.
 Brd verticals that are extended above or below the truss profile (if any) may require additional design consideration (by others) for lateral forces due to wind or seismic loads on the building.

REQ	X-LOC	REACT	SIZE	REQ'D	TOP CHORD	EL #2	4-7
1	3- 1-12	1652	3.50"	1.76"	2x6	EL SS	7-10
2	45-10- 4	4677	3.50"	4.99"	2x6	EL #1	13-17
3	49- 2-12	-814	3.50"	1.50"	2x6	EL #1	1-12, 8-18
4	49- 2-12	-814	3.50"	1.50"	2x4	EL #2	9-19
5	3748	.09	.33	.43	2x6	EL #2	
6	5813	.19	.22	.42			
7	6556	.17	.31	.58			
8	7005	.17	.79	.97			
9	4624	.08	.28	.34			
10	464	.05	.22	.27			
11	3387	.37	.48	.85			

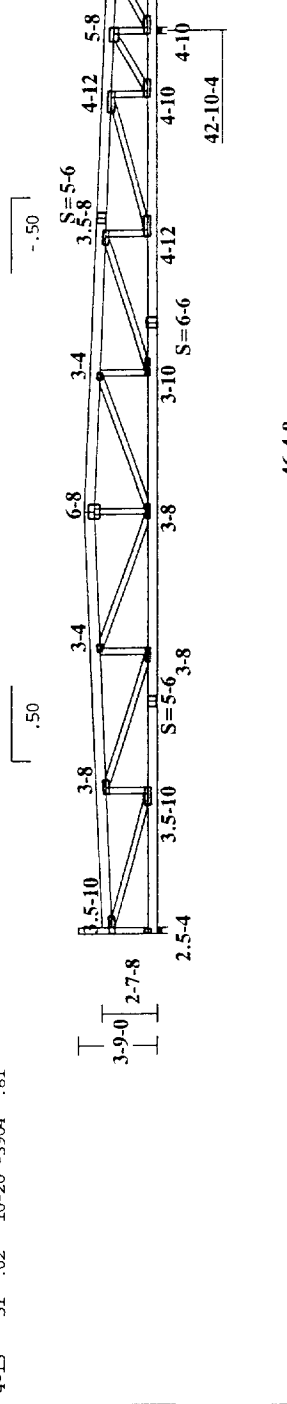
MAX DEFLECTION (span) :
 L/822 IN MEM 15-16 (LIVE)
 L_r = -.62" D_r = .40" T_r = 1.02"

Joint Locations

1	0- 0- 0	12	6- 9- 3
2	6- 9- 3	13	11- 0- 0
3	13- 4- 9	14	13- 4- 9
4	20- 0- 0	15	20- 0- 0
5	26- 7- 7	16	26- 7- 7
6	33- 2-13	17	29- 0- 0
7	33-11-10	18	33- 2-13
8	39-10- 4	19	39-10- 4
9	42-10- 4	20	42-10- 4
10	45-11- 0	21	45-11- 0
11	0- 0- 0		

REQ	X-LOC	REACT	SIZE	REQ'D	TOP CHORD	EL #2	4-7
1	0	.00	.08	.08	2x6	EL #2	
2	3985	.52	.12	.64	2x6	EL #1	
3	3985	.44	.10	.54	2x6	EL #1	
4	5907	.65	.10	.75	2x6	EL #1	
5	7101	.78	.12	.90	2x6	EL #1	
6	4943	.55	.13	.68	2x4	EL #2	
7	4943	.64	.15	.80			
8	-91	.00	.10	.10			
9	-3232	.05	.33	.38			
10	-128	.00	.33	.33			

REQ	X-LOC	REACT	SIZE	REQ'D	TOP CHORD	EL #2	4-7
10-21	-88	.68	5-15	-608	53		
11-11	2220	.40	5-16	-781	.12		
1-11	-1616	.38	6-16	2198	.89		
1-12	3951	.70	6-18	-1679	.28		
2-12	-1261	.17	8-18	4969	.88		
2-14	1953	.79	8-19	-2133	.28		
3-14	-684	.10	9-19	3479	.62		
3-15	688	.28	9-20	-2334	.29		
4-15	51	.02	10-20	-3904	.81		



Scale: 3/32" = 1'

OVER 3 SUPPORTS

Eng. Job : .EJ.
 Chk: CM
 Dsgn: EC

TC Live 20.0 psf
 TC Dead 9.0 psf
 BC Live .0 psf
 BC Dead 4.0 psf

TOTAL 33.0 psf

Rep Mbr Bnd 1.00
 O.C.Spacing 2- 0- 0
 Design Spec UBC

Segn. 03.20.98- 4664

WO: J8140
 Date: 5/13/98

DurFacs L=1.25 P=1.25
 DurFacs L=1.25 P=1.25

All plates are 20 gauge Truswal Connectors unless preceded by "18" for 18 gauge or "H" for 16 gauge.

WARNING Read all notes on this sheet and give a copy of it to the Erecting Contractor.
 This design is for an individual building component. It has been based on specifications provided by the component manufacturer and done in accordance with the current versions of TPI and AFPA design standards. No responsibility is assumed for dimensional accuracy. Dimensions are to be verified by the component manufacturer and/or building designer prior to fabrication. The building designer shall ascertain that the loads utilized on this design meet or exceed the loading imposed by the local building code. It is assumed that the top chord is laterally braced by the roof or floor sheathing and the bottom chord is laterally braced by a rigid sheathing material directly attached, unless otherwise noted. Bracing shown is for lateral support of components members only to reduce buckling length. This component shall not be placed in any environment that will cause the moisture content of the wood exceed 19% and/or cause connector plate corrosion. Fabricate, handle, install and brace this truss in accordance with the following standards: "TRUSCOM MANUAL", by Truswal, "QUALITY CONTROL STANDARD FOR METAL PLATE CONNECTED WOOD TRUSSES" - (QST-88), "HANDLING INSTALLING AND BRACING METAL PLATE CONNECTED WOOD TRUSSES" - (HIB-91) and "HIB-91 SUMMARY SHEET" by TPI. The Truss Plate Institute (TPI) is located at 583 D'Onofrio Drive, Madison, Wisconsin 53719. The American Forest and Paper Association (AFPA) is located at 1250 Connecticut Ave, NW, Ste 200, Washington, DC 20036.

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 3700 Riego Road, Elverta, CA 95626

Building Designer shall provide adequate bearing area and anchorage.
 ... BEARING AREA REQUIRED ...
 ID Brg Size Min Size -or- Min Area
 2 0- 3- 8 0- 5- 0 7.48 sq
 UPLIFT REACTION(S)
 support 3 -2220#
 ---LOAD CASE #1 DESIGN LOADS
 Dir L.R.F. L.Loc R.P.F. R.Loc LL/TL
 TC Vert 58.0 3- 0- 0 58.0 49- 3- 0 .69
 EC Vert 8.0 3- 0- 0 8.0 49- 3- 0 .69
 ...Type... X.Loc LL/TL
 TC Vert 200.0 23- 0- 0 .61
 TC Vert 200.0 29- 7- 7 .61
 TC Vert 700.0 32- 4- 4 .61

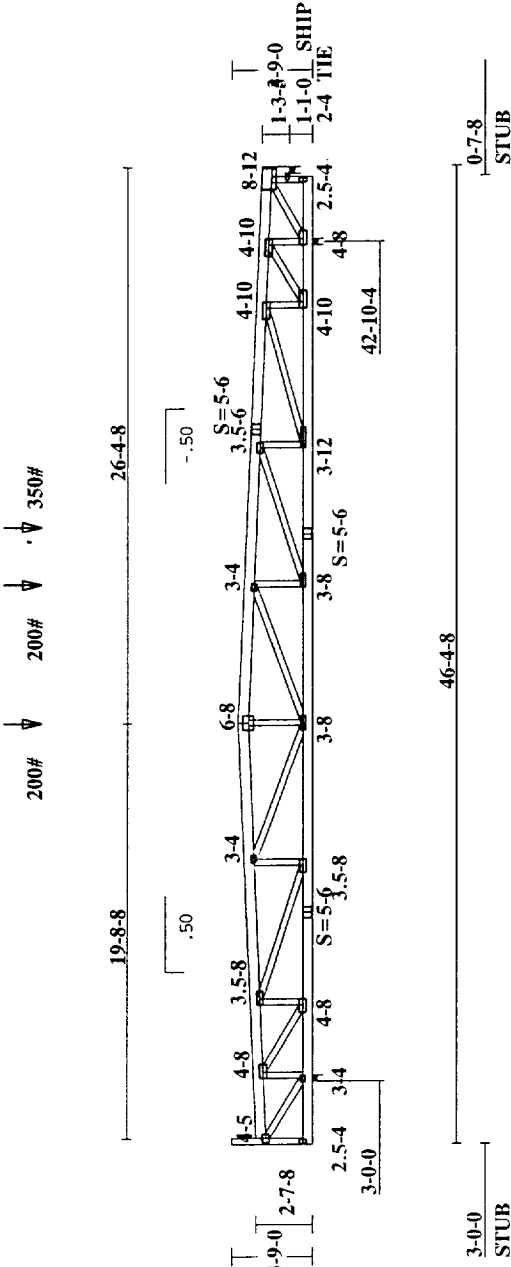
FRG	X-LOC	REACT	SIZE	REQ'D	TOP CHORD	2x6	FL #2	5-8	PLATING
1	6-1-12	1944	3.50"	2.07"	BOF CHORD	2x6	FL SS		TC
2	45-10-4	4168	3.50"	4.45"	WEB	2x4	FL #2		BC
3	49-2-12	-638	3.50"	1.50"		2x4	FL #2		
TC	FORCE	AXL	END	CSI					
1	133	.04	.13	.14					
2	-2318	.04	.18	.22					
3	-4872	.21	.26	.47					
4	5915	.31	.31	.62					
5	5916	.20	.20	.41					
6	-6318	.23	.50	.73					
7	-4273	.17	.22	.38					
8	-4289	.17	.22	.38					
9	126	.02	.21	.23					
10	2835	.36	.45	.81					
BC	FORCE	AXL	END	CSI					
1	0	.00	.11	.11					
2	311	.03	.10	.13					
3	2517	.31	.09	.40					
4	2517	.31	.09	.40					
5	5000	.64	.11	.75					
6	6399	.83	.12	.95					
7	4557	.59	.13	.72					
8	4557	.59	.14	.73					
9	268	.03	.08	.12					
10	-2688	.94	.30	.85					
11	-107	.00	.30	.30					
11-23	-74	.56	5-17	-101 .02	CSI				
BB	1838	.33	6-17	-570 .49	CSI				
1-12	-98	.02	6-18	-651 .10	CSI				
1-13	-158	.05	7-18	1882 .76	CSI				
2-13	-1803	.24	7-20	-1429 .20	CSI				
2-14	2469	1.00	9-20	4276 .75	CSI				
3-14	-1429	.20	9-21	-1969 .25	CSI				
3-16	2579	.46	10-21	3224 .57	CSI				
4-16	-917	.14	10-22	-2190 .28	CSI				
4-17	1022	.42	11-22	-3267 .68	CSI				

Plating spec : ANSI/TPI - 1995
 THIS DESIGN IS THE COMPOSITE RESULT OF
 MULTIPLE LOAD CASES.
 PLATE VALUES PER ICEO RESEARCH REPORT #1607.
 Location of 10 EBF non-concurrent BCLL.
 Location of interior bearings should be
 clearly marked on each truss.
 Interior support or temporary shoring must
 be in place before erecting this truss.
 PLATING BASED ON GREEN LUMBER VALUES.
 End verticals designed for axial loads only.
 End verticals that are exterior above or
 below the truss profile (if any) may require
 additional design consideration (by others)
 for lateral forces due to wind or seismic
 loads on the building.

This design based on chord bracing applied
 per the following schedule:
 max O.C. from to
 TC 24.0" 0-0-0 50-0-0
 BC 48.0" 0-0-0 50-0-0
 Building Designer shall provide adequate
 bearing area and anchorage.
 REPAIRING AREA REQUIRED.
 ID ... Brg Size Min Size or Min Area
 2 0-3-8 0-4-7 6.67 sq
 UPLIFT REACTION(S) :
 support 3 -1838#
 -----LOAD CASE #1 DESIGN LOADS-----
 Dir L.Plf L.Loc R.Loc L4/TL
 TC Vert 67.7 3-0-0 67.7 49-3-0 .69
 BC Vert 9.3 3-0-0 9.3 49-3-0 .00
 Type... lbs X.Loc L4/TL
 TC Vert 200.0 23-0-0 .61
 TC Vert 200.0 29-7-7 .61
 TC Vert 350.0 32-4-4 .61

MAX DEFLECTION (span) :
 L/961 IN MEM 17-18 (LIVE)
 L = .49" D = .32" T = -.81"
 MAX DEFLECTION (cant) :
 L/368 IN MEM 12-13 (LIVE)
 L = .10" D = .06" T = .16"

Joint Locations	1	2	3	4	5	6	7	8	9	10	11	12
0-0-0	13	3	3	6	2	14	6	9	2	15	11	0-0-0
3-0-0	14	6	9	2	15	11	0-0-0	0	0	16	13	4-9
6-0-0	15	11	0-0-0	0	0	16	13	4-9	0	17	20	0-0
9-0-0	16	13	4-9	0	0	17	20	0-0	0	18	26	7-7
12-0-0	17	20	0-0	0	0	18	26	7-7	0	19	29	0-0
15-0-0	18	26	7-7	0	0	19	29	0-0	0	20	33	2-13
18-0-0	19	29	0-0	0	0	20	33	2-13	0	21	39	10-4
21-0-0	20	33	2-13	0	0	21	39	10-4	0	22	42	10-4
24-0-0	21	39	10-4	0	0	22	42	10-4	0	23	45	11-0
27-0-0	22	42	10-4	0	0	23	45	11-0	0	24	45	11-0
30-0-0	23	45	11-0	0	0	24	45	11-0	0	25	45	11-0



All plates are 20 gauge Trussal Connectors unless preceded by "18" for 18 gauge or "H" for 16 gauge.

OVER 3 SUPPORTS

Scale: 3/32" = 1'

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Eng. Job: .EJ.	MO: J8140
Chk: CM	Date: 5/13/98
Dsgnr: EC	DurFacs L=1.25 P=1.25
TC Live 20.0 psf	Rep Mbr Bnd 1.00
TC Dead 9.0 psf	O.C.Spacing 2-4-0
BC Live .0 psf	Design Spec UBC
BC Dead 4.0 psf	
TOTAL 33.0 psf	Segm 03.20.98- 4660

Western Wood Fabricators
 3700 Riego Road, Elverta, CA 95626

PKG	X-LOC	REACT	SIZE	REQ'D	TOP CHFD	2x6	EL #2	5-8
1	6'-1-12	2677	3.50"	1.50"	BUT CHFD	2x6	EL #1	12-15
2	45'-10-4	8302	3.50"	4.43"	WEB	2x4	EL #2	9-20, 10-21
TC	FORCE	AXL	RD	CSI	Lumber shear allowable as per NDS-91. Drainage must be provided to avoid ponding. Permanent bracing is required (by others) to prevent rotation/toppling. See HIB-91 and ANSI/TPI 1-1995; 10.3.4.5 and 10.3.4.6. 2-PLY Nail w/10d BOX, staggered (NDS-91 Sect. 12) in: TC- 2/ft BC- 2/ft WEBS- 2/ft			
1	-126	.00	.07	.08				
2	-3440	.09	.09	.10				
3	-7850	.09	.13	.22				
4	-10738	.08	.16	.34				
5	-14222	.08	.17	.25				
6	-14222	.17	.61	.78				
7	-14230	.13	.48	.61				
8	-14326	.27	.68	.96				
9	-5963	.04	.55	.59				
10	-2473	.15	.44	.59				
BC	FORCE	AXL	RD	CSI				
1	0	.00	.07	.07				
2	416	.02	.07	.09				
3	3746	.24	.06	.30				
4	3746	.20	.06	.26				
5	8092	.44	.07	.51				
6	14092	.77	.09	.86				
7	14800	.81	.13	.94				
8	14800	.81	.18	.99				
9	6836	.37	.18	.55				
10	-2208	.00	.20	.20				
11	0	.00	.20	.20				

Plating spec: ANSI/TPI - 1995
 THIS DESIGN IS THE COMPOSITE RESULT OF MULTIPLE LOAD CASES.
 PLATE VALUES PER IBCO RESEARCH REPORT #1607. Loaded for 10 PSF non-concurrent BC/L. Nail pattern shown is for uniform loads only. Concentrated loads > 350# must be distributed (by others) equally to each ply, unless nail clusters are shown (). PLATING BASED ON GREEN LUMBER VALUES. End verticals designed for axial loads only. End verticals that are extended above or below the truss profile (if any) may require additional design consideration (by others) for lateral forces due to wind or seismic loads on the building.

Building Designer shall provide adequate bearing area and anchorage.

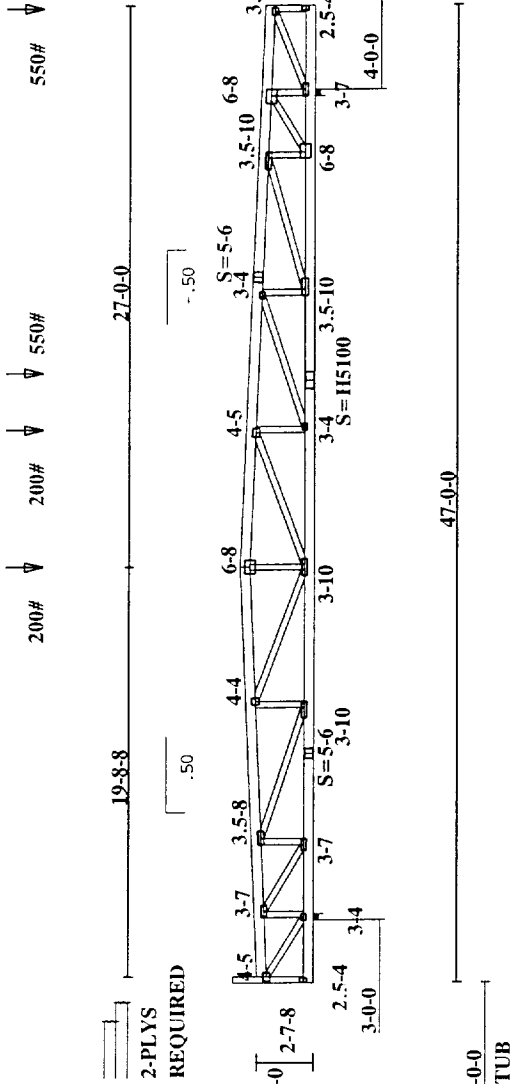
ID	Brig Size	Min Size	cr	Min Area
2	0-3-8	0-4-7		13.28 sq

LOAD CASE #1 DESIGN LIMITS

Dir	L.PLF	L.LOC	R.PLF	R.LOC	U./TL
TC Vert	58.0	3-0-0	0	0	0
TC Vert	408.0	32-0-0	0	0	0
BC Vert	8.0	3-0-0	0	0	0
TC Vert	200.0	23-0-0	0	0	0
TC Vert	200.0	29-7-7	0	0	0
TC Vert	550.0	32-4-4	0	0	0
TC Vert	550.0	49-10-4	0	0	0

MAX DEFLECTION (span) :
 L/904 IN MEM 17-18 (LIVE)
 L_c = .52" D = .34" T = .86"
 MAX DEFLECTION (cant) :
 L/319 IN MEM 22-23 (LIVE)
 L_c = .15" D = .10" T = .25"

Joint Locations	1	2	3	4	5	6	7	8	9	10	11	12
0-0-0	0	0	13	3	3	6	6	6	6	6	6	6
3-3-6	14	15	11	0	0	0	0	0	0	0	0	0
6-9-2	16	13	4	9	9	9	9	9	9	9	9	9
13-4-9	17	20	0	0	0	0	0	0	0	0	0	0
20-0-0	17	18	26	7	7	7	7	7	7	7	7	7
26-7-7	18	26	7	7	7	7	7	7	7	7	7	7
33-2-13	19	29	0	0	0	0	0	0	0	0	0	0
33-11-10	20	33	2	13	13	13	13	13	13	13	13	13
39-10-4	21	39	10	4	4	4	4	4	4	4	4	4
42-10-4	22	42	10	4	4	4	4	4	4	4	4	4
47-0-0	23	47	0	0	0	0	0	0	0	0	0	0



All plates are 20 gauge Truswal Connectors unless preceded by "18" for 18 gauge or "H" for 16 gauge.

Western Wood Fabricators
 3700 Riego Road, Elverta, CA 95626

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Eng. Job: .EJ.	WO: J8140
Chk: CM	Date: 5/13/98
Dsgnr: EC	DurFacs L=1.25 P=1.25
TC Live 20.0 psf	Rep Mbr Bnd 1.00
TC Dead 9.0 psf	O.C.Spacing 2-0-0
BC Live .0 psf	Design Spec UBC
BC Dead 4.0 psf	Seqn 03.20.98- 4653
TOTAL 33.0 psf	

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Scale: 3/32" = 1'

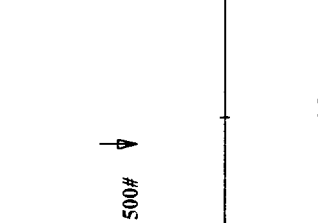
REQ	X-LOC	RECT	SIZE	REQ'D	TOP CHRD	FL #2	2x6	FL #2	2x4	FL #2	2-14
1	6-1-12	6214	3.50"	3.31"	WEB	2x4	FL #2	2x4	FL #2	2-14	
2	45-10-4	2262	3.50"	1.50"	WEB	2x4	FL #2	2x4	FL #2	2-14	
TC	FORCE	AXL	END	CSI	Nail pattern shown is for uniform loads only. Concentrated loads > 350# must be distributed (by others) equally to each ply, unless nail clusters are shown ().						
1	623	.04	.23	.26	PLATING BASED ON GREEN LUMBER VALUES.						
2	-5619	.04	.50	.54	2-FLY Nail w/10d BOX staggered (NLS-91)						
3	-10417	.15	.61	.77	Sect. 12) in: TC-2/ft BC-2/ft WEBS-2/ft						
4	-9937	.11	.49	.60							
5	-9906	.14	.13	.27							
6	-8940	.13	.17	.30							
7	-6653	.02	.00	.12							
8	-6666	.06	.12	.18							
9	-2705	.01	.10	.10							
10	-128	.00	.06	.06							
BC	FORCE	AXL	END	CSI							
1	0	.00	.13	.13							
2	390	.02	.12	.14							
3	6146	.39	.11	.51							
4	6146	.39	.12	.52							
5	10629	.68	.12	.81							
6	9045	.58	.07	.65							
7	6907	.44	.08	.52							
8	6907	.44	.08	.53							
9	3015	.19	.06	.25							
10	263	.01	.06	.07							
11	0	.00	.06	.06							

Plating spec: ANSI/TPI - 1995
 THIS DESIGN IS THE COMPOSITE RESULT OF MULTIPLE LOAD CASES.
 PLATE VALUES PER ICGO RECOMM. WITH REFORT #1607.
 Loaded for 10 PSF non-concurrent BIL.
 Drainage must be provided to avoid ponding.
 Permanent bracing is required (by others) to prevent rotation/twisting. See HB-91 and ANSI/TPI 1-1995, 10.3.4.5, and 10.3.4.6.
 Bld verticals designed for axial loads only. Bld verticals that are extended above or below the truss profile (if any) may require additional design consideration (by others) for lateral forces due to wind or seismic loads on the building.

DLY	L.PLF	L.Loc	R.PLF	R.Loc	LL/TL
TC Vert	280.0	0-0-0	280.0	3-0-0	0.00
TC Vert	338.0	3-0-0	338.0	18-9-0	.34
TC Vert	58.0	18-9-0	58.0	49-0-0	.69
BC Vert	8.0	3-0-0	8.0	49-0-0	.00
TC Vert	500.0	6-1-12	500.0	21-9-0	.61
TC Vert	500.0	21-9-0	500.0	61	.61

MAX DEFLECTION (span):
 L/999 IN MEM 17-18 (LIVE)
 L_F = .41" D = .27" T = .67"
 MAX DEFLECTION (CENT):
 L/395 IN MEM 12-13 (LIVE)
 L_F = .09" D = .06" T = .15"

Joint Locations	1	2	3	4	5	6	7	8	9	10	11	12
0-0-0	0	0	0	0	0	0	0	0	0	0	0	0
3-3-6	3	3	3	3	3	3	3	3	3	3	3	3
6-6-12	6	6	6	6	6	6	6	6	6	6	6	6
9-9-18	9	9	9	9	9	9	9	9	9	9	9	9
12-12-24	12	12	12	12	12	12	12	12	12	12	12	12
15-15-30	15	15	15	15	15	15	15	15	15	15	15	15
18-18-36	18	18	18	18	18	18	18	18	18	18	18	18
21-21-42	21	21	21	21	21	21	21	21	21	21	21	21
24-24-48	24	24	24	24	24	24	24	24	24	24	24	24
27-27-54	27	27	27	27	27	27	27	27	27	27	27	27
30-30-60	30	30	30	30	30	30	30	30	30	30	30	30
33-33-66	33	33	33	33	33	33	33	33	33	33	33	33
36-36-72	36	36	36	36	36	36	36	36	36	36	36	36
39-39-78	39	39	39	39	39	39	39	39	39	39	39	39
42-42-84	42	42	42	42	42	42	42	42	42	42	42	42
45-45-90	45	45	45	45	45	45	45	45	45	45	45	45
48-48-96	48	48	48	48	48	48	48	48	48	48	48	48
51-51-102	51	51	51	51	51	51	51	51	51	51	51	51
54-54-108	54	54	54	54	54	54	54	54	54	54	54	54
57-57-114	57	57	57	57	57	57	57	57	57	57	57	57
60-60-120	60	60	60	60	60	60	60	60	60	60	60	60
63-63-126	63	63	63	63	63	63	63	63	63	63	63	63
66-66-132	66	66	66	66	66	66	66	66	66	66	66	66
69-69-138	69	69	69	69	69	69	69	69	69	69	69	69
72-72-144	72	72	72	72	72	72	72	72	72	72	72	72
75-75-150	75	75	75	75	75	75	75	75	75	75	75	75
78-78-156	78	78	78	78	78	78	78	78	78	78	78	78
81-81-162	81	81	81	81	81	81	81	81	81	81	81	81
84-84-168	84	84	84	84	84	84	84	84	84	84	84	84
87-87-174	87	87	87	87	87	87	87	87	87	87	87	87
90-90-180	90	90	90	90	90	90	90	90	90	90	90	90
93-93-186	93	93	93	93	93	93	93	93	93	93	93	93
96-96-192	96	96	96	96	96	96	96	96	96	96	96	96
99-99-198	99	99	99	99	99	99	99	99	99	99	99	99
102-102-204	102	102	102	102	102	102	102	102	102	102	102	102
105-105-210	105	105	105	105	105	105	105	105	105	105	105	105
108-108-216	108	108	108	108	108	108	108	108	108	108	108	108
111-111-222	111	111	111	111	111	111	111	111	111	111	111	111
114-114-228	114	114	114	114	114	114	114	114	114	114	114	114
117-117-234	117	117	117	117	117	117	117	117	117	117	117	117
120-120-240	120	120	120	120	120	120	120	120	120	120	120	120



2-PLYS REQUIRED

SHIP

STUB


STUB

[Handwritten Signature]

Eng. Job: .BJ.
 Chk: CM
 Dsgnr: EC
 Date: 5/12/98
 DurFacs L=1.25 P=1.25
 Rep Mbr Bnd 1.00
 O.C.-Spacing 2-0-0
 Design Spec UBC
 Seqn 03.20.98- 4458

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Western Wood Fabricators

3700 Riego Road, Elverta, CA 95626

BRG	X-LOC	REACT	SIZE	REOY'D
1	21-10-4	1.99	3.50"	1.50"
2	45-10-4	1.93	3.50"	1.50"

TOP CHORD 2x6 FL #2
 BOT CHORD 2x6 FL #2
 WEB 2x4 FL STANDARD
 loaded for 10 PSF non-concurrent BOLL.
 Drainage must be provided to avoid ponding.
 Permanent bracing is required (by others) to prevent rotation/toppling. See HIB-91 and ANSI/TPI 1-1995; 10.3.4.5 and 10.3.4.6.
 End verticals designed for axial loads only.
 End verticals that are extended above or below the truss profile (if any) may require additional design consideration (by others) for lateral forces due to wind or seismic loads on the building.
 2-PLY; Nail w/10d BX, staggered (NDS-91, Sect. 12) in: TC- 2/ft BC- 2/ft WEBS- 2/ft

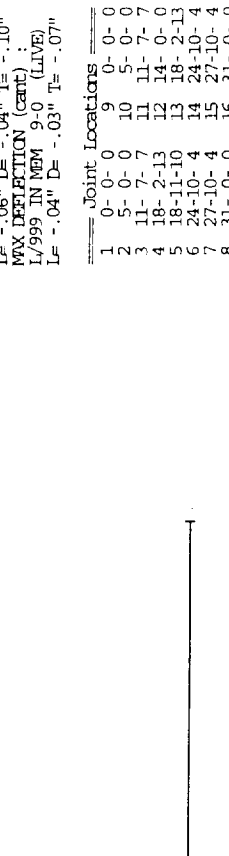
TC FORCE	AXL	END	CSI	WEB FORCE	CSI
1	-400	.00	.08	6-13	2096 .40
2	-1401	.00	.06	6-14	-868 .07
3	-2113	.00	10	7-14	1506 .29
4	-2198	.01	.09	7-15	-1011 .06
5	-2212	.01	.09	8-15	-158 .02
6	-1132	.00	.08	8-16	-75 .01
7	297	.02	.05		

MAX DEFLECTION (span):
 L/999 IN MEM 10-11 (LIVE)
 L_F = .06" D_F = -.04" T_F = -.10"
 MAX DEFLECTION (CENT):
 L/999 IN MEM 9-0 (LIVE)
 L_F = .04" D_F = -.03" T_F = -.07"

Joint Locations:
 1 0-0-0 9 0-0-0
 2 5-0-0 10 5-0-0
 3 11-7-7 11 11-7-7
 4 18-2-13 12 14-0-0
 5 18-11-10 13 18-2-13
 6 24-10-4 14 24-10-4
 7 27-10-4 15 27-10-4
 8 31-0-0 16 31-0-0

Plating spec: ANSI/TPI - 1995
 THIS DESIGN IS THE COMPOSITE RESULT OF MULTIPLE LOAD CASES.
 PLATE VALUES PER ICBO RESEARCH REPORT #1607.
 Nail pattern shown is for uniform loads only. Concentrated loads > 350# must be distributed (by others) equally to each ply, unless nail clusters are shown ().
 PLATING BASED ON GREEN LUMBER VALUES.
 Designed for 5.9 K lbs drag load applied evenly along the top chord to the bottom chord at the bearings, concurrently with dead loads only. Duration-1.33 Horizontal reaction = 5.9 K lbs. (each bearing).

DEFLECT REACTION(S):
 support 1 -72#
 support 2 -196#
 Dir L.Plf L.Loc R.Plf R.Loc L.Loc R.Loc L.Loc
 TC Vert 58.0 18.0 0.0 58.0 49.0 0.0
 EC Vert 8.0 18.0 0.0 8.0 49.0 0.0
 TC Vert 11.5 X.Loc 200.0 23-0-0
 EC Vert 200.0 29-7-7



2-PLYS REQUIRED
 5.0-0
 25-8-8
 -1.50
 3-9-0
 3-0-0
 18-0-0
 STUB
 31-0-0
 1-0-0
 STUB



All plates are 20 gauge Truswal Connectors unless preceded by "18" for 18 gauge or "H" for 16 gauge.

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Eng. Job: .EJ.	WO: J8140
Chk: CM	Date: 5/13/98
Desgr: EC	DurFacs L=1.25 P=1.25
TC Live 20.0 psf	Rep Mbr Bnd 1.00
TC Dead 9.0 psf	O.C.Spacing 2-0-0
BC Live .0 psf	Design Spec UBC
BC Dead 4.0 psf	
TOTAL 33.0 psf	Segn 03.20.98 - 4743

Dir	L.PLF	#1 DESIGN LOADS	R.Ioc	LL/TL
TC Vert	58.0	18-8-8	49-0-0	69
BC Vert	8.0	18-8-8	49-0-0	00
TC Vert	200.0	X.Loc		
TC Vert	200.0	29-7-7		.61

Dir	L.PLF	#1 DESIGN LOADS	R.Ioc	LL/TL
TC Vert	58.0	18-8-8	49-0-0	69
BC Vert	8.0	18-8-8	49-0-0	00
TC Vert	200.0	X.Loc		
TC Vert	200.0	29-7-7		.61

By continuous lateral WEB bracing (CLB), nailed w/2-10d, located for equal segments. OR 1x4 "m" brace nailed flat to edge of web w/ 8d at 8" o.c. OR a scab (same as web) nailed to face of web w/10d at 8" o.c. If 2 are required, attach "m" to both edges or scab to both faces. If 3 or more are req., use CLB. "m" or scab must be 90% of web length, and 2x6 if web is > 14'-0". Use stress-graded lumber & box or common rails. PLATING FABR ON GREEN LUMBER VALUES. End verticals designed for axial loads only. End verticals that are extended above or below the truss profile (if any) may require additional design consideration (by others) for lateral forces due to wind or seismic loads on the building.

2x6 FL #2
2x6 FL #2
2x4 FL STANDARD
Plating spec: ANSI/TPI - 1995
THIS DESIGN IS THE COMPOSITE RESULT OF MULTIPLE LOAD CASES.
PLATE VALUES PER IODD RESEARCH REPORT #1607.
Loaded for 10 psf non-concurrent ECLL.
Drainage must be provided to avoid ponding.
Permanent bracing is required (by others) to prevent rotation/toppling. See HIB-91 and ANSI/TPI 1-1995; 10.3.4.5 and 10.3.4.6.

BEG	X-LOC	REACT	SIZE	REQ'D
1	18-10-4	1196	3-50"	1-50"
2	45-10-4	1220	3-50"	1-50"

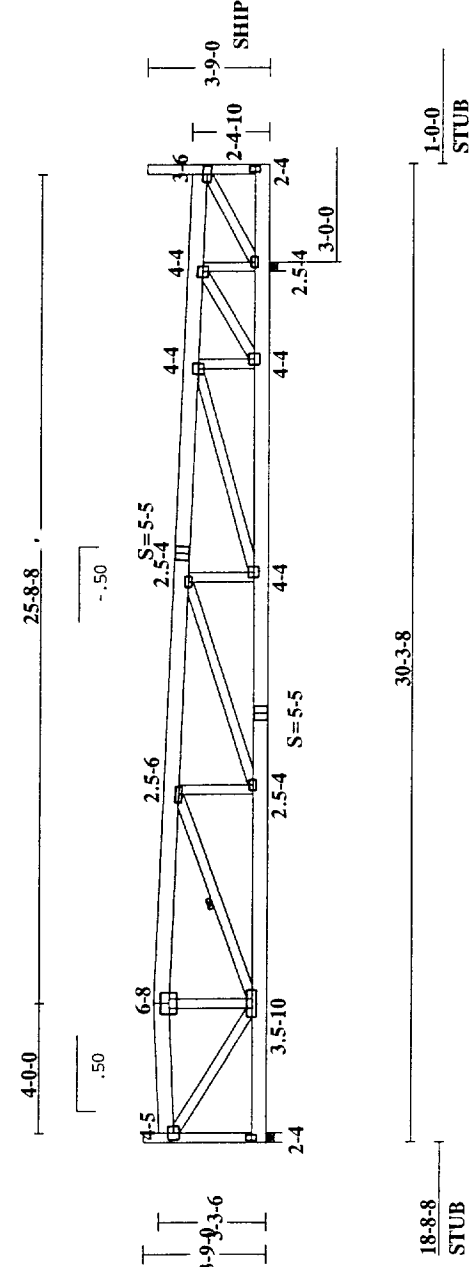
TC	FORCE	AXL	END	CSI
1	-1503	.01	.11	.12
2	-1497	.01	.22	.23
3	-2833	.04	.18	.23
4	-2632	.03	.17	.20
5	-2705	.04	.19	.23
6	-1302	.01	.17	.18
7	128	.01	.08	.10

BC	FORCE	AXL	END	CSI
1	0	.00	.03	.03
2	2818	.36	.07	.43
3	2776	.35	.08	.43
4	2776	.35	.08	.44
5	1443	.17	.06	.24
6	154	.01	.06	.07
7	0	.00	.07	.07

WEB	FORCE	CSI	WEB	FORCE	CSI
1-9	-1179	.28	6-13	1394	.57
1-10	1802	.73	6-14	848	.11
2-10	-381	.06	7-14	1434	.58
3-10	-1447	.51	7-15	-1097	.14
3-11	32	.01	8-15	-147	.04
4-11	101	.04	8-16	-.77	.02
4-13	-433	.06			

MAX DEFLECTION (SPAN):
L/999 IN MEM 11-12 (LIVE)
L = .14" D = .09" T = .23"
MAX DEFLECTION (CENT):
L/999 IN MEM 15-16 (LIVE)
L = .04" D = .02" T = .06"

Joint Locations
1 0-0-0 9 0-0-0
2 4-3-8 10 4-3-8
3 10-10-15 11 10-10-15
4 17-6-5 12 13-3-8
5 18-3-2 13 17-6-5
6 24-1-12 14 24-1-12
7 27-1-12 15 27-1-12
8 30-3-8 16 30-3-8



All plates are 20 gauge Trussal Connectors unless preceded by "18" for 18 gauge or "H" for 16 gauge.

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Western Wood Fabricators
3700 Riego Road, Elverta, CA 95626

Eng. Job: .EJ.	WO: JB140
Chk: CM	Date: 5/13/98
Desgr: EC	DurFacs L=1.25 P=1.25
TC Live 20.0 psf	Rep Mbr Bnd 1.00
TC Dead 9.0 psf	O.C.Spacing 2-0-0
BC Live .0 psf	Design Spec UBC
BC Dead 4.0 psf	
TOTAL 33.0 psf	Segn 03.20.98- 4740

Job Name: CARLS JR. 14DC

Truss ID: R

Drwg: R

Dir	TC Vert	BC Vert	...Type	BC Vert	Dir	L.Plf	L. Loc	R.Plf	R. Loc	LL/TL
						60.0	0-0-0	60.0	17-9-8	53
						8.0	0-0-0	8.0	17-9-8	.00
						648.5	8-10-12	1.00		
						729.6	8-10-12	.00		

Plating spec : ANSI/TPI - 1995
 THIS DESIGN IS THE COMPOSITE RESULT OF
 MULTIPLE LOAD CASES.
 PLATE VALUES PER ICEO RESEARCH REPORT #1607.
 Loaded for 10 PSF ice-concurrent BOLL.
 PLATING BASED ON GREEN LUMBER VALUES.

Permanent bracing is required (by others) to
 prevent rotation/toppling. See HB-91 and
 ANSI/TPI 1-1995; 10.3.4.5 and 10.3.4.6.

REQ'D	SIZE	REQ'D	SIZE	REQ'D	SIZE
1	0-1-12	1294	3-50"	1-50"	1-50"
2	17-7-12	1294	3-50"	1-50"	1-50"

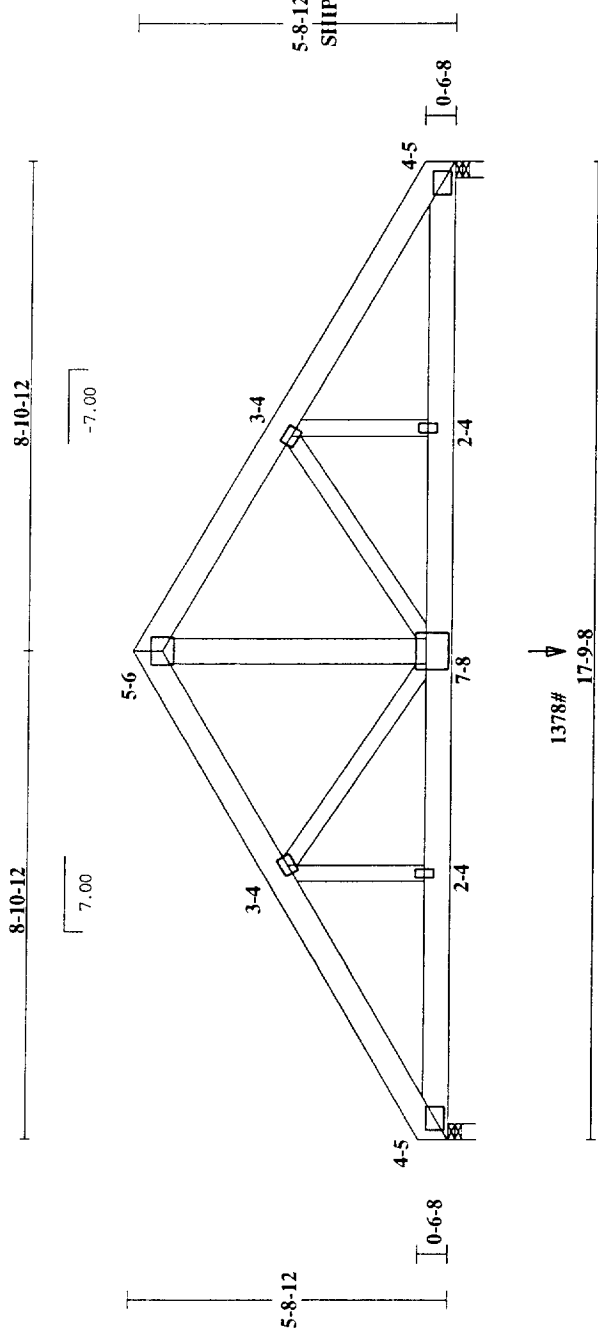
TC FORCE	AXL	END	CSI
1	-2178	.02	.10
2	-1978	.02	.10
3	-1978	.02	.10
4	-2178	.02	.10

BC FORCE	AXL	END	CSI
1	1820	.24	.07
2	1820	.24	.07
3	1820	.24	.07
4	1820	.24	.07

WEB FORCE	CSI	WEB FORCE	CSI
2-7	.37	.02	4-8
2-8	-197	.05	4-9
3-8	1679	.22	37

MAX DEFLECTION (span) :
 L/999 IN MEM 7-8 (LIVE)
 L_e = .03" D = -.04" T = -.07"

Joint Locations	
1	0-0-0
2	4-10-0
3	8-10-12
4	12-11-8
5	17-9-8



[Handwritten signature]

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Western Wood Fabricators
 3700 Riego Road, Elverta, CA 95626

Eng. Job: .EJ.	WO: J8140
Chk: CM	Date: 5/13/98
Design: EC	
TC Live	16.0 psf
TC Dead	14.0 psf
BC Live	.0 psf
BC Dead	4.0 psf
TOTAL	34.0 psf
DurFacs	L=1.25 P=1.25
Rep Mbr Bnd	1.00
O.C.Spacing	2-0-0
Design Spec	UBC
Segn	03.20.98-4804

BRG	X-LOC	REFCT	SIZE	REQ'D	TOP CHRD	EL #2
1	0-2-8	535	4.95"	1.50"	HOT CHRD	2x6 EL #2
2	12-6-3	-68	1.50"	1.50"	WEB	2x4 EL #2
3	12-5-15	847	1.50"	1.50"		

Loaded for 10 PSF non-concentric B.L.L.
Interior support or temporary shoring must be in place before erecting this truss.
Shim bearings (if needed) for req. support.
PLATING BASED ON GREEN DIMENSIONAL VALUES.

TC FORCE	AXL	END	CSI
1	-1031	.00	.18
2	-78	.00	.18
3	-55	.00	.04
4	0	.00	.00

BC FORCE	AXL	END	CSI
1	920	.09	.29
2	905	.09	.35
3	0	.00	.00

WEB FORCE	CSI	WEB FORCE	CSI
2-5	537	22	3-6
2-6	-1007	.64	

Plating spec: ANSI/TPI - 1995
THIS DESIGN IS THE COMPOSITE RESULT OF MULTIPLE LOAD CASES.
PLATE VALUES PER ICGO RESEARCH REPORT #1607.
Location of interior bearings should be clearly marked on each truss.
Permanent bracing is required (by others) to prevent rotation/twisting. See HIB-91 and ANSI/TPI 1-1995; 10.3.4.5 and 10.3.4.6.

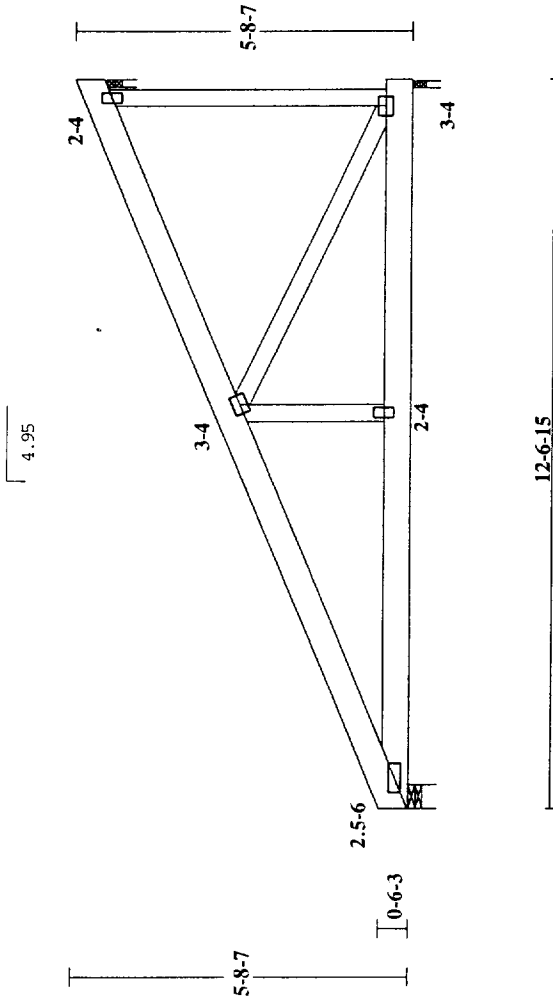
UPLIFT REACTION(S) :
support 2 -136#

Dir	L,Plf	R,Plf	L,Loc	R,Loc	L/TL
TC Vert	58.0	0	0-0-0	0-0-0	4-0-0
TC Vert	29.0	0	0-0-0	0-0-0	11-2-0
BC Vert	0	0	0-0-0	0-0-0	12-6-15
BC Vert	4.0	0	0-0-0	0-0-0	11-2-0
BC Vert	0	0	0-0-0	0-0-0	12-6-15

MAX DEFLECTION (span) :
L/999 IN MEM 5-6 (LIVE)
L = -.03" D = -.02" T = -.05"

Joint Locations

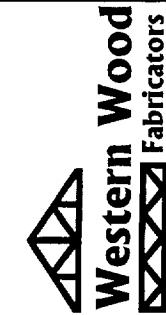
1	0-0-0	4	0-0-0
2	6-9-14	5	6-9-14
3	12-6-15	6	12-6-15



All plates are 20 gauge Truswal Connectors unless preceded by "18" for 18 gauge or "H" for 16 gauge.

OVER 3 SUPPORTS

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3700 Riego Road, Elverta, CA 95626

Eng. Job: .EJ.

Chk: CM

Dsgnr: EC

TC Live 20.0 psf

TC Dead 9.0 psf

BC Live .0 psf

BC Dead 4.0 psf

TOTAL 33.0 psf

Date: 5/13/98

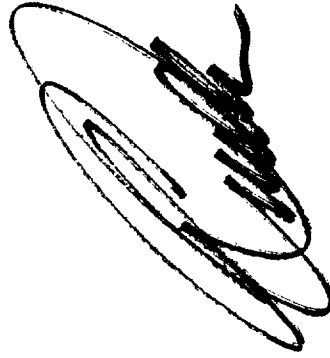
DurFacs L=1.25 P=1.25

Rep Mbr Bnd 1.00

O.C.Spacing 2-0-0

Design Spec UBC

Seqn 03.20.98- 4819



BRG	X-LOC	REFCT	SIZE	REQ'D	TOP CHORD	2x6 EL #2	EL #2
1	0'-2'-8"	416	4.95"	1.50"	TOP CHORD	2x6 EL #2	EL #2
2	11'-0'-14"	-47	1.50"	1.50"	WEB	2x4 EL STANDARD	EL #2
3	11'-0'-9"	638	1.50"	1.50"	WEB	2x4 EL STANDARD	EL #2

Loaded for 10 PPF in-concurrent BOLL.
Interior support or temporary shoring must be in place before erecting this truss.
Slab bearings (if needed) for req. support.
PLATING BASED ON GREEN LUMBER VALUES.

TC	FORCE	AXL	END	CSI
1	-784	.00	.13	.13
2	-67	.00	.13	.13
3	-39	.00	.03	.03
4	0	.00	.00	.00

BC	FORCE	AXL	END	CSI
1	697	.06	.17	.24
2	686	.06	.22	.28
3	0	.00	.00	.00

WEB	FORCE	CSI	WEB	FORCE	CSI
2-5	372	.15	3-6	-186	.06
2-6	-767	.37			

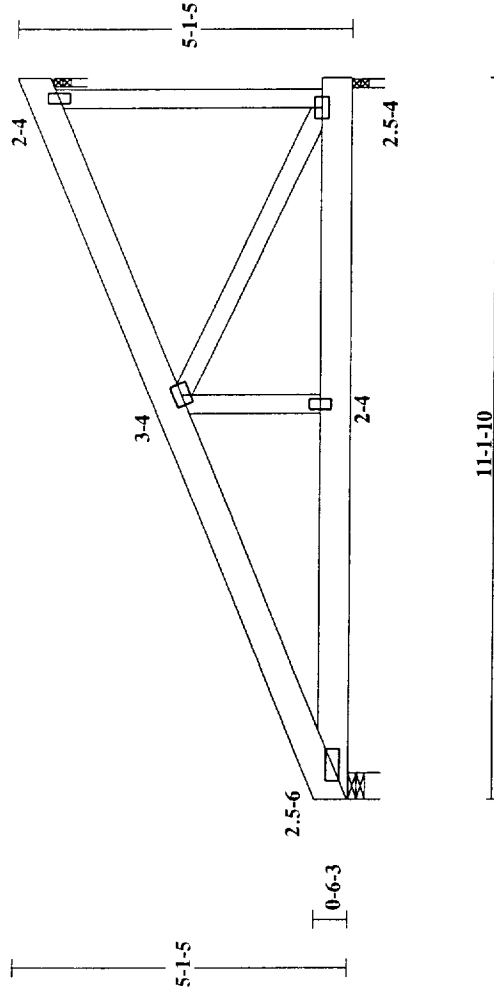
Plating spec: ANS/TPI - 1995
THIS DESIGN IS THE COMPOSITE RESULT OF MULTIPLE LOAD CASES.
PLATE VALUES PER JOBO RESEARCH REQ'RT #1607.
Location of interior bearings should be clearly marked on each truss.
Permanent bracing is required (by others) to prevent rotation/toppling. See HB-91 and ANS/TPI 1-1995; 10.3.4.5 and 10.3.4.6.

UPLIFT REACTION(S) :
Support 2
-94#

Dir	L.Plf	L.Loc	R.Loc	LL/TL
TC Vert	58.0	4'-0"-0"	4'-0"-0"	69
TC Vert	58.0	4'-0"-0"	9'-8"-11"	69
TC Vert	29.0	9'-8"-11"	11'-1"-10"	69
BC Vert	95.6	0'-0"-0"	9'-8"-11"	69
BC Vert	4.0	9'-8"-11"	11'-1"-10"	.00

MAX DEFLECTION (span) :
L/999 IN MEM 5-6 (LIVE)
L_F = -.02" D_F = -.01" T_F = -.03"

Joint Locations	1	2	3	4	5	6
1	0'-0"-0"	4	0'-0"-0"			
2	6'-1'-3"	5	6'-1'-3"			
3	11'-1"-10"	6	11'-1"-10"			



All plates are 20 gauge Truswal Connectors unless preceded by "18" for 18 gauge or "H" for 16 gauge.

OVER 3 SUPPORTS

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Western Wood Fabricators
3700 Riego Road, Elverta, CA 95626

Eng. Job: .EJ.

Chk: CM

Dsgnr: EC

TC Live	20.0 psf
TC Dead	9.0 psf
BC Live	.0 psf
BC Dead	4.0 psf
TOTAL	33.0 psf

WO: J8140

Date: 5/13/98

DurFacs L=1.25 P=1.25

Rep Mbr End 1.00

O.C.Spacing 2'-0"-0"

Design Spec UBC

Seqn 03.20.98-4805

Job Name: CARLS JR. 14DC

Truss ID: EJ2

Drwg: EJ2

REQ	X-LOC	REACT	SIZE	REQ'D	TOP CHRD	2x6	FL #2
1	0-11-2	69	3-50"	1-50"	Interior support or temporary shoring must be in place before erecting this truss.	2x4	FL #2
2	1-11-4	53	1-50"	1-50"	PLATING BASED ON GREEN LUMBER VALUES.		
3	1-11-4	10	1-50"	1-50"			

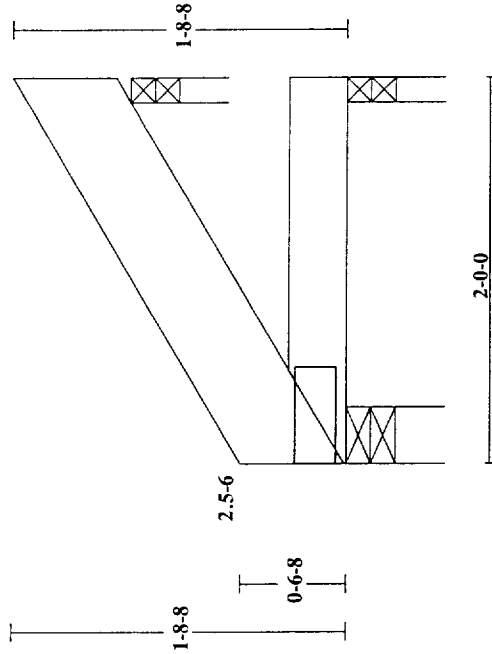
TC FORCE	AXL	END	CSI
1	24	.00	.02
2	-1	.00	.00

BC FORCE	AXL	END	CSI
1	0	.00	.01
2	0	.00	.00

Plating spec : ANSI/TPI - 1995
 PLATE VALUES PER ICHO RESEARCH REPORT #1607.
 Location of interior bearings should be clearly marked on each truss.
 Shim bearings (if needed) for req. support.

MAX DEFLECTION (span) :
 L/999 IN MEM 3-4 (LIVE)
 L= .00" D= .00" T= .00"

Joint Locations	
1	0-0-0 3 0-0-0
2	2-0-0 4 2-0-0



All plates are 20 gauge Truswal Connectors unless preceded by "18" for 18 gauge or "H" for 16 gauge.

OVER 3 SUPPORTS

Western Wood Fabricators
 3700 Riego Road, Elverta, CA 95626

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Eng. Job: .EJ.
 Chk: CM
 Dgnr: EC

Date: 5/13/98	
TC Live	20.0 psf
TC Dead	9.0 psf
BC Live	.0 psf
BC Dead	4.0 psf
TOTAL	33.0 psf

WO: J8140

Date: 5/13/98

DurFacs	L=1.25	P=1.25
Rep Mbr End	1.15	
O.C.Spacing	2- 0- 0	
Design Spec	UBC	
Seqn	03.20.98-	4815

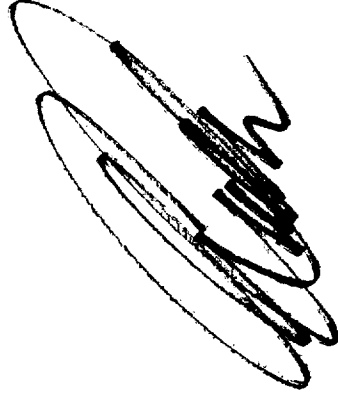
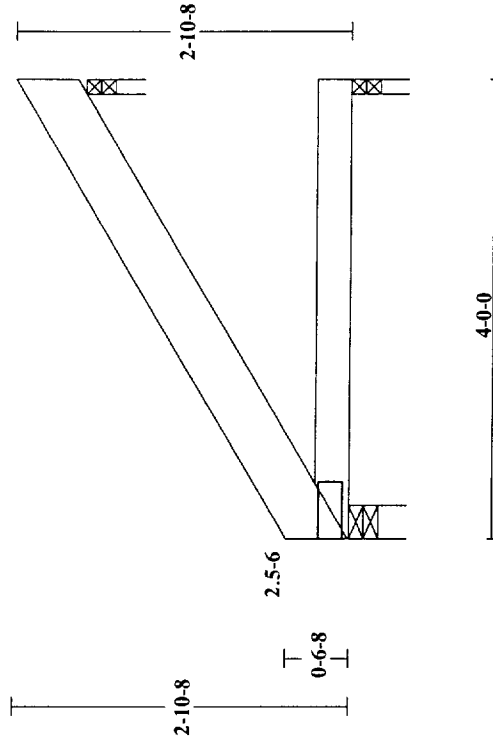
FRG	X-LOC	REFCT	SIZE	REQ'D	TOP CHRD	2x6	EL #2
1	0-1-12	135	3.50"	1.50"	2x4	EL #2	
2	3-11-4	110	1.50"	1.50"	Interior support or temporary shoring must be in place before erecting this truss.		
3	3-11-4	19	1.50"	1.50"	PLATING BASED ON GREEN LIVER VALUES.		

TC	FORCE	AXL	END	CSI
1	53	.00	.09	.09
2	-1	.00	.00	.00

BC	FORCE	AXL	END	CSI
1	0	.00	.05	.05
2	0	.00	.00	.00

Plating spec : ANSI/AISI - 1995
 PLATE VALUES PER ICBO RESEARCH REPORT #1607.
 Location of interior bearings should be clearly marked on each truss.
 Stum bearings (if needed) for req. support.

MAX DEFLECTION (span) :
 L/999 IN MEM 3-4 (LIVE)
 L/ .00" D= .00" T= -.01"
 --- Joint Locations ---
 1 0-0-0 3 0-0-0
 2 4-0-0 4 4-0-0



All plates are 20 gauge Truswal Connectors unless preceded by "18" for 18 gauge or "H" for 16 gauge.

OVER 3 SUPPORTS

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Western Wood Fabricators
 3700 Riego Road, Elverta, CA 95626

Eng. Job: .EJ.	WO: J8140
Chk: CM	Date: 5/13/98
Dsgnr: EC	
TC Live	20.0 psf
TC Dead	9.0 psf
BC Live	.0 psf
BC Dead	4.0 psf
TOTAL	33.0 psf
Rep Mbr End	1.15
O.C.Spacing	2- 0 - 0
Design Spec	UBC
Seqn	03.20.98 - 4814

LEG	X-LOC	REACT	SIZE	REC'D	TOP CHORD	2x6	EL #2
1	0'-11-12	201	3.50"	1.50"	BOT CHORD	2x4	EL #2
2	5'-11-4	-110	1.50"	1.50"	WEB	2x4	EL STANDARD
3	5'-11-4	492	1.50"	1.50"			

Loaded for 10 PSF non-concurrent BOLL.
Interior support or temporary shoring must be in place before erecting this truss.
PLATING BASED ON GREEN LOWER VALUES.

TC	FORCE	AXL	END	CSI
1	-107	.00	.17	.17
2	-158	.00	.07	.07
3	-1	.00	.00	.00

BC	FORCE	AXL	END	CSI
1	33	.01	.09	.09
2	0	.00	.00	.00

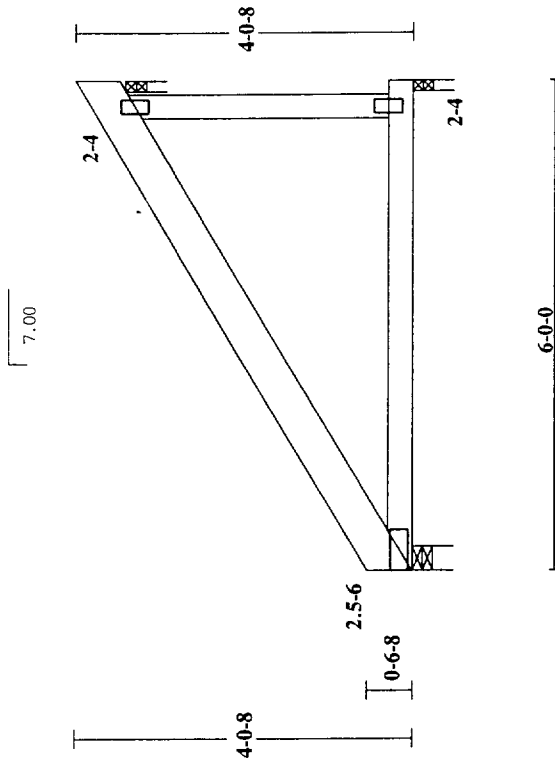
WEB	FORCE	CSI	WEB	FORCE	CSI
2-4	-467	.10			

UPLIFT REACTION(S) :
support 2 -297#

Plating spec : ANSI/TPI - 1995
THIS DESIGN IS THE COMPOSITE RESULT OF MULTIPLE LOAD CASES.
PLATE VALUES PER ICBO RESEARCH REPORT #1607.
Location of interior bearings should be clearly marked on each truss.
Shim bearings (if needed) for req. support.

MAX DEFLECTION (span) :
1/999 IN MEM 3-4 (LIVE)
L_r = .02" D = .01" T = -.03"

Joint Locations
1 0-0-0 3 0-0-0
2 6-0-0 4 6-0-0



All plates are 20 gauge Truswal Connectors unless preceded by "18" for 18 gauge or "H" for 16 gauge.

OVER 3 SUPPORTS

Western Wood Fabricators
3700 Riego Road, Elverta, CA 95626

WARNING Read all notes on this sheet and give a copy of it to the Erecting Contractor.
This design is for an individual building component. It has been based on specifications provided by the component manufacturer and done in accordance with the current versions of TPI and AFPA design standards. No responsibility is assumed for dimensional accuracy. Dimensions are to be verified by the component manufacturer and/or building designer prior to fabrication. The building designer shall ascertain that the loads utilized on this design meet or exceed the loading imposed by the local building code. It is assumed that the top chord is laterally braced by the roof or floor sheathing and the bottom chord is laterally braced by a rigid sheathing material directly attached, unless otherwise noted. Bracing shown is for lateral support of components members only to reduce buckling length. This component shall not be placed in any environment that will cause the moisture content of the wood exceed 19% and/or cause connector plate corrosion. Fabricate, handle, install and brace this truss in accordance with the following standards: "TRUSCOM MANUAL", by Truswal, "QUALITY CONTROL STANDARD FOR METAL PLATE CONNECTED WOOD TRUSSES", (QST-88), "HANDLING INSTALLING AND BRACING METAL PLATE CONNECTED WOOD TRUSSES", (HIB-91) and "HIB-91 SUMMARY SHEET" by TPI. The Truss Plate Institute (TPI) is located at 583 D'Onofrio Drive, Madison, Wisconsin 53719. The American Forest and Paper Association (AFPA) is located at 1250 Connecticut Ave, NW, Ste 200, Washington, DC 20036.

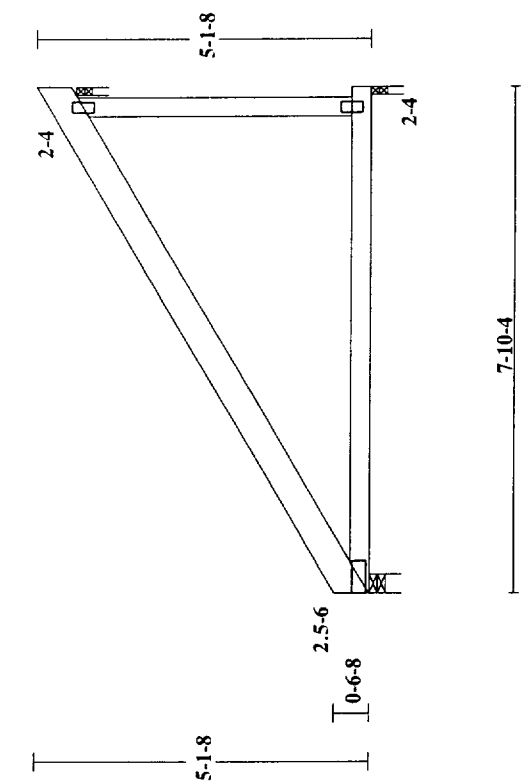
Eng. Job: .EJ.	WO: JB140
Chk: CM	Date: 5/13/98
Dsgnr: EC	DurFacs L=1.25 P=1.25
TC Live 20.0 psf	Rep Mbr Bnd 1.15
TC Dead 9.0 psf	O.C.Spacing 2-0-0
BC Live .0 psf	Design Spec UBC
BC Dead 4.0 psf	TOTAL 33.0 psf
	Seqn 03.20.98- 4812

Plating spec : ANSI/TPI - 1995
 THIS DESIGN IS THE COMPOSITE RESULT OF
 MULTIPLE LOAD CASES.
 PLATE VALUES PER ICB0 RESEARCH REPORT #1607.
 Location of interior bearings should be
 clearly marked on each truss.
 Shim bearings (if needed) for req. support.

UPLIFT REACTION(S) :
 support 2 -604#

MAX DEFLECTION (span) :
 L/999 IN MEM 3-4 (LIVE)
 L_c = .06" D = -.04" T = -.10"

Joint Locations
 1 0-0-0 3 0-0-0
 2 7-10-4 4 7-10-4



All plates are 20 gauge Truswal Connectors unless preceded by "18" for 18 gauge or "H" for 16 gauge.

OVER 3 SUPPORTS

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Eng. Job: .EJ.	WO: J8140
Chk: CM	Date: 5/13/98
Dsgnr: EC	DurFace L=1.25 P=1.25
TC Live 20.0 psf	Rep Mbr Bnd 1.15
TC Dead 9.0 psf	O.C.Spacing 2- 0- 0
BC Live .0 psf	Design Spec UBC
BC Dead 4.0 psf	
TOTAL 33.0 psf	Segn 03.20.98- 4822

Western Wood Fabricators
 3700 Riego Road, Elverta, CA 95626

ENG	X-LOC	REACT	SIZE	REVD
1	0-1-12	267	3-50"	1-50"
2	7-11-4	-256	1-50"	1-50"
3	7-11-4	894	1-50"	1-50"

TC	FORCE	AXL	END	CSI
1	-147	.00	.31	.31
2	-328	.00	.16	.16
3	-1	.00	.00	.00

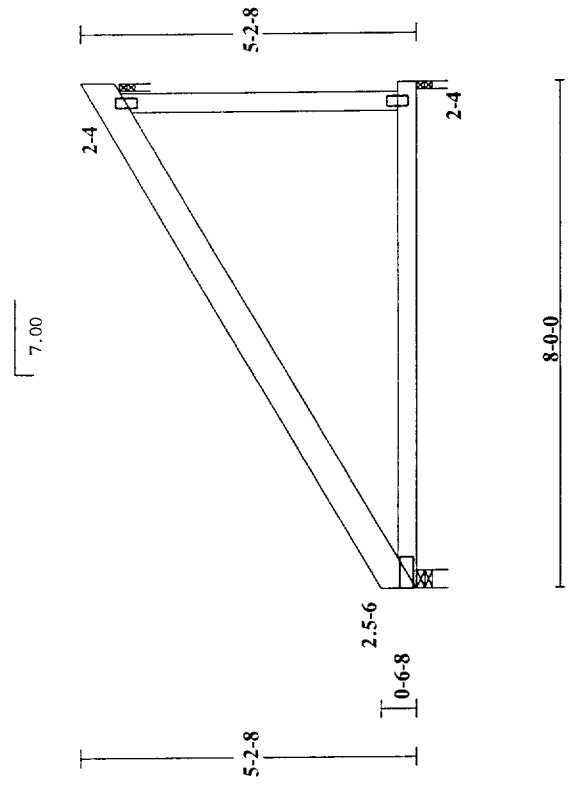
BC	FORCE	AXL	END	CSI
1	46	.00	.20	.20
2	0	.00	.00	.00

WEB	FORCE	CSI	WEB	FORCE	CSI
2-4	-860	.31			

Plating spec: ANSI/TPI - 1995
 THIS DESIGN IS THE COMPOSITE RESULT OF
 MULTIPLE LOAD CASES.
 PLATE VALUES PER ICGO RESEARCH REPORT #1607.
 Location of interior bearings should be
 clearly marked on each truss.
 Shim bearings (if needed) for req. support.

UPLEFT REACTION(S) :
 support 2 - -633#

MAX DEFLECTION (span) :
 L/999 IN MEM 3-4 (LIVE)
 L_c = -.07" D_c = -.04" T_c = -.11"
 Joint Locations
 1 0-0-0 3 0-0-0
 2 8-0-0 4 8-0-0



All plates are 20 gauge Truswal Connectors unless preceded by "18" for 18 gauge or "H" for 16 gauge.

OVER 3 SUPPORTS

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Eng. Job: .EJ.	NO: J8140
Chk: CM	Date: 5/13/98
Dsgnr: EC	DurFacs L=1.25 P=1.25
TC Live 20.0 psf	Rep Mbr Bnd 1.15
TC Dead 9.0 psf	O.C.Spacing 2- 0- 0
BC Live .0 psf	Design Spec UBC
BC Dead 4.0 psf	Seqm 03.20.98-
TOTAL 33.0 psf	4813