

**CITY OF SACRAMENTO**

1231 I Street, Sacramento, CA 95814

Permit No: 0403195

Insp Area: 1

Thos Bros: 298A5

Site Address: 604 DITTMAR WY SAC

Parcel No: 005-0213-005

Sub-Type: ASFR

Housing (Y/N): N

CONTRACTOR

OWNER

CRAIG JOHNSON  
604 DITTMAR WY  
SACTO, CA 95819

ARCHITECT

Nature of Work: 332 sf family rm & remodel kitchen/porch, truss roof

**CONSTRUCTION LENDING AGENCY:** I hereby affirm under penalty of perjury that there is a construction lending agency for the performance of the work for which this permit is issued (Sec. 3097, Civ. C).

Lender's Name \_\_\_\_\_ Lender's Address \_\_\_\_\_

**LICENSED CONTRACTORS DECLARATION:** I hereby affirm under penalty of perjury that I am licensed under provisions of Chapter 9 (commencing with section 7000) of Division 3 of the Business and Professions Code and my license is in full force and effect.

License Class \_\_\_\_\_ License Number \_\_\_\_\_ Date \_\_\_\_\_ Contractor Signature \_\_\_\_\_

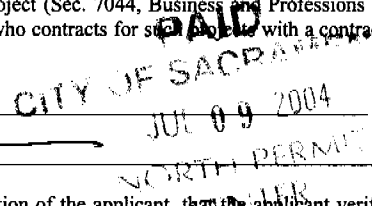
**OWNER-BUILDER DECLARATION:** I hereby affirm under penalty of perjury that I am exempt from the contractors License Law for the following reason (Sec. 7031.5, Business and Professions Code; any city or county which requires a permit to construct, alter, improve, demolish, or repair any structure, prior to its issuance, also requires the applicant for such permit to file a signed statement that he or she is licensed pursuant to the provisions of the Contractors License Law (Chapter 9 (commencing with Section 7000) of Division 8 of the Business and Professions Code) or that he or she is exempt therefrom and the basis for the alleged exemption. Any violation of Section 7031.5 by any applicant for a permit subjects the applicant to a civil penalty of not more than five hundred dollars (\$500.00);

I, as a owner of the property, or my employees with wages as their sole compensation, will do the work, and the structure is not intended or offered for sale (Sec. 7044, Business and Professional Code: The Contractors License Law does not apply to an owner of property who builds or improves thereon, and who does such work himself or herself or through his/her own employees, provided that such improvements are not intended or offered for sale. If, however, the building or improvement is sold within one year of completion, the owner-builder will have the burden of proving that he/she did not build or improve for the purpose of sale.)

I, as owner of the property, am exclusively contracting with licensed contractors to construct the project (Sec. 7044, Business and Professions Code: The Contractors License Law does not apply to an owner of property who builds or improves thereon, and who contracts for such work with a contractor(s) licensed pursuant to the Contractors License Law).

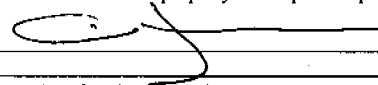
I am exempt under Sec. \_\_\_\_\_ B & PC for this reason: \_\_\_\_\_

Date 7-9-04 Owner Signature 



**IN ISSUING THIS BUILDING PERMIT,** the applicant represents, and the city relies on the representation of the applicant, that the applicant verified all measurements and locations shown on the application or accompanying drawings and that the improvement to be constructed does not violate any law or private agreement relating to permissible or prohibited locations for such improvements. This building permit does not authorize any illegal location of any improvement or the violation of any private agreement relating to location of improvements.

I certify that I have read this application and state that all information is correct. I agree to comply with all city and county ordinances and state laws relating to building construction and hereby authorize representative(s) of this city to enter upon the abovementioned property for inspection purposes.

Date 7-9-04 Applicant/Agent Signature 

**WORKER'S COMPENSATION DECLARATION:** I hereby affirm under penalty of perjury one of the following declarations:

I have and will maintain a certificate of consent to self-insure for workers' compensation as provided for by Section 3700 of the Labor Code, for the performance of work for which the permit is issued.

I have and will maintain workers' compensation insurance, as required by Section 3700 of the Labor Code, for the performance of the work for which this permit is issued. My workers' compensation insurance carrier and policy number are:

Carrier \_\_\_\_\_ Policy Number \_\_\_\_\_ Exp Date \_\_\_\_\_

(This section need not be completed if the permit is for \$100 or less) I certify that in the performance of the work for which this permit is issued, I shall not employ any person in any manner so as to become subject to the workers' compensation laws of California and agree that if I should become subject to the workers' compensation provisions of Section 3700 of the Labor Code, I shall forthwith comply with those provisions.

Date 7-9-04 Applicant Signature 

**WARNING: FAILURE TO SECURE WORKER'S COMPENSATION COVERAGE IS UNLAWFUL AND SHALL SUBJECT AN EMPLOYER TO CRIMINAL PENALTIES AND CIVIL FINES UP TO ONE HUNDRED THOUSAND DOLLARS (\$100,000) IN ADDITION TO THE COST OF COMPENSATION, DAMAGES AS PROVIDED FOR IN SECTION 3706 OF THE LABOR CODE, INTEREST AND ATTORNEY'S FEE.**

**THIS PERMIT SHALL EXPIRE BY LIMITATION IF WORK IS NOT COMMENCED WITHIN 180 DAYS.**

Department of Planning and Development  
Building Inspection Division

Grading and Erosion Control Questionnaire

To be completed for all residential new construction and additions

**PART I (To be completed by applicant)**

Site Address 604 DITTMAR WAY A.P.N. 005-0213-005

Applicant Information

Name CRAMA JOHNSON  
Address 604 DITTMAR WAY  
SACRAMENTO CA 95819  
Phone 916-947-1483

Project Information (Check One)

Single Family Dwelling  **X**  
Duplex   
Triplex   
Deep Lot Development

**PART II (To be completed by the applicant when the project is not a part of a larger subdivision)**

Are there existing structures on site?  Y  N  
Does the site front on a paved road?  Y  N \*  
Is the site higher than the crown of adjacent road?  Y  N \*  
Is the proposed building site higher than the back of the sidewalk or curb?  Y  N \*  
Describe existing frontage improvements along road.  
 Ditch \*  Curb and Gutter  Curb, Gutter, and Sidewalk  
The direction of drainage on this site is:  
 Front to Rear \*  Rear to Front  Side to Side \*  
Does an adjacent site drain across this parcel?  Y \*  N  
Does this site have an existing low area or drainage swale?  Y \*  N  
Will construction require cut or fill on site? (\* >50FT3 or >2FT)  
- How much cut? \_\_\_\_\_ Yards \_\_\_\_\_ Depth  
- How much fill? \_\_\_\_\_ Yards \_\_\_\_\_ Depth  
Has building site been previously been filled?  Y \*  N  
Will existing drainage be re-routed?  Y \*  N  
Do you plan to construct or modify culverts or drainage ditches?  Y \*  N

Print Name CRAMA JOHNSON Title OWNER

Signature [Signature] Date 6.18.04  
Owner or Contractor

**PART III (To be completed by staff)**

What is the acreage of the parcel to be built on? \_\_\_\_\_ Acres.  
If greater than 1/2 acre has an approved erosion and sediment control plan been provided?  Y  N  
If greater than 5 acres has the applicant provided a copy of the State General Permit NOI and the SWPPP?  Y  N  
Is the parcel to be built on part of a larger subdivision?  Y  N  
Subdivision Name: \_\_\_\_\_  
If yes has an approved erosion and sediment control plan been provided?  Y  N  
If the original subdivision is greater than 5 acres has the applicant provided a copy of the State General Permit NOI and the SWPPP?  Y  N  
Is grading and drainage approval required prior to permit issuance?  Y  N  
Approved by: S. Boyd Date: 7-9-04  
Building permit #: 0403195

NOTE: ADDITION DOESN'T IMPACT YARD DRAINAGE  
MICROFILM THIS DOCUMENT

White Copy - Permit Jacket  
Yellow - Utilities  
Pink - Bldg. Div.

REVISION ON ACTIVE PERMIT

NEW PLAN CHECK NO#: 096687  
 OLD PLAN CHECK NO#: 0903195

DATE: 10/5/04

This sheet is to be used only when a permit has been issued, is still active, and the applicant wishes to make changes to the existing approved plans.

All revisions clouded? YES \_\_\_\_\_ NO \_\_\_\_\_

JOB ADDRESS \_\_\_\_\_ SUITE \_\_\_\_\_ PERMIT NO \_\_\_\_\_

AREA: \_\_\_\_\_ DBA: \_\_\_\_\_

DESCRIPTION OF REVISIONS Fire place Extension

DISCIPLINE	<u>B</u>	L	P	M	E	F	S	R	D
CHECKED BY									
ROUTE TO									
CODE									
HOURS SPENT									

CONTACT: CRAIG JOHNSON

ADDRESS: 604 DITMAR WAY SAC CA 95819

PHONE#: 916 947 1483

# OF PLANS SUBMITTED \_\_\_\_\_ SUBMITTED TO TJO

I understand that I am responsible for all plan check fees that I incur during the course of this additional plan check and that any approved plans not claimed and paid for within 3 months of notification will be disposed of and an invoice procedure for the amount due will be initiated. I further understand that an unclaimed revision may result in delay of final approval for the subject project.

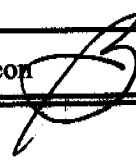
DATE NOTIFIED	PLAN BIN

APP FEE	PAID

\_\_\_\_\_  
 Applicant signature Date

AGENCY	TOTAL HRS	TOTAL FEES
BLDG		
PW		
PLEASE PAY THIS AMOUNT		

City of Sacramento Planning Division  
**PLANNING REVIEW FOR BUILDING PERMIT SUBMITTAL**

ADDRESS: 604 Dittmar Way	APN: 005-0213-005
DRPB AREA / PUD / SPD: None	ZONING: R-1
EXISTING LAND USE: <del>RSF</del>	
PROPOSED USE: Addition	
<b>PLANNING STAFF WILL CHECK ONE OR MORE OF THE ITEMS BELOW:</b>	
<input type="checkbox"/>	Planning review is <b>NOT</b> required.
<input type="checkbox"/>	Use is <b>NOT</b> allowed; applicant <b>CANNOT</b> submit for plan check.
<input type="checkbox"/>	Requires APPLICATION(s):    PC        ZA        IR        ER        DR        PB Required Planning application must be submitted <i>before</i> project can be submitted for plan check.
<input type="checkbox"/>	<b>Application(s) IN PROGRESS:</b> Applicant may submit for concurrent building permit plan check, at applicant's risk. Building Division must check with Planning staff and/or SITE before issuing building permit.
<input type="checkbox"/>	<b>Application(s) COMPLETED:</b> Building permit must conform to approved plans and comply with all conditions of approval. Do NOT issue building permit prior to end of 10 day appeal period.
<input type="checkbox"/>	Plans may be submitted for plan check. Plan checker(s) shall confirm compliance with Zoning Ordinance requirements and all applicable development standards <i>prior to issuance</i> of building permit.
<input checked="" type="checkbox"/>	Meets setback & lot coverage requirements as shown on site plan provided.
<input checked="" type="checkbox"/>	Plans to be submitted have been stamped/signed by Planning counter staff.
<input type="checkbox"/>	Route to SITE for plan check and inspection.
<input type="checkbox"/>	Preliminary review <b>ONLY</b> ; the information on this form must be reviewed again and confirmed at the time of building permit submittal.
COMMENTS: Lot area = 11,140 (Metroscan). Lot coverage = 1408 (existing) + 332 (proposed) = 1740 1740 / 11,140 = 16% total lot coverage. Meets all setback and lot coverage requirements. No additional Planning Entitlements required.	
DATE: 03/04/04	BY: Bonnie Surgeon 

Department of Planning and Development  
Building Inspection Division

Grading and Erosion Control Questionnaire

To be completed for all residential new construction and additions

PART I (To be completed by applicant)

Site Address 604 DITTMAR WAY A.P.N. 005-0213-005

Applicant Information

Name CRALA JOHNSON  
Address 604 DITTMAR WAY  
SACRAMENTO CA 95819  
Phone 916-947-1483

Project Information (Check One)

Single Family Dwelling  X  
Duplex    
Triplex    
Deep Lot Development

PART II (To be completed by the applicant when the project is not a part of a larger subdivision)

- Are there existing structures on site?  Y  N
- Does the site front on a paved road?  Y  N\*
- Is the site higher than the crown of adjacent road?  Y  N\*
- Is the proposed building site higher than the back of the sidewalk or curb?  Y  N\*
- Describe existing frontage improvements along road.
  - Ditch \*
  - Curb and Gutter
  - Curb, Gutter, and Sidewalk
- The direction of drainage on this site is:
  - Front to Rear \*
  - Rear to Front
  - Side to Side \*
- Does an adjacent site drain across this parcel?  Y\*  N
- Does this site have an existing low area or drainage swale?  Y\*  N
- Will construction require cut or fill on site? (\* >50FT3 or >2FT)
  - How much cut? \_\_\_\_\_ Yards Depth  Y\*  N
  - How much fill? \_\_\_\_\_ Yards Depth  Y\*  N
- Has building site been previously been filled?  Y\*  N
- Will existing drainage be re-routed?  Y\*  N
- Do you plan to construct or modify culverts or drainage ditches?  Y\*  N

Print Name CRALA JOHNSON Title OWNER

Signature [Signature] Date 6-18-04  
Owner or Contractor

PART III (To be completed by staff)

What is the acreage of the parcel to be built on? \_\_\_\_\_ Acres.

If greater than 1/2 acre has an approved erosion and sediment control plan been provided?  Y  N

If greater than 5 acres has the applicant provided a copy of the State General Permit NOI and the SWPPP?  Y  N

Is the parcel to be built on part of a larger subdivision?  Y  N  
Subdivision Name: \_\_\_\_\_

If yes has an approved erosion and sediment control plan been provided?  Y  N

If the original subdivision is greater than 5 acres has the applicant provided a copy of the State General Permit NOI and the SWPPP?  Y  N

Is grading and drainage approval required prior to permit issuance?  Y  N

Approved by: G. Boyd Date: \_\_\_\_\_

Building permit #: 0403195

OFFICE COPY

NOTE: ADDITION DOESN'T IMPACT YARD DRAINAGE  
MICROFILM THIS DOCUMENT

White Copy - Permit Jacket  
Yellow - Utilities  
Pink - Bldg. Div.



**CITY OF SACRAMENTO  
BUILDING INSPECTION  
DIVISION**

**PERMIT OFFICES**  
Downtown (916) 264-7619  
1231 I St., Rm. 200, Sacramento 95814  
Natomas Center (916) 808-2534  
2101 ARENA BL., Sacramento 95834  
<http://www.sacto.org>

**RESIDENTIAL PLAN REVIEW  
2001 CBC Adopted Codes  
Effective November 1<sup>st</sup>, 2002**

**PROJECT ADDRESS  
& DESCRIPTION**

604 DITTMAR WY. ADDITION

**PERMIT**

**No. 0403195**

These sheets, when attached to a set of plans, become part of those plans and must remain attached thereto. The approval of this plan and the specifications shall not be held to permit or approve the violation of any City ordinance or State or Federal law. (Note: Authorized agent must provide a letter from Owner verifying Authorization.) The code requirements circled do not limit the code requirements for this project.

I have read and will comply with the items in this document and as marked on the plans.

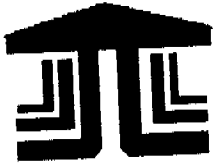
Signature of: [Signature]  Owner  Authorized Agent  Contractor  Architect/Engineer

Date 6-18-02

**BUILDING CODE REQUIREMENTS**

- B-1 Smoke detector location within dwelling units.** In dwelling units, a detector shall be installed in each sleeping room and at a point centrally located in the corridor or area giving access to each separate sleeping area. When the dwelling unit has more than one story and in dwellings with basements, a detector shall be installed on each story and in the basement. In dwelling units where a story or basement is split into two or more levels, the smoke detector shall be installed on the upper level except that, when the lower level contains a sleeping area, a detector shall be installed on each level. When sleeping rooms are on an upper level, the detector shall be placed at the ceiling of the upper level in close proximity to the stairway. In dwelling units where the ceiling height of a room open to the hallway serving the bedrooms exceeds that of the hallway by 24 inches (610 mm) or more, smoke detectors shall be installed in the hallway and in the adjacent room. Detectors shall sound an alarm audible in all sleeping areas of the dwelling unit in which they are located. In new construction, required smoke detectors shall receive their primary power from a commercial source and have a battery back up. 2001 CBC, Section 310.9.1.
- B-2** When alteration, repairs, or additions having a value in excess of \$1,000 are made, provide an approved smoke detector to protect existing sleeping rooms. The detector may be battery operated as per 2001 CBC, Section 310.9.1.2.  
**Exception:** Repairs to the exterior surfaces of a Group R occupancy are exempt from the requirements of this section.
- B-3 Emergency escape and rescue.** Basements in dwelling units and every sleeping room below the fourth story shall have at least one operable window or door approved for emergency escape or rescue that shall open directly into a public street, public way, yard, or exit court. Escape or rescue windows shall have a minimum net clear openable area of 5.7square feet / 821 SQ. inches. The minimum net clear openable height dimension shall be 24 inches. The minimum net clear openable width dimension shall be 20 inches. Emergency escape or rescue windows shall have a finished sill height not more than 44 inches above the floor. 2001 CBC, Section 310.4.
- B-4 All Group U occupancies attached to Group R, Division 3 occupancies shall be separated by** materials approved for one-hour fire-resistive construction. The separation may be limited to the garage side only and requires a self-closing, tight fitting solid wood door 1 3/8 inches in thickness or a self-closing, tight fitting door having a fire protection rating of not less than 20 minutes. CBC, Section 302. **Exception:** Section 3. **Note:** All members supporting such separation shall be equivalent fire-resistive construction as per 2001 UBC, Section 302. All electrical outlet boxes on opposite sides of the wall shall be separated by a horizontal distance on not less than 24 inches per 2001 CBC 709.7, Exception 1.

**OFFICE COPY**



# LATHAM TRUSS, INC.

Mare Island • Bldg. 507 • P.O. Box 2094 • Vallejo, CA 94592  
Ph (707) 562-3540 • Fax (707) 562-3542

## ENGINEERING DRAWING PACKAGE

Customer: Craig Johnson CITY OF SACRAMENTO  
 Project Name: Johnson Addition NORTH PERMIT  
2374 CENTER  
 Job # 2374 OCT 04 2004  
 Job Address: 604 Dittmar Way, Sacramento, CA 95819

**RECEIVED**

NOTES:

- ALL LATERAL BRACING SPECIFIED IS FOR BRACING COMPRESSION WEB MEMBERS AND MUST BE INSTALLED. TOP CHORDS ARE ASSUMED TO BE LATERALLY RESTRAINED BY PLYWOOD OR SPACED SHEATING. BOTTOM CHORDS TO BE LATERALLY REINFORCED BY MINIMUM OF 1x4 AT 10'-0" O.C. MAXIMUM, EXCEPT IF OTHERWISE NOTED ON DRAWING.
- VERIFICATION OF LOADING, DEFLECTION LIMITATIONS, FRAMING METHODS, WIND BRACING OR OTHER LATERAL BRACING THAT IS ALWAYS REQUIRED, IS THE RESPONSIBILITY OF THE PROJECT ARCHITECT OR ENGINEER.
- DUE TO VARIATIONS IN WEATHER, LUMBER DIMENSIONS, AND MOISTURE CONTENT AT THE TIME OF FABRICATION, LATHAM TRUSS CANNOT BE RESPONSIBLE FOR ANY TRUSS DIMENSION VARIANCE OF 1/4" OR LESS.

**DO NOT CUT OR ALTER TRUSSES**



This set of plans and specifications must be kept on the job at all times and it is unlawful to make any changes or alterations from the same without written permission from the Building Inspection Division.

The approval of this plan and specification SHALL NOT be held to permit or approve the violation of any City Ordinance or State Law.

QUALITY INSPECTION #  
NER-QA430

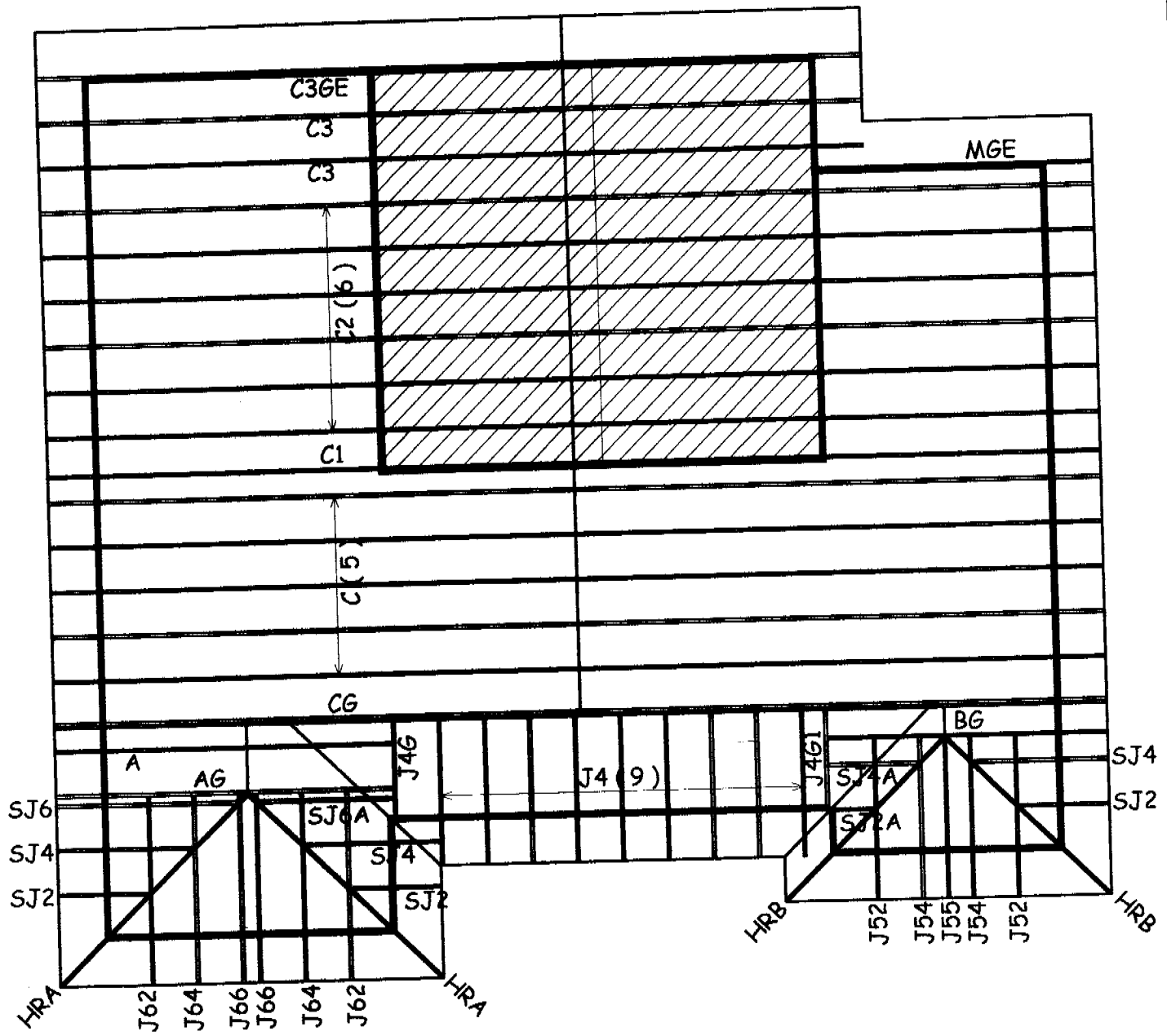


**OFFICE COPY**

Revised 10/5/04

# 0403195

ISSUED  
City of Sacramento  
JUL 11 2004  
NORTH PERMIT  
CENTER



Client:

**CRAIG JOHNSON**

Telephone

Fax

Site Information:

**JOHNSON ADDITION  
604 DITTMAR WAY  
SACRAMENTO**

**Latham Truss, Inc.**

P.O. Box 2094 Mare Island  
Vallejo, CA 94592

TEL: 707.532.2512

Re: 2374  
JOHNSON ADDITION

The truss drawing(s) referenced below have been prepared by MiTek Industries, Inc. under my direct supervision based on the parameters provided by Latham Truss Inc..

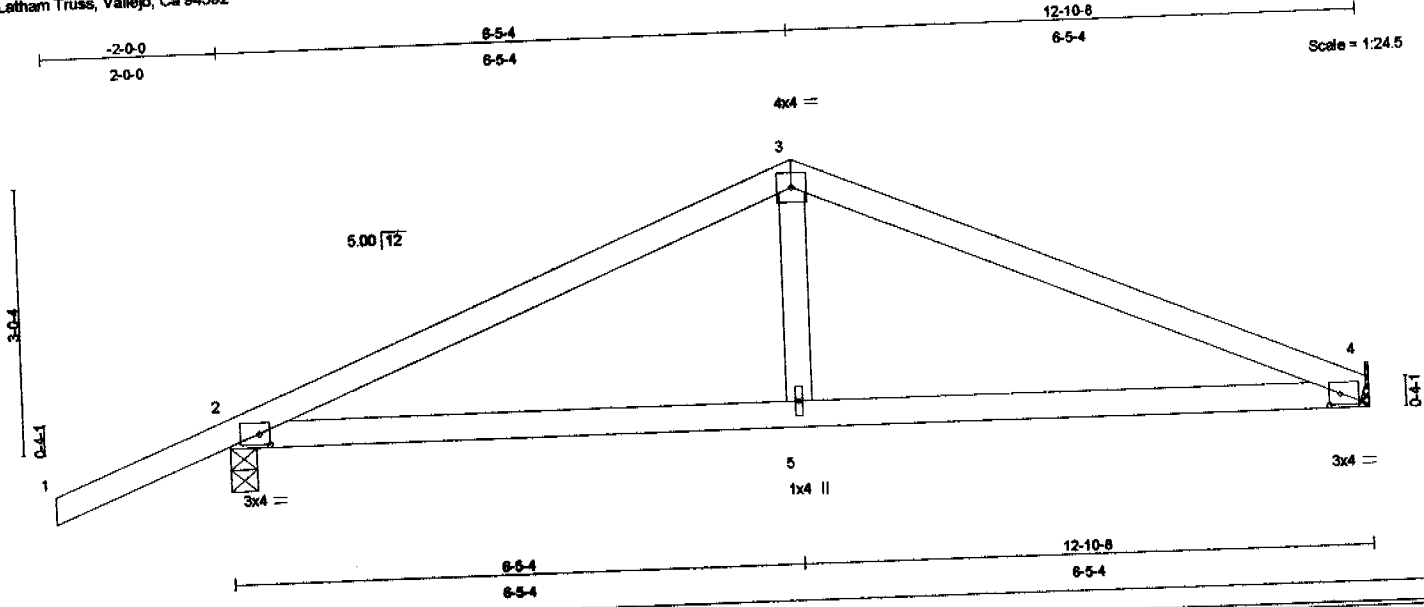
Pages or sheets covered by this seal: R13676976 thru R13676986  
My license renewal date for the state of California is June 30, 2005.



September 24, 2004

Anderson, Bob

The seal on these drawings indicate acceptance of professional engineering responsibility solely for the truss components shown. The suitability and use of this component for any particular building is the responsibility of the building designer, per ANSI/TPI-2002 Chapter 2.



LOADING (psf)		SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL	16.0	Plates Increase	1.25	TC	0.41	Vert(LL)	-0.03	2-5	>999	360
TCDL	14.0	Lumber Increase	1.25	BC	0.32	Vert(TL)	-0.06	2-5	>999	180
BCLL	0.0	Rep Stress Incr	YES	WB	0.09	Horz(TL)	0.01	4	n/a	n/a
BCDL	8.0	Code	UBC97/ANSI95	(Simplified)	Wind(LL)	0.01	5	>999	240	Weight: 43 lb

**LUMBER**  
 TOP CHORD 2 X 4 DF No. 1&Btr G  
 BOT CHORD 2 X 4 DF No. 1&Btr G  
 WEBS 2 X 4 DF Std G

**BRACING**  
 TOP CHORD Sheathed or 6-0-0 oc purlins.  
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

**REACTIONS (lb/size)** 4=481/Mechanical, 2=610/3-8  
 Max Horz 2=54(load case 4)  
 Max Uplift 4=-93(load case 5), 2=-196(load case 5)

**FORCES (lb) - Maximum Compression/Maximum Tension**  
 TOP CHORD 1-2=0/22, 2-3=-611/118, 3-4=-610/117  
 BOT CHORD 2-5=-61/561, 4-5=-61/561  
 WEBS 3-5=0/228

- NOTES**
- Unbalanced roof live loads have been considered for this design.
  - This truss has been designed for the wind loads generated by 80 mph winds at 25 ft above ground level, using 5.0 psf top chord dead load and 5.0 psf bottom chord dead load, 100 mi from hurricane coastline, on an occupancy category I, condition I enclosed building, of dimensions 45 ft by 24 ft with exposure C ASCE 7-93 per UBC97/ANSI95 if end verticals or cantilevers exist, they are exposed to wind. If porches exist, they are not exposed to wind. The lumber DOL increase is 1.33, and the plate grip increase is 1.33.
  - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
  - A plate rating reduction of 20% has been applied for the green lumber members.
  - Refer to girder(s) for truss to truss connections.

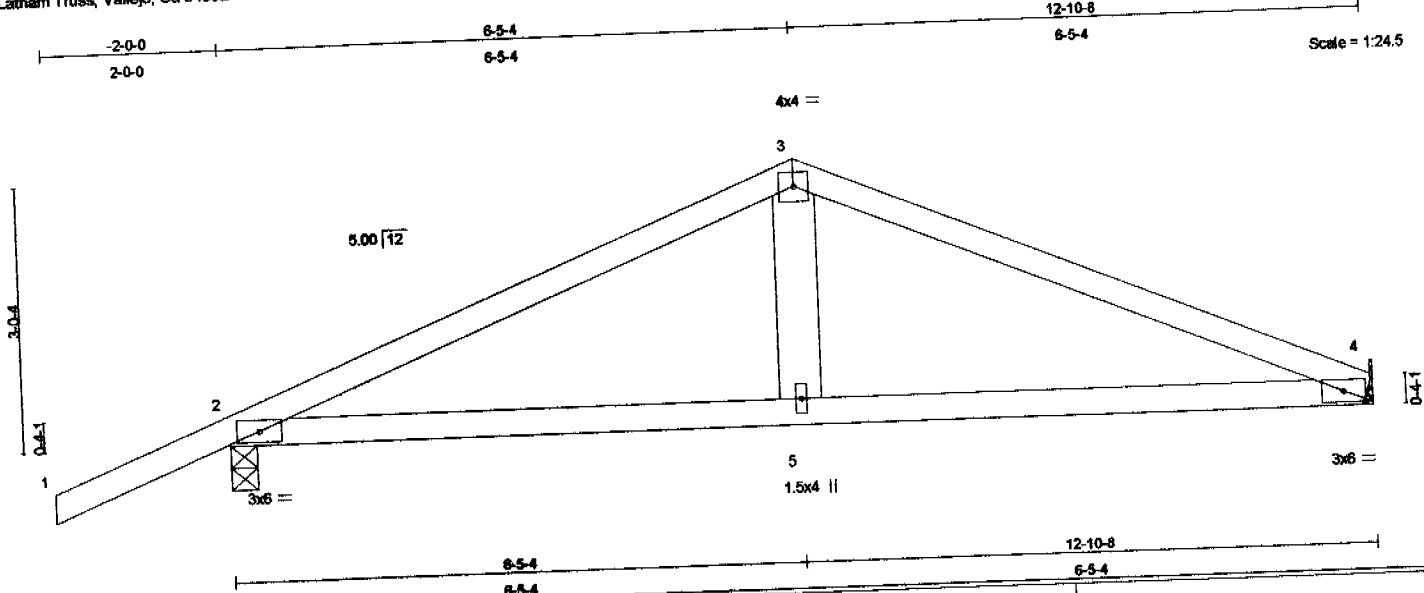
LOAD CASE(S) Standard



September 24, 2004

**WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.**  
 Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the fabricator. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANS/TPI1 Quality Criteria, DSB-89 and BCS11 Building Component Safety information available from Truss Plate Institute, 563 D'Onofrio Drive, Madison, WI 53718.

7777 Greenback Lane  
 Suite 109  
 Citrus Heights, CA, 95610



LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc)	I/defl	L/d	PLATES	GRIP
TCLL 16.0	Plates Increase 1.25	TC 0.61	Vert(LL) -0.06	2-5 >999	360	MT20	220/195
TCDL 14.0	Lumber Increase 1.25	BC 0.73	Vert(TL) -0.12	2-5 >999	180		
BCLL 0.0	Rep Stress Incr NO	WB 0.06	Horz(TL) 0.02	4 n/a	n/a		
BCDL 8.0	Code UBC97/ANSI95	(Simplified)	Wind(LL) 0.03	5 >999	240		Weight: 45 lb

**LUMBER**  
TOP CHORD 2 X 4 DF No.1&Btr G  
BOT CHORD 2 X 4 DF No.1&Btr G  
WEBS 2 X 6 DF No.2 G

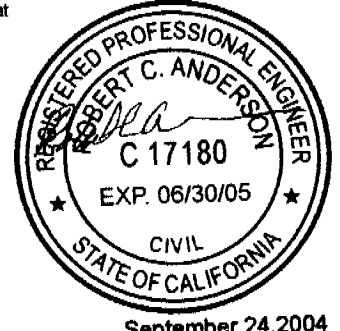
**BRACING**  
TOP CHORD Sheathed or 4-4-2 oc purlins.  
BOT CHORD Rigid ceiling directly applied or 9-8-9 oc bracing.

**REACTIONS (lb/size)** 4=897/Mechanical, 2=10300-3-8  
Max Horz 2=54(load case 4)  
Max Uplift 4=-228(load case 5), 2=-333(load case 5)

**FORCES (lb) - Maximum Compression/Maximum Tension**  
TOP CHORD 1-2=0/22, 2-3=-1572/526, 3-4=-1589/525  
BOT CHORD 2-5=-436/1443, 4-5=-436/1443  
WEBS 3-5=0/438

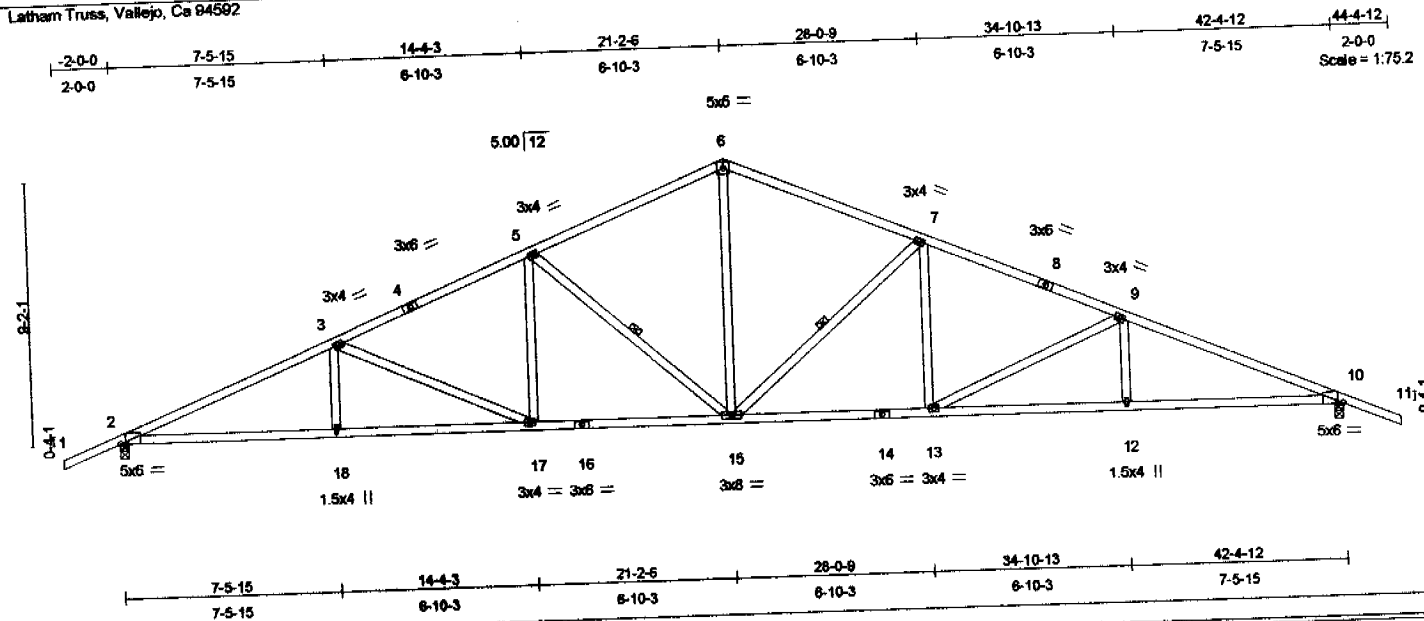
- NOTES**
- Unbalanced roof live loads have been considered for this design.
  - This truss has been designed for the wind loads generated by 80 mph winds at 25 ft above ground level, using 5.0 psf top chord dead load and 5.0 psf bottom chord dead load, 100 mi from hurricane oceanline, on an occupancy category I, condition I enclosed building, of dimensions 45 ft by 24 ft with exposure C ASCE 7-93 per UBC97/ANSI95 if end verticals or cantilevers exist, they are exposed to wind. If porches exist, they are not exposed to wind. The lumber DOL increase is 1.33, and the plate grip increase is 1.33.
  - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
  - A plate rating reduction of 20% has been applied for the green lumber members.
  - Refer to girder(s) for truss to truss connections.
  - Girder carries hip end with 6-5-4 end setback.
  - Special hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 678lb down and 371lb up at 6-5-4 on top chord. The design/selection of such special connection device(s) is the responsibility of others.
  - In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

**LOAD CASE(S) Standard**  
1) Regular: Lumber Increase=1.25, Plate Increase=1.25  
Uniform Loads (psf)  
Vert: 1-3=-60, 3-4=-60, 2-4=-28(F=-12)  
Concentrated Loads (lb)  
Vert: 3=-678



September 24, 2004

<p><b>WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MII-7473 BEFORE USE.</b></p> <p>Design valid for use only with Mittek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is the responsibility of building designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSIP11 Quality Criteria, DSB-89 and BCS1 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53716.</p>	<p>7777 Greenback Lane Suite 109 Citrus Heights, CA, 95610</p>
---	--



LOADING (psf)		SPACING	2-0-0	CSI	DEFL	in (loc)	l/defl	L/d	PLATES	GRIP
TCLL	16.0	Plates Increase	1.25	TC	0.72	Vert(LL)	-0.17	15	>999	360
TCDL	14.0	Lumber Increase	1.25	BC	0.67	Vert(TL)	-0.44	15-17	>999	180
BCLL	0.0	Rep Stress Incr	YES	WB	0.62	Horz(TL)	0.15	10	n/a	n/a
BCDL	8.0	Code	UBC97/ANSI95	(Simplified)	Wind(LL)	0.18	15	>999	240	Weight: 204 lb

**LUMBER**  
 TOP CHORD 2 X 4 DF No. 1&Btr G  
 BOT CHORD 2 X 4 DF No. 1&Btr G  
 WEBS 2 X 4 DF Std G

**BRACING**  
 TOP CHORD Sheathed or 2-11-8 oc purlins.  
 BOT CHORD Rigid ceiling directly applied or 9-3-13 oc bracing.  
 WEBS 1 Row at midpt 5-15, 7-15

**REACTIONS (lb/size)** 2=1729/0-3-8, 10=1729/0-3-8  
 Max Horz 2=28 (load case 4)  
 Max Uplift 2=-411 (load case 5), 10=-411 (load case 5)

**FORCES (lb) - Maximum Compression/Maximum Tension**  
**TOP CHORD** 1-2=0/22, 2-3=-3345/645, 3-4=-2713/582, 4-5=-2713/582, 5-6=-2058/509, 6-7=-2058/509, 7-8=-2713/582, 8-9=-2713/582, 9-10=-3345/645, 10-11=0/22  
**BOT CHORD** 2-18=-491/3074, 17-18=-491/3074, 16-17=-332/2504, 15-16=-332/2504, 14-15=-332/2504, 13-14=-332/2504, 12-13=-491/3074, 10-12=-491/3074  
**WEBS** 3-18=0/256, 5-17=-5468, 6-15=-223/1172, 7-13=-5468, 9-12=0/256, 3-17=-633/176, 5-15=-808/221, 7-15=-808/221, 9-13=-633/176

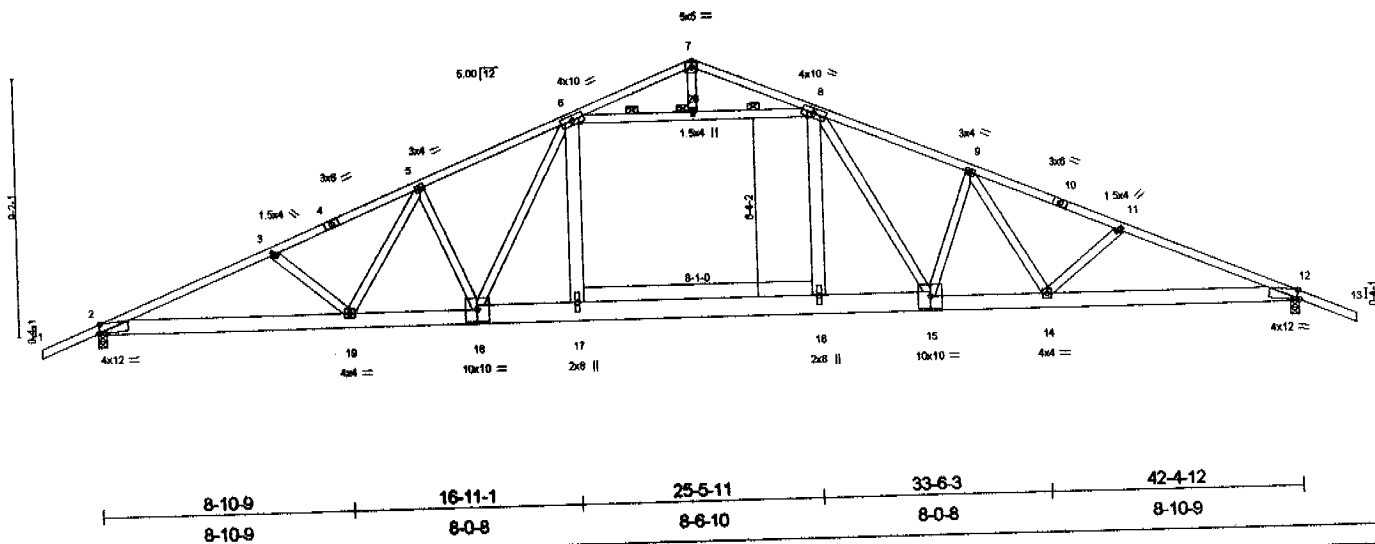
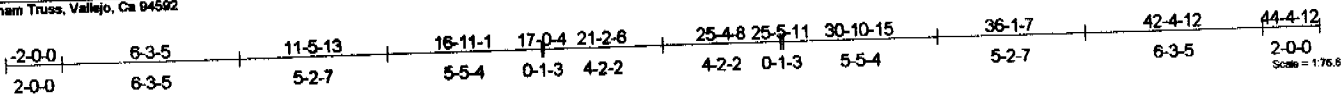
**NOTES**  
 1) Unbalanced roof live loads have been considered for this design.  
 2) This truss has been designed for the wind loads generated by 80 mph winds at 25 ft above ground level, using 5.0 psf top chord dead load and 5.0 psf bottom chord dead load, 100 mi from hurricane coastline, on an occupancy category I, condition I enclosed building, of dimensions 45 ft by 24 ft with exposure C ASCE 7-93 per UBC97/ANSI95 if end verticals or cantilevers exist, they are exposed to wind. If porches exist, they are not exposed to wind. The lumber DOL increase is 1.33, and the plate grip increase is 1.33.  
 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.  
 4) A plate rating reduction of 20% has been applied for the green lumber members.

**LOAD CASE(S)** Standard



September 24, 2004

<p><b>WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.</b>          Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANS/IF11 Quality Criteria, DSB-89 and BCS1 Building Component Safety information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.</p>	<p>7777 Greenback Lane          Suite 109          Citrus Heights, CA, 95610</p>
--	--



LOADING (psf)		SPACING	CS	DEFL	in (loc)	Vdefl	L/d	PLATES	GRIP
TCLL	16.0	2-0-0	TC 0.40	Vert(LL)	-0.30	16-17	>999	MT20	220/195
TCDL	14.0	Plates Increase 1.25	BC 0.92	Vert(TL)	-0.54	16-17	>937		
BCLL	0.0	Lumber Increase 1.25	WB 0.45	Horz(TL)	0.14	12	n/a		
BCDL	8.0	Rep Stress Incr YES	(Matrix)	Wind(LL)	0.16	17	>999		Weight: 269 lb
		Code UBC97/ANSI95							

**LUMBER**  
**TOP CHORD** 2 X 4 DF No. 1&Btr G  
**BOT CHORD** 2 X 6 DF No. 2 G \*Except\*  
 15-18 2 X 8 DF No. 2-G  
**WEBS** 2 X 4 DF Std G \*Except\*  
 6-17 2 X 6 DF No. 2-G, 8-16 2 X 6 DF No. 2-G

**BRACING**  
**TOP CHORD** Sheathed or 2-10-9 oc purlins.  
**BOT CHORD** Rigid ceiling directly applied or 10-0-0 oc bracing.  
**WEBS** 1 Row at midpt 6-20, 8-20  
**JOINTS** 1 Brace at Jt(s) 20

**REACTIONS (lb/size)** 2=2156/0-3-8, 12=2156/0-3-8  
 Max Horz 2=-35(load case 3)  
 Max Uplift 2=-326(load case 5), 12=-326(load case 5)

**FORCES (lb) - Maximum Compression/Maximum Tension**  
**TOP CHORD** 1-2=0/49, 2-3=-4669/461, 3-4=-4392/400, 4-5=-4333/413, 5-6=-3992/424, 6-7=-680/87, 7-8=-680/87, 8-9=-4032/437,  
 9-10=-4332/412, 10-11=-4392/400, 11-12=-4669/461, 12-13=0/49  
**BOT CHORD** 2-19=-316/4241, 18-19=-189/3827, 17-18=-44/3436, 16-17=-44/3434, 15-16=-44/3436, 14-15=-189/3826,  
 12-14=-316/4241  
**WEBS** 6-20=-2853/298, 8-20=-2853/298, 6-17=0/974, 8-16=0/944, 5-18=-540/165, 5-19=-14/440, 3-19=-329/165,  
 9-15=-538/165, 9-14=-13/437, 11-14=-329/165, 7-20=0/220, 6-18=-211/638, 8-15=-219/665

**NOTES**

- 1) Unbalanced roof live loads have been considered for this design.
- 2) This truss has been designed for the wind loads generated by 80 mph winds at 25 ft above ground level, using 5.0 psf top chord dead load and 5.0 psf bottom chord dead load, 100 mi from hurricane oceanline, on an occupancy category I, condition I enclosed building, of dimensions 45 ft by 42 ft with exposure C ASCE 7-93 per UBC97/ANSI95 If end verticals or cantilevers exist, they are exposed to wind. If porches exist, they are not exposed to wind. The lumber DOL increase is 1.33, and the plate grip increase is 1.33
- 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) Ceiling dead load (5.0 psf) on member(s). 6-20, 8-20
- 5) Bottom chord live load (40.0 psf) and additional bottom chord dead load (5.0 psf) applied only to room. 16-17
- 6) A plate rating reduction of 20% has been applied for the green lumber members.

**LOAD CASE(S)** Standard

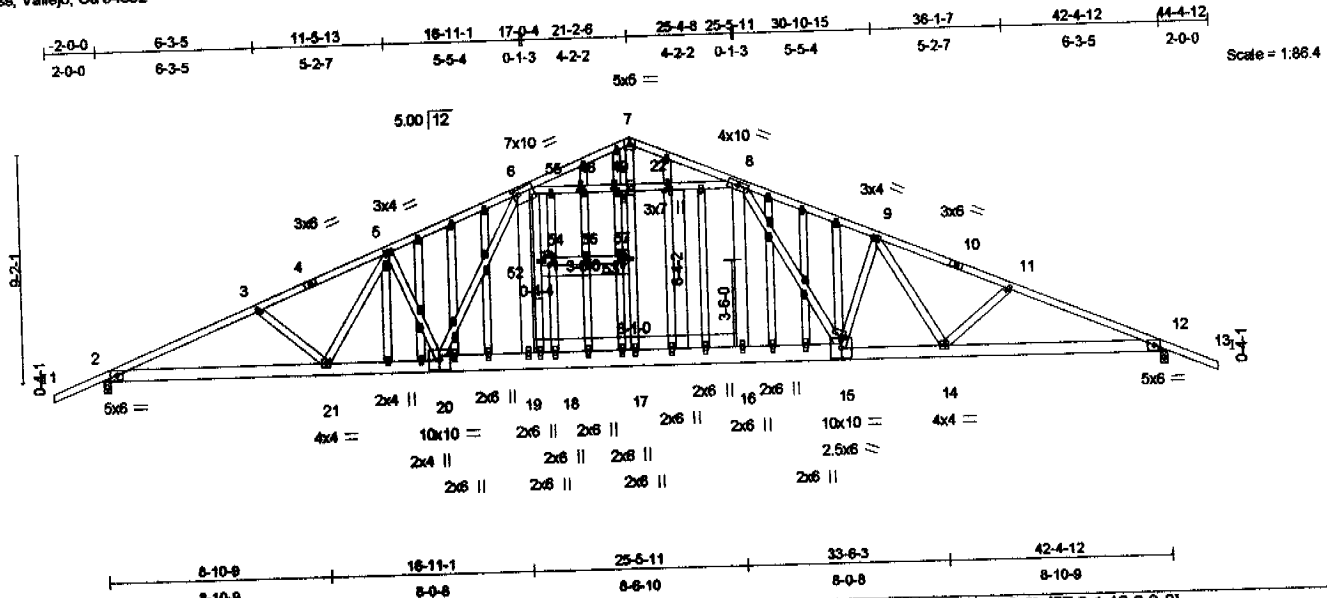


September 24, 2004

**WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.**  
 Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANS/TP11 Quality Criteria, DSB-89 and BCS1 Building Component Safety Information available from Truss Plate Institute, 563 D'Onofrio Drive, Madison, WI 53719.

7777 Greenback Lane  
 Suite 109  
 Citrus Heights, CA, 95610





LOADING (psf)	SPACING 2-0-0	CSI	DEFL in (loc)	I/defl	L/d	PLATES	GRIP
TCLL 16.0	Plates Increase 1.25	TC 0.29	Vert(LL) -0.19 15-16	>999	360	MT20	220/195
TCDL 14.0	Lumber Increase 1.25	BC 0.57	Vert(TL) -0.39 19-20	>999	180		
BCLL 0.0	Rep Stress Incr YES	WB 0.55	Horz(TL) 0.10 12	n/a	n/a		
BCDL 8.0	Code UBC97/ANSI95	(Matrix)	Wind(LL) 0.16 19	>999	240		Weight: 386 lb

**LUMBER**  
TOP CHORD 2 X 4 DF No. 1&Btr G  
BOT CHORD 2 X 6 DF No. 2 G \*Except\*  
15-20 2 X 8 DF No. 2-G  
WEBS 2 X 4 DF Std G \*Except\*  
6-19 2 X 6 DF No. 2-G, 8-16 2 X 6 DF No. 2-G  
OTHERS 2 X 4 DF Std G

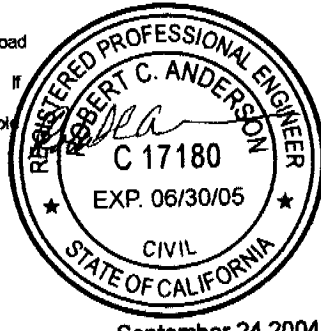
**BRACING**  
TOP CHORD Sheathed or 3-4-7 oc purlins.  
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.  
JOINTS 1 Brace at Jt(s): 22, 52, 53

**REACTIONS (lb/size)** 2=1729/0-3-8, 12=1729/0-3-8  
Max Horz 2=-26(load case 3)  
Max Uplift 2=-411(load case 5), 12=-411(load case 5)

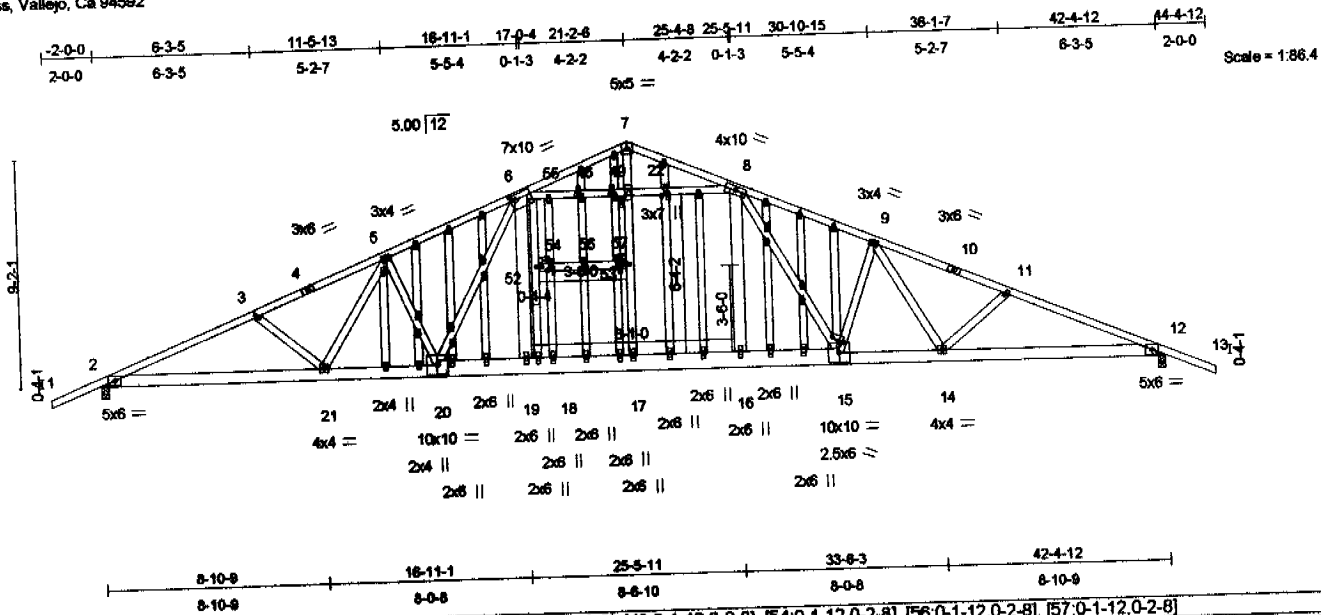
**FORCES (lb) - Maximum Compression/Maximum Tension**  
TOP CHORD 1-2=0/49, 2-3=-3571/680, 3-4=-3291/620, 4-5=-3230/632, 5-6=-2886/646, 6-7=-764/167, 7-8=-764/167, 8-9=-2928/659,  
9-10=-3230/632, 10-11=-3291/620, 11-12=-3571/680, 12-13=0/49  
BOT CHORD 2-21=-518/3230, 20-21=-393/2805, 19-20=-260/2322, 18-19=-260/2322, 17-18=-259/2321, 16-17=-258/2319,  
15-16=-258/2320, 14-15=-394/2807, 12-14=-518/3229  
WEBS 6-55=-1664/438, 46-55=-1659/436, 46-49=-1659/436, 22-49=-1659/436, 8-22=-1663/438, 6-19=0/191, 8-16=-226/281,  
5-20=-537/163, 5-21=-11/435, 3-21=-337/163, 9-15=-534/164, 9-14=-10/431, 11-14=-337/163, 7-22=-23/343, 6-20=-177/811,  
8-15=-191/839, 17-53=0/264, 22-53=0/238, 18-52=-315/117, 6-52=-288/233, 52-54=-26/18, 54-56=-26/18, 56-57=-26/18,  
53-57=-26/18, 54-55=-161/38, 46-56=-35/36, 49-57=-9/55

- NOTES**
- 1) Unbalanced roof live loads have been considered for this design.
  - 2) This truss has been designed for the wind loads generated by 80 mph winds at 25 ft above ground level, using 5.0 psf top chord dead load and 5.0 psf bottom chord dead load, 100 mi from hurricane oceanline, on an occupancy category I, condition I enclosed building, of dimensions 45 ft by 24 ft with exposure C ASCE 7-93 per UBC97/ANSI95. If end verticals or cantilevers exist, they are exposed to wind. If porches exist, they are not exposed to wind. The lumber DOL increase is 1.33, and the plate grip increase is 1.33.
  - 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Mitek "Standard Gable End Detail".
  - 4) All plates are 1.5x4 MT20 unless otherwise indicated.
  - 5) Gable studs spaced at 1-4-0 oc.
  - 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
  - 7) A plate rating reduction of 20% has been applied for the green lumber members.

LOAD CASE(S) Standard  
September 24, 2004



<p><b>WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.</b>  Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANS/TP1 Quality Criteria, D88-89 and BCS1 Building Component Safety Information available from TrussPlate Institute, 583 D'Onofrio Drive, Madison, WI 53719.</p>	<p>7777 Greenback Lane  Suite 109  Citrus Heights, CA, 95610</p>
--	--



LOADING (psf)		SPACING	CSI	DEFL		PLATES	GRIP
TCLL	16.0	2-0-0	TC 0.29	in (loc)	I/defl	MT20	220/195
ICDL	14.0	Plates Increase 1.25	BC 0.57	Vert(LL) -0.19 15-16	>999 360		
BCLL	0.0	Lumber Increase 1.25	WB 0.55	Vert(TL) -0.39 19-20	>999 180		
BCDL	8.0	Rep Stress Incr YES	(Matrix)	Horz(TL) 0.10 12	n/a n/a		
		Code UBC97/ANSI95		Wind(LL) 0.16 19	>999 240		Weight: 386 lb

**LUMBER**  
 TOP CHORD 2 X 4 DF No.1&Btr G  
 BOT CHORD 2 X 6 DF No.2 G \*Except\*  
 15-20 2 X 8 DF No.2-G  
 WEBS 2 X 4 DF Std G \*Except\*  
 6-19 2 X 6 DF No.2-G, 8-16 2 X 6 DF No.2-G  
 OTHERS 2 X 4 DF Std G

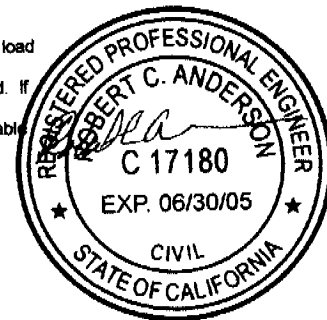
**BRACING**  
 TOP CHORD Sheathed or 3-4-7 oc purlins.  
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.  
 JOINTS 1 Brace at Jt(s): 22, 52, 53

**REACTIONS** (lb/size) 2=1729/0-3-8, 12=1729/0-3-8  
 Max Horz 2=-26(load case 3)  
 Max Up/lt 2=-411(load case 5), 12=-411(load case 5)

**FORCES** (lb) - Maximum Compression/Maximum Tension  
 TOP CHORD 1-2=0/49, 2-3=-3571/680, 3-4=-3291/620, 4-5=-3230/632, 5-6=-2886/646, 6-7=-764/167, 7-8=-764/167, 8-9=-2928/659,  
 9-10=-3230/632, 10-11=-3291/620, 11-12=-3571/680, 12-13=0/49  
 BOT CHORD 2-21=-518/3230, 20-21=-393/2805, 19-20=-260/2322, 18-19=-260/2322, 17-18=-259/2321, 16-17=-258/2319,  
 15-16=-258/2320, 14-15=-394/2807, 12-14=-518/3229  
 WEBS 6-55=-1664/438, 46-55=-1659/436, 46-49=-1659/436, 22-49=-1659/436, 8-22=-1663/438, 6-19=0/191, 8-16=-226/261,  
 5-20=-537/163, 5-21=-11/435, 3-21=-337/163, 9-15=-534/164, 9-14=-10/431, 11-14=-337/163, 7-22=-23/343, 6-20=-177/811,  
 8-15=-191/839, 17-53=0/264, 22-53=0/238, 18-52=-315/117, 6-52=-288/233, 52-54=-26/18, 54-56=-26/18, 56-57=-26/18,  
 53-57=-26/18, 54-55=-161/38, 46-56=-35/36, 49-57=-9/55

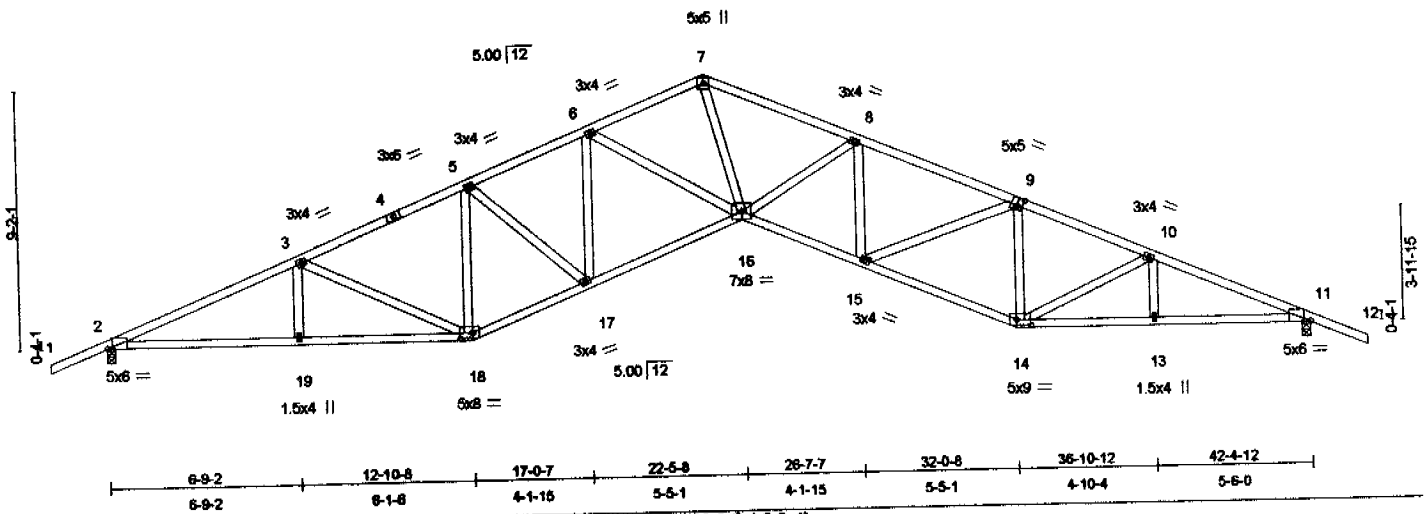
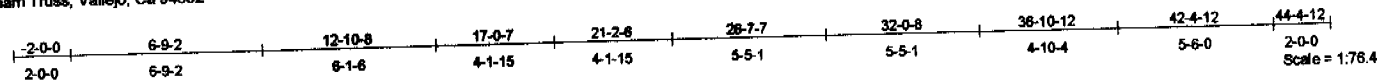
- NOTES**
- Unbalanced roof live loads have been considered for this design.
  - This truss has been designed for the wind loads generated by 80 mph winds at 25 ft above ground level, using 5.0 psf top chord dead load and 5.0 psf bottom chord dead load, 100 mi from hurricane oceanline, on an occupancy category I, condition I enclosed building, of dimensions 45 ft by 24 ft with exposure C ASCE 7-93 per UBC97/ANSI95 If end verticals or cantilevers exist, they are exposed to wind. If porches exist, they are not exposed to wind. The lumber DOL increase is 1.33, and the plate grip increase is 1.33
  - Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
  - All plates are 1.5x4 MT20 unless otherwise indicated.
  - Gable studs spaced at 1-4-0 oc.
  - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
  - A plate rating reduction of 20% has been applied for the green lumber members.

**LOAD CASE(S)** Standard



September 24, 2004

<p><b>WARNING</b> - Verify design parameters and READ NOTES ON TRUSS AND INCLUDED MITEK REFERENCE PAGE MII-7476 BEFORE USE.          Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component.          Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANS/THI Quality Criteria, D88-89 and BC91 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.</p>	<p>7777 Greenback Lane          Suite 109          Citrus Heights, CA, 95610</p>
--	--



LOADING (psf)		SPACING		CSI		DEPL		PLATES		GRIP	
TCLL	16.0	2-0-0	1.25	TC	0.52	in (loc)	l/defl	L/d	MT20	220/195	
TCDL	14.0	Plate Increase	1.25	BC	0.60	Vert(LL)	>999	360			
BCLL	0.0	Lumber Increase	1.25	WB	0.41	Vert(TL)	-0.64	16			
BCDL	8.0	Rep Stress Inor	YES	(Simplified)		Horz(TL)	0.31	11			
		Code	UBC97/ANSI95			Wind(LL)	0.27	16			Weight: 210 lb

**LUMBER**  
 TOP CHORD 2 X 4 DF No. 1&Btr G  
 BOT CHORD 2 X 4 DF No. 1&Btr G  
 WEBS 2 X 4 DF Std G "Except"  
 7-16 2 X 4 DF No. 1&Btr-G

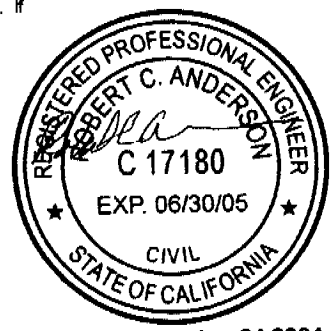
**BRACING**  
 TOP CHORD Sheathed or 2-11-10 oc purlins.  
 BOT CHORD Rigid ceiling directly applied or 9-1-5 oc bracing.

**REACTIONS** (lb/size) 2=1729/0-3-8, 11=1729/0-3-8  
 Max Horz 2=26(load case 4)  
 Max Uplift 2=-411(load case 5), 11=-411(load case 5)

**FORCES** (lb) - Maximum Compression/Maximum Tension  
 TOP CHORD 1-2=0/22, 2-3=-3406/656, 3-4=-2859/605, 4-5=-2859/605, 5-6=-3254/668, 6-7=-3416/627, 7-8=-4153/731, 8-9=-3928/746,  
 9-10=-3093/640, 10-11=-3504/675, 11-12=0/22  
 BOT CHORD 2-19=-507/3128, 18-19=-507/3128, 17-18=-397/2838, 16-17=-383/3254, 15-16=-491/3928, 14-15=-458/3053,  
 13-14=-533/3214, 11-13=-533/3214  
 WEBS 3-19=0/227, 3-18=-546/152, 5-18=-782/140, 5-17=0/502, 6-17=-402/32, 6-16=-24/290, 7-16=-417/2710, 8-16=0/337,  
 8-15=-547/73, 9-15=-36/859, 9-14=-920/178, 10-14=-407/114, 10-13=0/180

**NOTES**  
 1) Unbalanced roof live loads have been considered for this design.  
 2) This truss has been designed for the wind loads generated by 80 mph winds at 25 ft above ground level, using 5.0 psf top chord dead load and 5.0 psf bottom chord dead load, 100 mi from hurricane coastline, on an occupancy category I, condition I enclosed building, of dimensions 45 ft by 24 ft with exposure C ASCE 7-93 per UBC97/ANSI95 if end verticals or cantilevers exist, they are exposed to wind. If porches exist, they are not exposed to wind. The lumber DOL increase is 1.33, and the plate grip increase is 1.33  
 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.  
 4) A plate rating reduction of 20% has been applied for the green lumber members.

**LOAD CASE(S)** Standard



September 24, 2004

<p><b>WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGES PRI-1473 BEFORE USE.</b>          Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANS/TP1 Quality Criteria, D38-87 and ICS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.</p>	<p>7777 Greenback Lane          Suite 109          Citrus Heights, CA, 95610</p>
--	--

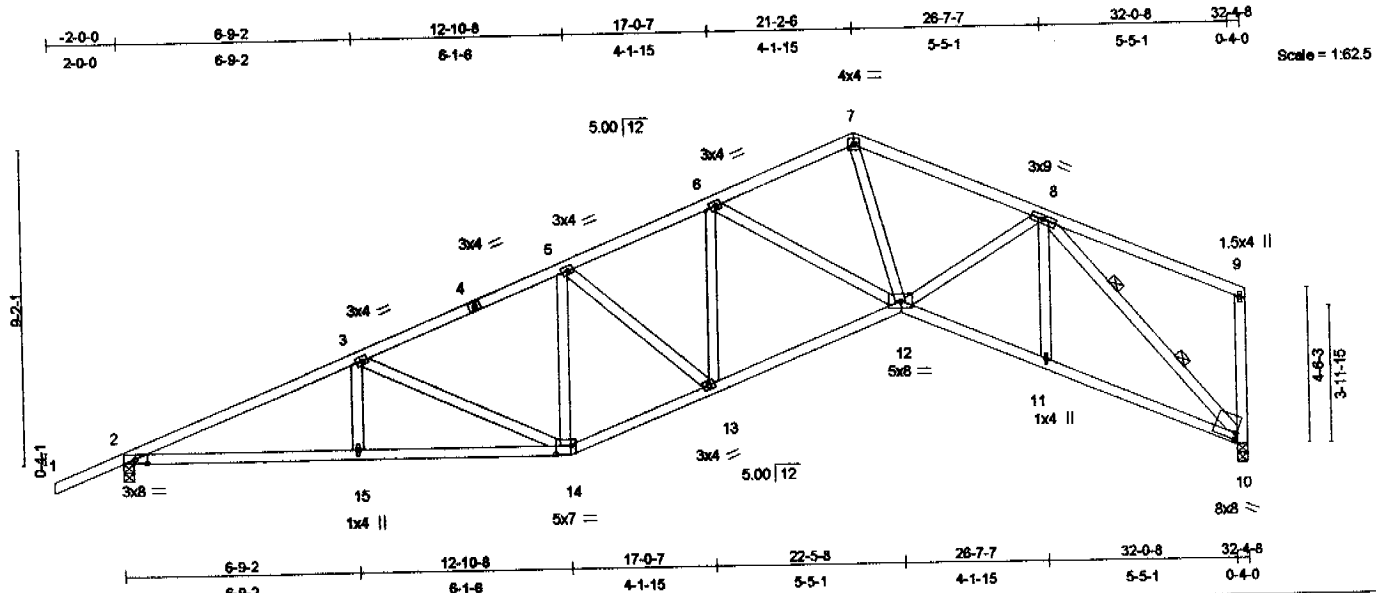


Plate Offsets (X, Y): [2-0-4-2,0-1-8], [10-0-0-5,0-2-4], [14-0-5-4,0-2-12]					
<b>LOADING</b> (psf)	<b>SPACING</b> 2-0-0	<b>CSI</b>	<b>DEFL.</b> in (loc) l/defl L/d	<b>PLATES</b>	<b>GRIP</b>
TCLL 16.0	Plates Increase 1.25	TC 0.43	Vert(LL) -0.10 13 >999 360	MT20	220/195
TCDL 14.0	Lumber Increase 1.25	BC 0.51	Vert(TL) -0.23 14-15 >999 180		
BCLL 0.0	Rep Stress Incr YES	WB 0.53	Horz(TL) 0.16 10 n/a n/a		
BCDL 8.0	Code UBC97/ANSI95	(Simplified)	Wind(LL) 0.11 13 >999 240		Weight: 173 lb

**LUMBER**  
 TOP CHORD 2 X 4 DF No. 1&Btr G  
 BOT CHORD 2 X 4 DF No. 1&Btr G  
 WEBS 2 X 4 DF Std G \*Except\*  
 7-12 2 X 4 DF No. 1&Btr-G

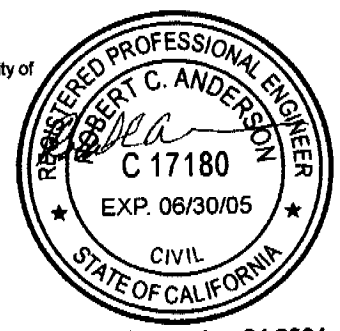
**BRACING**  
 TOP CHORD Sheathed or 3-11-0 oc purlins, except end verticals.  
 BOT CHORD Rigid ceiling directly applied or 8-7-3 oc bracing.  
 WEBS 2 Rows at 1/3 pts 8-10

**REACTIONS** (lb/size) 2=1346/0-3-8, 10=1220/0-3-8  
 Max Horz 2=307 (load case 4)  
 Max Up/rt 2=337 (load case 5), 10=234 (load case 5)

**FORCES** (lb) - Maximum Compression/Maximum Tension  
 TOP CHORD 1-2=0/22, 2-3=-2440/470, 3-4=-1881/416, 4-5=-1881/416, 5-6=-1955/443, 6-7=-1699/436, 7-8=-2021/499, 8-9=-15/124, 9-10=-168/116  
 BOT CHORD 2-15=587/2241, 14-15=-587/2241, 13-14=-479/1870, 12-13=-492/1955, 11-12=-364/1398, 10-11=-364/1398  
 WEBS 3-15=0/227, 3-14=-559/155, 5-14=-403/170, 5-13=-15/122, 6-13=-39/93, 6-12=-342/178, 7-12=-257/1181, 8-12=-135/678, 8-11=0/175, 8-10=-2015/446

- NOTES**
- 1) Unbalanced roof live loads have been considered for this design.
  - 2) This truss has been designed for the wind loads generated by 80 mph winds at 25 ft above ground level, using 5.0 psf top chord dead load and 5.0 psf bottom chord dead load, 100 mi from hurricane coastline, on an occupancy category I, condition I enclosed building, of dimensions 45 ft by 24 ft with exposure C ASCE 7-93 per UBC97/ANSI95 if end verticals or cantilevers exist, they are exposed to wind. If porches exist, they are not exposed to wind. The lumber DOL increase is 1.33, and the plate grip increase is 1.33
  - 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
  - 4) A plate rating reduction of 20% has been applied for the green lumber members.
  - 5) Bearing at joint(s) 10 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

**LOAD CASE(S)** Standard



September 24, 2004

<p><b>WARNING</b> - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MD-1478 BEFORE USE.          Design valid for use only with MITEK connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI Quality Criteria, D88-87 and IBC311 Building Component Safety Information available from Truss Plate Institute, 563 D'Onofrio Drive, Madison, WI 53719.</p>	<p>7777 Greenbeck Lane          Suite 109          Citrus Heights, CA, 95610</p>
--	--

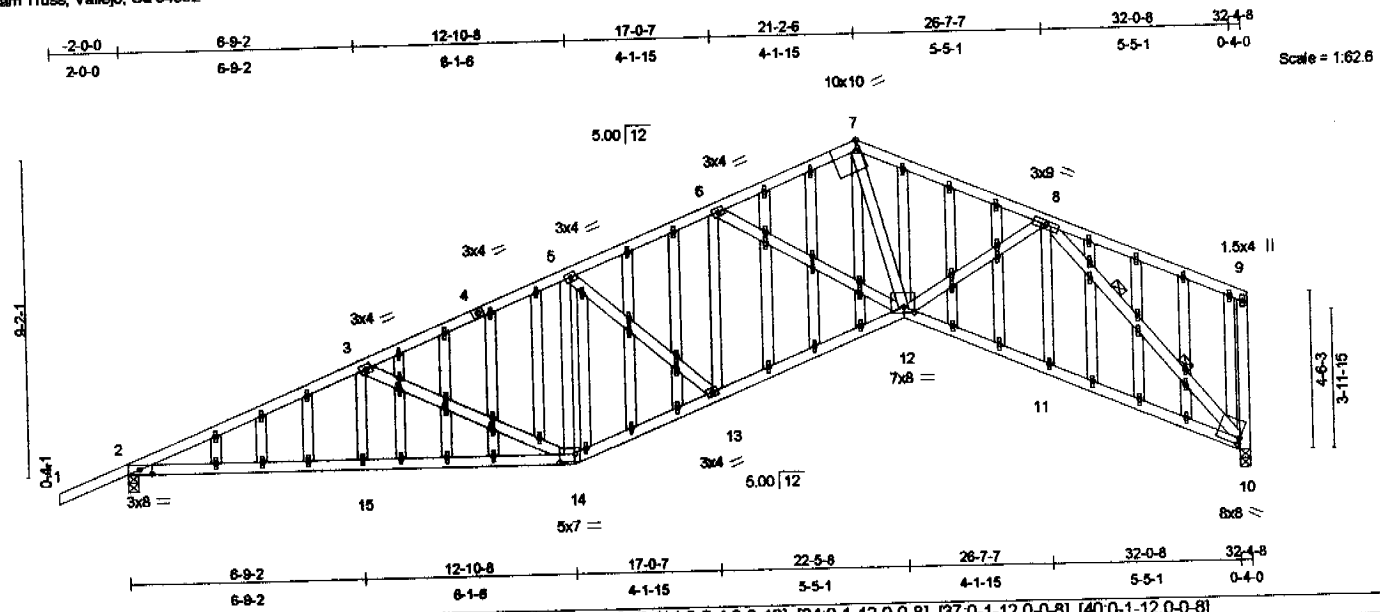


Plate Offsets (X, Y): [2:0-4-2,0-1-8], [7:0-1-7,Edge], [10:0-0-5,0-2-0], [12:0-3-12,0-2-4], [14:0-5-4,0-2-12], [34:0-1-12,0-0-8], [37:0-1-12,0-0-8], [40:0-1-12,0-0-8]

LOADING (psf)	SPACING	CSJ	DEFL	PLATES	GRIP
TCLL 16.0	2-0-0	TC 0.43	in (loc) l/defl L/d	MT20	220/195
TCDL 14.0	Plates Increase 1.25	BC 0.51	Vert(LL) -0.10 13 >999 360		
BCLL 0.0	Lumber Increase 1.25	WB 0.53	Vert(TL) -0.23 14-15 >999 180		
BCDL 8.0	Rep Stress Incr YES	(Simplified)	Horz(TL) 0.16 10 n/a n/a		
	Code UBC97/ANSI95		Wind(LL) 0.11 13 >999 240		Weight: 276 lb

**LUMBER**  
 TOP CHORD 2 X 4 DF No. 1&Btr G  
 BOT CHORD 2 X 4 DF No. 1&Btr G  
 WEBS 2 X 4 DF Std G "Except"  
 7-12 2 X 4 DF No. 1&Btr-G  
 OTHERS 2 X 4 DF Std G

**BRACING**  
 TOP CHORD Sheathed or 3-11-0 cc purlins, except end verticals.  
 BOT CHORD Rigid ceiling directly applied or 8-7-3 cc bracing.  
 WEBS 2 Rows at 1/3 pts 8-10

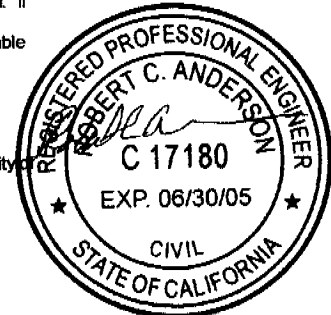
**REACTIONS** (lb/size) 2=1346/0-3-8, 10=1220/0-3-8  
 Max Horz 2=307 (load case 4)  
 Max Uplift 2=337 (load case 5), 10=234 (load case 5)

**FORCES** (lb) - Maximum Compression/Maximum Tension  
 TOP CHORD 1-2=0/22, 2-3=2440/470, 3-4=1881/416, 4-5=1881/416, 5-6=1955/443, 6-7=1699/436, 7-8=2021/499, 8-9=15/124,  
 9-10=168/116  
 BOT CHORD 2-15=587/2241, 14-15=587/2241, 13-14=479/1870, 12-13=492/1855, 11-12=364/1398, 10-11=364/1398  
 WEBS 3-15=0/227, 3-14=559/155, 5-14=403/170, 5-13=15/122, 6-13=39/93, 6-12=342/178, 7-12=257/1181, 8-12=135/678,  
 8-11=0/175, 8-10=2015/446

**NOTES**

- Unbalanced roof live loads have been considered for this design.
- This truss has been designed for the wind loads generated by 80 mph winds at 25 ft above ground level, using 5.0 psf top chord dead load and 5.0 psf bottom chord dead load, 100 mi from hurricane coastline, on an occupancy category I, condition I enclosed building, of dimensions 45 ft by 24 ft with exposure C ASCE 7-93 per UBC97/ANSI95 if end verticals or cantilevers exist, they are exposed to wind. If porches exist, they are not exposed to wind. The lumber DOL increase is 1.33, and the plate grip increase is 1.33
- Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
- All plates are 1x4 MT20 unless otherwise indicated.
- Gable studs spaced at 1-4-0 oc.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- A plate rating reduction of 20% has been applied for the green lumber members.
- Bearing at joint(s) 10 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.

**LOAD CASE(S)** Standard



September 24, 2004

<p><b>WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MI-7478 BEFORE USE.</b>          Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSITR1 Quality Criteria, D38-89 and ICS11 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.</p>	<p>7777 Greenback Lane          Suite 109          Citrus Heights, CA, 95610</p>
--	--



Job 2374	Truss CG	Truss Type DBL. HOWE	Qty 1	Ply 2	JOHNSON ADDITION WHITE Job Reference (optional)	R13676685
-------------	-------------	-------------------------	----------	----------	---	-----------

Latham Truss, Vallejo, Ca 94592


6.100 s Aug 25 2004 Mitek Industries, Inc. Fri Sep 24 08:07:57 2004 Page 2

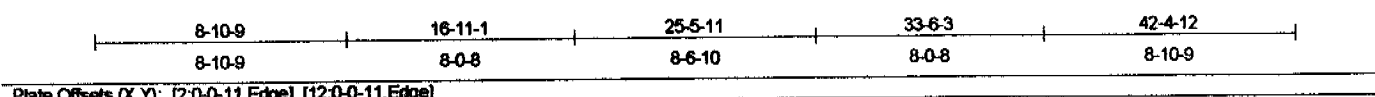
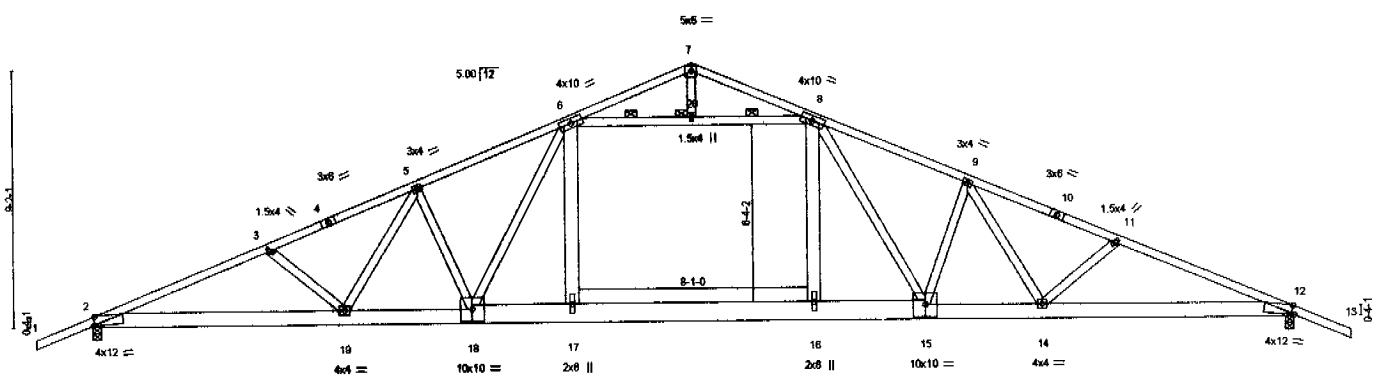
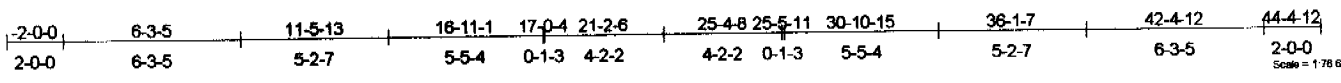
**NOTES**

- 9) A plate rating reduction of 20% has been applied for the green lumber members.
- 10) Girder carries tie-in span(s): 4-6-0 from 12-10-8 to 32-0-8
- 11) Special hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 910lb down and 175lb up at 12-10-8, and 605lb down and 117lb up at 32-0-8 on bottom chord. The design/selection of such special connection device(s) is the responsibility of others.

**LOAD CASE(S)** Standard

- 1) Regular: Lumber Increase=1.25, Plate Increase=1.25
- Uniform Loads (plf)
  - Vert: 2-64=-16, 64-65=-58(F=-42), 12-65=-16, 1-7=-60, 7-13=-60
- Concentrated Loads (lb)
  - Vert: 64=-910(F) 65=-605(F)

<p><b>WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-747'S BEFORE USE.</b>          Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult <b>ANSI/TR1 Quality Criteria, D38-87 and ECSI Building Component Safety Information</b> available from Truss Plate Institute, 563 D'Onofrio Drive, Madison, WI 53719.</p>	<p>7777 Greenback Lane          Suite 100          Citrus Heights, CA, 95610</p> 
---	--



LOADING (psf)		SPACING		CSI		DEFL		PLATES		GRIP	
TCLL	16.0	Plates Increase	2-0-0	TC	0.40	Vert(LL)	-0.30 16-17 >999 360	MT20		220/195	
TCDL	14.0	Lumber Increase	1.25	BC	0.92	Vert(TL)	-0.54 16-17 >937 180				
BCLL	0.0	Rep Stress Incr	YES	WB	0.45	Horz(TL)	0.14 12 n/a n/a				
BCDL	8.0	Code	UBC97/ANSI95	(Matrix)		Wind(LL)	0.16 17 >999 240				Weight: 269 lb

**LUMBER**  
 TOP CHORD 2 X 4 DF No. 1&Btr G  
 BOT CHORD 2 X 6 DF No. 2 G \*Except\*  
 15-18 2 X 8 DF No. 2-G  
 WEBS 2 X 4 DF Std G \*Except\*  
 6-17 2 X 6 DF No. 2-G, 8-16 2 X 6 DF No. 2-G

**BRACING**  
 TOP CHORD Sheathed or 2-10-9 oc purlins.  
 BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.  
 WEBS 1 Row at midpt 6-20, 8-20  
 JOINTS 1 Brace at Jt(s): 20

**REACTIONS** (lb/size) 2=2156/0-3-8, 12=2156/0-3-8  
 Max Horz 2=-35(load case 3)  
 Max Uplift 2=-326(load case 5), 12=-326(load case 5)

**FORCES** (lb) - Maximum Compression/Maximum Tension  
 TOP CHORD 1-2=0/49, 2-3=-4669/461, 3-4=-4392/400, 4-5=-4333/413, 5-6=-3992/424, 6-7=-680/87, 7-8=-680/87, 8-9=-4032/437,  
 9-10=-4332/412, 10-11=-4392/400, 11-12=-4669/461, 12-13=0/49  
 BOT CHORD 2-19=-316/4241, 18-19=-189/3827, 17-18=-44/3436, 16-17=-44/3434, 15-16=-44/3436, 14-15=-189/3826,  
 12-14=-316/4241  
 WEBS 6-20=-2853/298, 8-20=-2853/298, 6-17=0/974, 8-16=0/944, 5-19=-540/165, 5-19=-14/440, 3-19=-329/165,  
 9-15=-538/165, 9-14=-13/437, 11-14=-329/165, 7-20=0/220, 6-18=-211/638, 8-15=-219/665

- NOTES**
- 1) Unbalanced roof live loads have been considered for this design.
  - 2) This truss has been designed for the wind loads generated by 80 mph winds at 25 ft above ground level, using 5.0 psf top chord dead load and 5.0 psf bottom chord dead load, 100 mi from hurricane oceanline, on an occupancy category I, condition I enclosed building, of dimensions 45 ft by 42 ft with exposure C ASCE 7-93 per UBC97/ANSI95 If end verticals or cantilevers exist, they are exposed to wind. If porches exist, they are not exposed to wind. The lumber DOL increase is 1.33, and the plate grip increase is 1.33
  - 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
  - 4) Ceiling dead load (5.0 psf) on member(s). 6-20, 8-20
  - 5) Bottom chord live load (40.0 psf) and additional bottom chord dead load (5.0 psf) applied only to room. 16-17
  - 6) A plate rating reduction of 20% has been applied for the green lumber members.

**LOAD CASE(S)** Standard

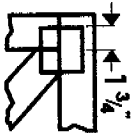


September 24, 2004

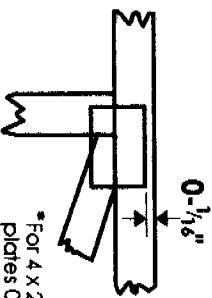
<p><b>WARNING</b> - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MIT-7473 BEFORE USE</p> <p>Design valid for use only with MITek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANS/IRI Quality Criteria, D88-89 and ICSI Building Component Safety Information available from Truss Plate Institute, 583 D'Oroff Drive, Madison, WI 53719.</p>	<p>7777 Greenback Lane          Suite 108          Citrus Heights, CA, 95610</p>
---	--

# Symbols

## PLATE LOCATION AND ORIENTATION



\* Center plate on joint unless X, Y offsets are indicated. Dimensions are in fractions. Apply plates to both sides of truss and securely seal.



\* For 4 x 2 orientation, locate plates 0 1/8" from outside edge of truss.



\* This symbol indicates the required direction of slots in connector plates.

\* Plate location details available in **MITREK 20/20 software** or upon request.

## PLATE SIZE

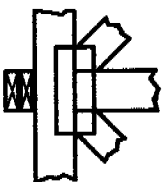
4 X 4

The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.

## LATERAL BRACING



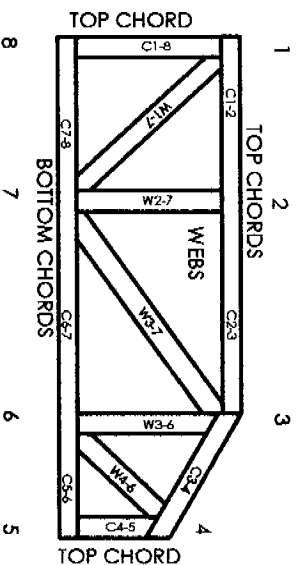
Indicated by symbol shown and/or by text in the bracing section of the output. Use T, L or Eliminator bracing if indicated.



Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

**Industry Standards:**  
ANSI/PPI1: National Design Specification for Metal Plate Connected Wood Truss Construction.  
DSB-89: Design Standard for Bracing.  
BCS11: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

# Numbering System



**JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.**

**CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.**

## CONNECTOR PLATE CODE APPROVALS

BOCA	96-31, 95-43, 96-20-1, 96-67, 84-32
ICBO	4922, 5243, 5363, 3907
SBCCI	9667, 9730, 9604B, 9511, 9432A



MITREK Engineering Reference Sheet: MII-7473

# General Safety Notes

## Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCS11.
2. Never exceed the design loading shown and never stock materials on inadequately braced trusses.
3. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
4. Cut members to bear tightly against each other.
5. Place plates on each face of truss at each joint and embed fully. Knots and wane of joint locations are regulated by ANSI/PPI1.
6. Design assumes trusses will be suitably protected from the environment in accord with ANSI/PPI1.
7. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
8. Unless expressly noted, this design is not applicable for use with fire retardant or preservative treated lumber.
9. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
10. Plate type, size, orientation and location dimensions shown indicate minimum plating requirements.
11. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
12. Top chords must be sheathed or purlins provided at spacing shown on design.
13. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
14. Connections not shown are the responsibility of others.
15. Do not cut or alter truss member or plate without prior approval of a professional engineer.
16. Install and load verifiably unless indicated otherwise.

© 2004 MITREK®