

CITY OF SACRAMENTO
1231 I Street, Sacramento, CA 95814

Permit No: 0113913

Insp Area: 4

Thos Bros:

Sub-Type: NSFR

Housing (Y/N): N

Site Address: 21 BLUE FERN CT SAC

Parcel No: 274-0570-023

NATOMAS W 2 LOT 43

CONTRACTOR

KAUFMAN AND BROAD
611 ORANGE DR
VACAVILLE CA. 95687

OWNER

ARCHITECT

Nature of Work: NSFR MP2318 2 STORY 10 RMS

CONSTRUCTION LENDING AGENCY : I hereby affirm under penalty of perjury that there is a construction lending agency for the performance of the work for which this permit is issued (Sec. 3097, Civ. C).

Lender's Name _____ Lender's Address _____

LICENSED CONTRACTORS DECLARATION: I hereby affirm under penalty of perjury that I am licensed under provisions of Chapter 9 (commencing with section 7000) of Division 3 of the Business and Professions Code and my license is in full force and effect.

License Class B License Number 761970 Date 11-01-01 Contractor Signature [Signature]

OWNER-BUILDER DECLARATION: I hereby affirm under penalty of perjury that I am exempt from the contractors License Law for the following reason (Sec. 7031.5, Business and Professions Code; any city or county which requires a permit to construct, alter, improve, demolish, or repair any structure, prior to its issuance, also requires the applicant for such permit to file a signed statement that he or she is licensed pursuant to the provisions of the Contractors License Law (Chapter 9 (commencing with Section 7000) of Division 8 of the Business and Professions Code) or that he or she is exempt therefrom and the basis for the alleged exemption. Any violation of Section 7031.5 by any applicant for a permit subjects the applicant to a civil penalty of not more than five hundred dollars (\$500.00);

_____, I, as a owner of the property, or my employees with wages as their sole compensation, will do the work, and the structure is not intended or offered for sale (Sec. 7044, Business and Professional Code: The Contractors License Law does not apply to an owner of property who builds or improves thereon, and who does such work himself or herself or through his/her own employees, provided that such improvements are not intended or offered for sale. If, however, the building or improvement is sold within one year of completion, the owner-builder will have the burden of proving that he/she did not build or improve for the purpose of sale.)

_____, I, as owner of the property, am exclusively contracting with licensed contractors to construct the project (Sec. 7044, Business and Professions Code: The Contractors License Law does not apply to an owner of property who builds or improves thereon, and who contracts for such projects with a contractor(s) licensed pursuant to the Contractors License Law).

_____ I am exempt under Sec. _____ B & PC for this reason: _____

Date _____ Owner Signature _____

IN ISSUING THIS BUILDING PERMIT, the applicant represents, and the city relies on the representation of the applicant, that the applicant verified all measurements and locations shown on the application or accompanying drawings and that the improvement to be constructed does not violate any law or private agreement relating to permissible or prohibited locations for such improvements. This building permit does not authorize any illegal location of any improvement or the violation of any private agreement relating to location of improvements.

I certify that I have read this application and state that all information is correct. I agree to comply with all city and county ordinances and state laws relating to building construction and hereby authorize representative(s) of this city to enter upon the abovementioned property for inspection purposes.

Date 11-01-01 Applicant/Agent Signature [Signature]

WORKER'S COMPENSATION DECLARATION: I hereby affirm under penalty of perjury one of the following declarations:

_____ I have and will maintain a certificate of consent to self-insure for workers' compensation as provided for by Section 3700 of the Labor Code, for the performance of work for which the permit is issued.

2 I have and will maintain workers' compensation insurance, as required by Section 3700 of the Labor Code, for the performance of the work for which this permit is issued. My workers' compensation insurance carrier and policy number are:

Carrier AMERICAN CASULATY Policy Number WC247837616 Exp Date 05/01/2002

_____ (This section need not be completed if the permit is for \$100 or less) I certify that in the performance of the work for which this permit is issued, I shall not employ any person in any manner so as to become subject to the workers' compensation laws of California and agree that if I should become subject to the workers' compensation provisions of Section 3700 of the Labor Code, I shall forthwith comply with those provisions.

Date 11-01-01 Applicant Signature [Signature]

WARNING: FAILURE TO SECURE WORKER'S COMPENSATION COVERAGE IS UNLAWFUL AND SHALL SUBJECT AN EMPLOYER TO CRIMINAL PENALTIES AND CIVIL FINES UP TO ONE HUNDRED THOUSAND DOLLARS (\$100,000) IN ADDITION TO THE COST OF COMPENSATION, DAMAGES AS PROVIDED FOR IN SECTION 3706 OF THE LABOR CODE, INTEREST AND ATTORNEY'S FEE.

THIS PERMIT SHALL EXPIRE BY LIMITATION IF WORK IS NOT COMMENCED WITHIN 180 DAYS.

A P A

The Engineered Wood Association

**FIELD SERVICES DIVISION
CONFIDENTIAL COMPLAINT
OBSERVATION REPORT**

(For APA staff and manufacturer use only)

FC# 71-052-02
Ext. of FC# _____
CCA Ref. # _____
Mill Claim # 189837

This is not an official reinspection report.

FSD 7-19-02 Randall Carter
Field Mgr. _____
Field EWS 7-19-02 Mike Drorbaugh
Observed 7-24-02 Mike Drorbaugh

Verbal Report To Mill After Observation?
 Yes No
Date Called: 7-25-02
Report Mailed: 7-26-02

MANUFACTURER Louisiana-Pacific Corporation Phone: 800-642-7881
Address: Southern Division; P. O. Box 3107 Fax: 800-450-6109
City / State: Conroe, TX 77305 E-mail: Ernie.Pounders@LPCorp.com

AUTHORIZING OFFICIAL Ernest Pounders/Sherri White Title: Claims Analyst

Send copy of report & invoice to: Sherri White, Warranty/Technical Coordinator (Sherri.White@lpcorp.com)

PARTIES INVOLVED:

- | | Instructed Contact | Complaining Party | Panel Location |
|---|-------------------------------------|-------------------------------------|-------------------------------------|
| 1. <input type="checkbox"/> Wholesaler <input type="checkbox"/> Sales Agency
Company: _____
Address: _____
Contact: _____
Phone: _____ | <input type="checkbox"/> | <input type="checkbox"/> | <input type="checkbox"/> |
| 2. <input type="checkbox"/> Dealer <input type="checkbox"/> Wholesaler
Company: _____
Address: _____
Contact: _____
Phone: _____ | <input type="checkbox"/> | <input type="checkbox"/> | <input type="checkbox"/> |
| 3. <input checked="" type="checkbox"/> Other (Specify) <u>Builder</u>
Company: <u>Marchbrook Building Company</u>
Address: <u>P.O. Box 7576</u>
<u>Stockton, CA 95267-0576</u>
Contact: _____
Phone: _____ | <input checked="" type="checkbox"/> | <input type="checkbox"/> | <input type="checkbox"/> |
| 4. <input checked="" type="checkbox"/> Other (Specify) <u>Engineer</u>
Company: <u>Waleed Mari & Assoc.</u>
Address: <u>1020 15th St. Suite 22</u>
<u>Modesto, CA 95354</u>
Contact: _____
Phone: _____ | <input checked="" type="checkbox"/> | <input type="checkbox"/> | <input type="checkbox"/> |
| 5. <input checked="" type="checkbox"/> Other (Specify) <u>Homeowner</u>
Name: <u>Doug Park Residence</u>
Address: <u>21 Bluefeather Court</u>
<u>Sacramento, CA 95834</u>
Contact: _____
Phone: _____ | <input checked="" type="checkbox"/> | <input checked="" type="checkbox"/> | <input checked="" type="checkbox"/> |

299-521-3979

PANELS AT ISSUE: Mill # PTL #103 Forex Chambord/Louisiana-Pacific Corporation

No. Pieces	Thickness	Description / Grade
60	23/32	Uni-Floor (PS2-92/CSA O325 Single Floor 24 oc)

Date Shipped: _____ Car # _____ Order #: _____

DETAILS OF COMPLAINT:

Homeowner is complaining about ridges in the floor under carpet & pad.

OBSERVATION REQUIREMENTS: (Note: Bold or shaded items are added and/or revised.)

1. Handle ASAP. Always call to schedule observation.
2. **Urgent!** Requires fast service!!
3. Observe panels. Job site(s). Storage.
4. Verify trademarks, mill numbers; report all mill marks on panels.
5. Make arrangements, in advance, to have qualified workers present to remove a panel to verify trademarks, if necessary.
6. Determine and report nature and extent of problem.
7. Check random 10% (or as required) for alleged defects, and tally results. Check with the mill if necessary, for additional instructions. Job site(s) Storage
8. Furnish number of panels delaminating, and tally results. Job site(s) Storage
9. Give total number of panels used (and/or in storage).
10. Describe exposure and storage conditions.
11. Discuss allowables, and resolve complaint if appropriate.
12. Give applicable construction details (nailing, spacing, ventilation, alignment of framing, etc.)
13. Determine finishing materials and procedures. **(What coverings are used, area, and location)**
14. Take color pictures. Do not refer to or describe photos in text of the report.
 35mm prints & photos Take digital photos & send directly to mill via e-mail.
15. Obtain samples from sound stock for APA Lab. Include representative sample. Identify by complaint number.
16. Obtain a 3' x 4' representative sample for mill (with GTM, if possible). Identify sample and ship to: _____
17. Make arrangements, in advance, to have qualified workers present to remove siding, floor or roof covering. Call mill if this cannot be arranged before going on complaint.
18. If appropriate, discuss ways this condition can be avoided in the future.
19. Contact Sherri White @ 800-642-7881 ASAP to advise when the complaint will be observed.
20. **Call Ernie Ponders for additional details.**
21.

Always check with the mill for additional instructions if unexpected circumstances are encountered.
(Observation Report Attached)

OBSERVATIONS AND CONCLUSIONS

PANELS AT ISSUE: Mill # PTL #103 Forex Chambord

Description of trademarks:

Trademark follows PTL
Trademark Format for:

Uni-Floor
23/32 INCH
24 oc floor span
References to standards PS2-92
and CSA O325



NO APA STAMPS WERE SEEN

Spacing Stamp? ?

Plyclip Stamp? ?

Comments: One trademark observed

Panel Dimensions: 23/32" x 48" x 96" ; NA plies NA layers

#PANELS: Storage: -0- Job site: (unused) -0- (in place) 60 = 60 Total

Comments: Number of panels is estimated based on estimated area of second floor.

BACKGROUND:

According to the homeowner, Mr. Park, the initial building permit for his house located in Sacramento, CA was issued during November 1999 and the final building inspection was issued in March 2000. Since that time the Parks have brought up various construction issues with the builder. As related to the second story floor, about 1-month ago and according to Mr. Park, there were issues with the carpet and a carpet installer removed carpet from the upstairs rooms in preparation for new carpet. According to Mr. Park, the installer indicated he could not do a good job until the uneven floors were fixed. Several parties have been involved in trying to address the various construction issues involved and they decided to ask for an APA observation of the OSB flooring to determine if it was still satisfactory.

On July 24th I traveled from Seattle to Sacramento and met at the house with several interested parties who were present during my observations:

- Mr. Nicholas Buchberger, Principal Inspector for City of Sacramento, Planning & Building Department
- Mr. Ron Yasui, Structural Supervisor for City of Sacramento, Planning and Building Department
- Mr. Gene Horn, Superintendent, Marchbrook Building Company, the builder
- Mr. Waleed Mari, Engineer for the builder
- Mr. & Mrs. Doug Park, the homeowners

Mr. Park expressed concern about several OSB issues including: holes in the floor, unevenness of the floor, delamination, buckling, excessive deflection and high & low nails.

CONSTRUCTION & EXPOSURE DETAILS:

According to Mr. Park, the house was subject to rain for at least 30 days during the construction period. Some of the rainfall was heavy and I believe he indicated a period of at least 29 days of continuous rain when the house probably had no roof installed. The OSB floor panels appeared dry at the time of my observation. They were gray, dirty and had uneven surfaces and uneven edges consistent with moisture exposure. Nail heads were rusty.

CONSTRUCTION & EXPOSURE DETAILS: (continued)

The house is a two-story home of approximately 3,500 sq. ft. in size. It is built slab-on-grade foundation and the issues I was asked to address cover only the 1,860 sq. ft. of second floor panels. These 23/32 inch thick 24" oc span rated panels are installed over parallel chord metal plate-connected 2 x 4 wood trusses spaced 24" on center and spanning about 17 - 18 feet. Panels are attached to the trusses with 10d common nails spaced about 3 to 5" on center at panel ends and 6 to 12" on center at intermediate supports. I observed a few nail heads to be slightly raised above the panel surface in some places and many fasteners were slightly below the surface of the panels. According to the engineer, Mr. Mari, he could not immediately recall the diaphragm-nailing schedule called out for the floor.

In one small area checked it appears that construction glue was lightly applied to the top truss chord in a hit-and-miss fashion. There is no glue in the T&G joints. APA recommends glued floors include the use of a continuous bead of glue about 1/4" thick on supports and a smaller bead of glue in the T&G joints. Building codes do not mandate using glued floor construction. *LP recommends glue also.*

Panels observed have various amounts of spacing remaining at their ends and edges. In general, there is 0 to 1/8" of end spacing with many panel ends being tightly butted. Panel edges have more spacing remaining and gaps range from 1/16" to 1/4" with most panels having about 1/16" of edge spacing remaining. Spacing of 1/8" of panel ends and edges is recommended at the time of construction to allow for the dry wood panels to expand as they get wetted or as they acclimate to equilibrium moisture conditions. It is typical that panels exposed to moisture during construction exhibit less than 1/8" of spacing.

NATURE & EXTENT OF THE COMPLAINT:

I observed no mismanufactured panels. I observed no delaminated panels. Other than some small broken areas the panels appeared weathered but sound. Panel surfaces are rough but surface flakes are tight and not easily detached. Panel surfaces and interior edges appear firm when probed with a sharp tool. I did not observe surface voids or low-density surface spots. Panels are not rotted or moldy. A panel that was cut out of the floor was measured with a caliper accurate to 0.01" and it was 0.79 inches thick. This is a very normal interior area thickness measurement for 23/32" panels that start at about 0.72" of thickness at time of manufacture (nominal 23/32" is 0.719"). Panel ends/edges are thicker than this measurement but are not accessible for measurement on installed panels. There is no evidence that panels will further deteriorate if not subjected to further high moisture or further mechanical damage.

There are some serviceability issues with the panels. The most noticeable feature is panel edge swell and some minor panel buckling. Future references to edge swell includes panel end condition as well as edges. Most panels show thickened edges of about 1/16" to 1/8", and most of the panel's joints are uneven due to variable thickness of edges. This pattern of edge swell is not uncommon for OSB exposed to several days of wetting. APA recommends that uneven edges be sanded as necessary to relieve edge swell. Edge swell not a manufacturing defect.

Typical industry performance criteria for floors is that they should be flat within 1/4" for any 32" x 32" area and that within any 20' of floor no spot should be more than 1/2" lower or higher. Some panels were observed that appeared to be bowed downward between supports about 1/4" (excluding edge swell). In places where there is both edge swell and bowing, a height difference of about 1/2" was noted. Panel buckling did not appear to be widespread nor did it appear to be more than about 1/4" in the 8 to 10 places where I could detect downward bowing and checked it with a straight edge.

There are places under walls where the floor appears to be lower than the field of the floors. In one case where measurements were made the panel surface was 9/16" lower than the panel over the truss 12" away. This was because the panel was cut for an air duct that ran through the wall at that point, and structural blocking did not support the narrow panel width near the cut. Some wall lines appear offset from trusses in places and the floor appeared slightly lower (about 1/2" or less) in these areas.

NATURE & EXTENT OF THE COMPLAINT: (continued)

Panel edge swell is the primary reason some nail heads are below the panel surface. This is typical and APA testing has shown that nails below the surface due to edge swell need not be considered as damaging the diaphragm action of the floors. Nails that are high are probably from nail pop. Nail pop is caused by shrinkage of framing (and panels) after they have been nailed. Nail pops represented less than 5% of the total nails. Nail pops occur primarily due to moisture changes. Due to the stable moisture environment of the panels and framing at this time I do not believe additional nail pop will occur.

There are floor squeaks and noises in several areas of the floor that appear to be related to the nails. While these may be irritating they do not represent structural defects with the panels or floor system. Proper gluing of the panels and the T&G joint greatly reduces floor noises but cannot totally eliminate them. Screw fasteners are often used to reduce floor noises.

Panel strength and stiffness appeared normal to me. Although not carefully measured, deflections of most panels appeared to be about 1/10" to 1/8" under my weight of 225 pounds. This is consistent with performance criteria for this type of panel when tested after a single wet/redry cycle at this span (0.108" of deflection with 200 pounds of load). Panels subjected to repeated wet/redry cycles may not meet manufacturing performance criteria for deflection after a single wet/redry cycle, but this performance loss due to extra weathering does not indicate mismanufacture. Some panels exhibited more deflection than 1/8 inch. In particular, two panels with an improperly fitted T&G joint (too much spacing) had more deflection and two other panels near a door are broken at the T&G joint.

Several holes were noted in the floors. In perhaps 8 - 10 places I observed what appeared to be holes caused by a hammer. These holes were typically 1 to 1-1/2" in diameter. In many places there were smaller holes, less than 1/4" in diameter. These smaller holes appeared to be nailing holes coming up from below. While it is not advisable to pound holes through panels with a hammer, it did appear that the hammer holes were of minimum structural significance although they were big enough to be noticed through pad and carpet. The smaller diameter holes (from nails?) are of little structural significance.

Most of the serviceability issues raised by the homeowners about the OSB can be reasonably addressed with some sanding of the uneven panel edges and the addition of a thin layer of plywood underlayment. The underlayment would help even the floor, bridge small structural imperfections and reduce differential deflection between adjacent panels. Mr. Ron Yasui, Structural Supervisor for City of Sacramento, Planning and Building Department suggested a thin foam layer between the panels as is sometimes used for hardwood composite flooring. Mr. Yasui suggests that the foam layer may help reduce the potential for floor noises that might occur because of differential deflection of the two floor layers.

The OSB floor panels were identified based on viewing a trademark from a panel cut from the floor. In addition, I matched this trademark with the green panel edge seal and noted that except for a strip of 2' x 16' in one room, all other panels appeared to have the same green edge seal and same general flake geometry. The panel manufacturer is: Forex Chambord; PTL stamped as Uni-Floor 24 INCH O.C.; ROOF 48 INCH O.C.; 23/32 INCH; EXPOSURE 1; HUD ???; PRP-232?; PS2-92; MILL 103.

RECOMMENDATIONS/SPECIFICATIONS DISCUSSED WITH COMPLAINANT:

Yes No Non-applicable

TALLY ATTACHED? NO PICTURES COMING? YES SAMPLE (To Mill)? NO

FLOOR CONSTRUCTION DETAILS

FC # 71-052-02

Page 1 of 2

Panel Location First Floor Second Floor Other _____

Panel Use: Subfloor Single Floor Underlayment

Panel Length Orientation: Parallel Perpendicular

Floor Supports: Joists Trusses Other _____

Joist/Chord Size: 2x4 2x10 Other _____

Lumber Grade & Species: _____ S-Dry S-Grn 15%

Spacing: 24 " oc Maximum Span Observed: 17-18'

Foundation Type: Crawl Space Basement Slab Other _____

Is the Basement/Crawl Space Dry? Yes No Moist/Damp Standing Water Signs of Standing Water

Condition When Observed: Wet Dry Other _____

Is Site Drainage a Factor? Yes No If Yes, Explain _____

Is a Ground Cover Vapor Retarder Used? Yes No

Is the Crawl Space Well Ventilated (if used)? Yes No

Size of Foundation Vents: _____ Number of Vents: _____

Was the Crawl Space Ventilation Calculated? Yes No

Are the Calculations Attached? Yes No

Insulated Floor? Yes No If Yes, Describe: _____

Is the Framing Aligned? Yes No If No, Explain: UNKNOWN AND NOT CHECKED

Have the Panels Been Wet? Yes No Unknown

Condition of Panels When Observed: Wet Dry Moist/Damp Other _____

Are Panels Tight to Framing? Yes No Not in All Places

Fastener Information:

End Spacing _____ " oc; Varies from 3 " to 5 " oc

Intermediate Spacing: _____ " oc; Varies from 6 " to 12 " oc

Edge/Field Spacing: _____ " oc; Varies from _____ " to _____ " oc

Max. Edge Distance: _____ " Varies from 1/4 " to 1 "

FLOOR CONSTRUCTION DETAILS (Continued)

FC # 71-052-02

Page 2 of 2

Fastener Type: Nails Staples Size: 10d AFG-01 Glue Other UNKNOWN

Shank Type: Smooth Ring Screw Other _____

Fastening Applies to: Generally, All Panels Only Where Observed

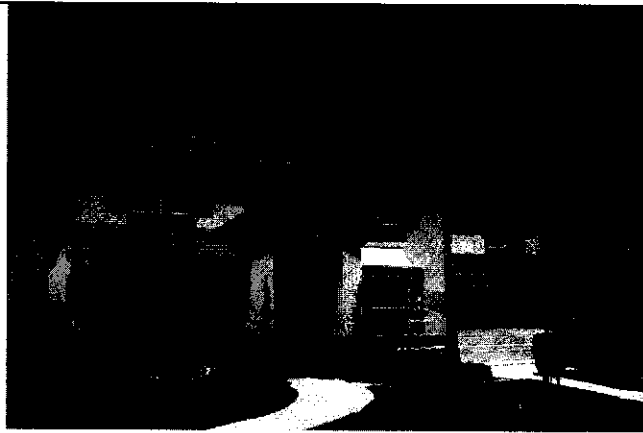
Panel Spacing Information:

Panel Edge Spacing: _____ "; Varies from 1/16 " to 1/4 "

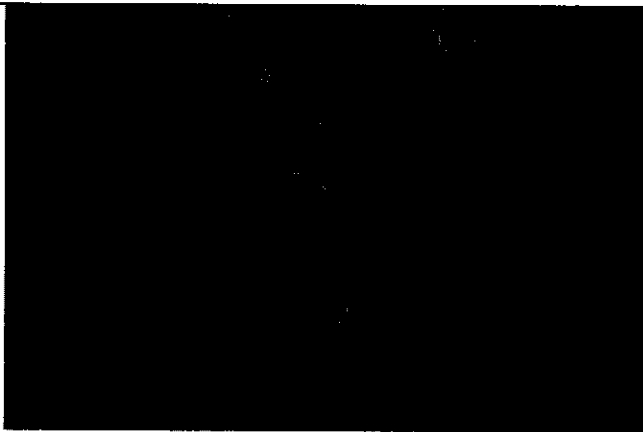
Panel End Spacing: _____ "; Varies from 0 " to 1/8 "

Spacing Applies to: Generally, All Panels Only Where Observed

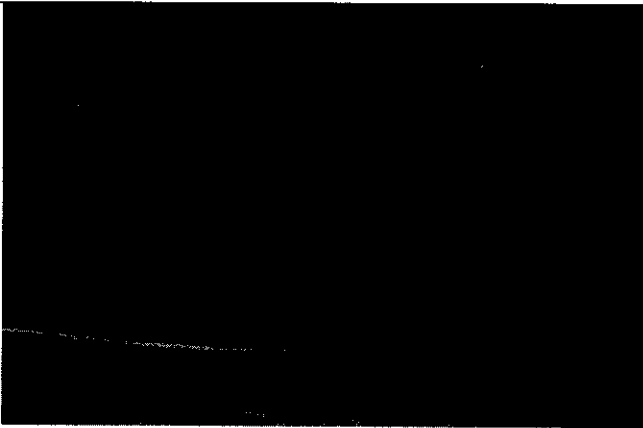
Additional Information:

PHOTO DESCRIPTION LIST**Photo #1:****Photo description:**

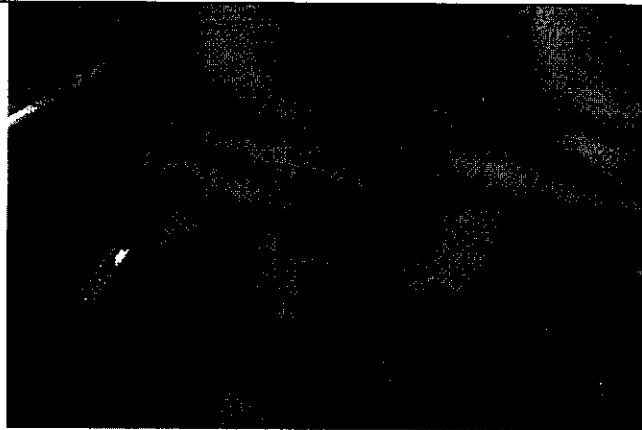
House at 21 Bluefeather Court, Sacramento, CA belonging to Mr. & Mrs. Doug Park. House is 3500 s.f. and second floor is about 1860 s.f.

Photo #2:**Photo description:**

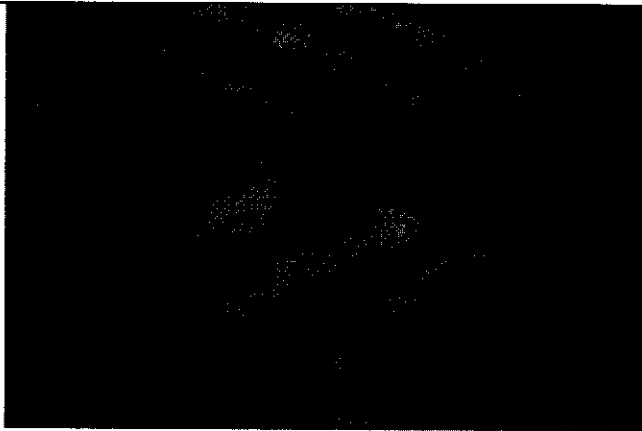
Trademark observed on one panels cut from second floor.

Photo #3:**Photo description:**

Panels are installed over parallel chord trusses 16" deep and spaced 24" oc. They span about 17 - 18 feet.

PHOTO DESCRIPTION LIST**Photo #4:****Photo description:**

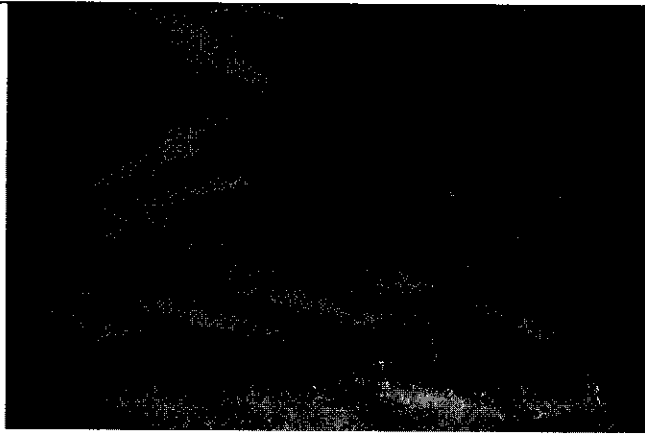
Typical appearance of floor.

Photo #5:**Photo description:**

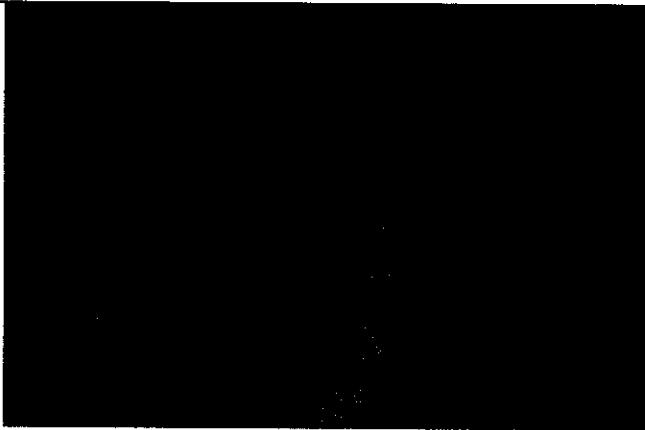
Typical buckled panels plus edge swell. This one measured 3/8" .

Photo #6:**Photo description:**

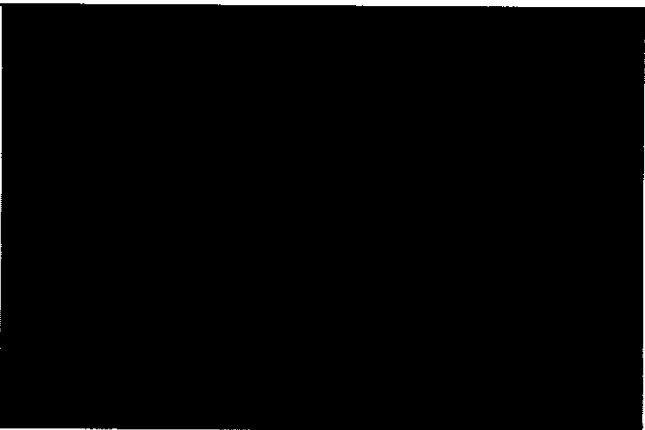
Wall lower than floor. In this case the marked difference was due to an air duct hole cut into the floor (duct running up through the wall) and the narrow strip of OSB was not blocked. This is 9/16" height difference between edge swelled ridge and surface of deflected floor.

PHOTO DESCRIPTION LIST**Photo #7:****Photo description:**

Typical floor appearance showing end nailing.

Photo #8:**Photo description:**

Uneven surfaces due to edge swell.

Photo #9:**Photo description:**

Hole in floor appeared to have been made with a hammer.

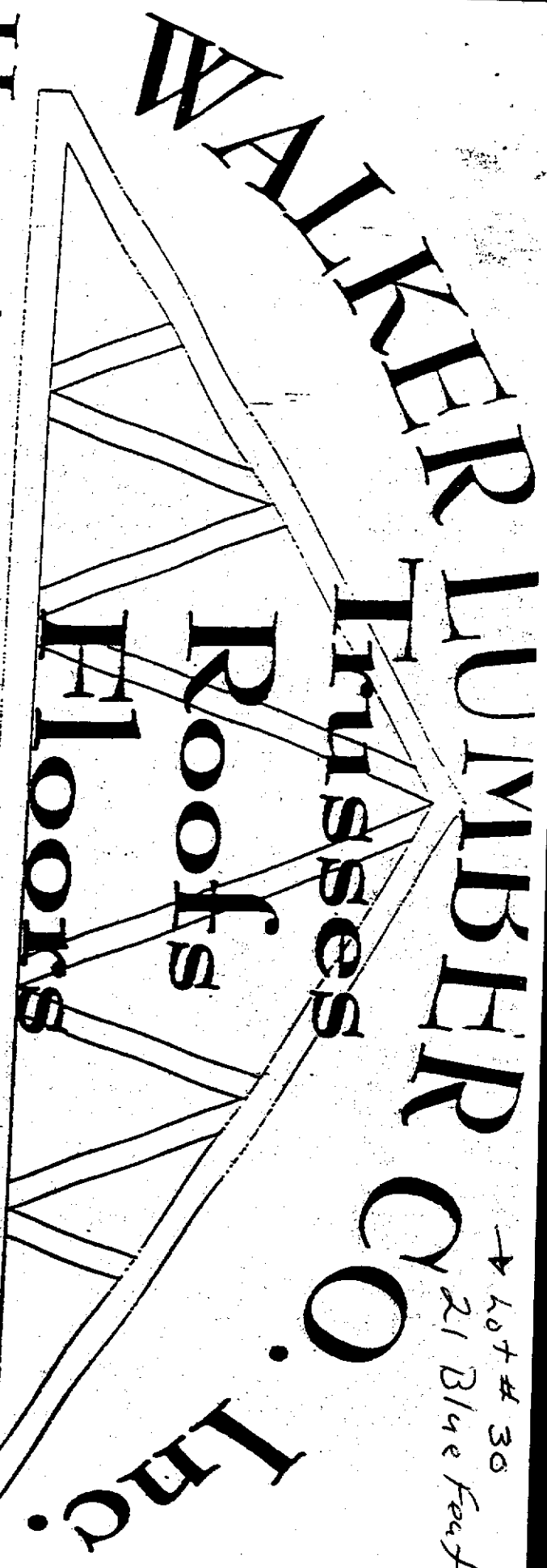
PHOTO DESCRIPTION LIST**Photo #10:****Photo description:**

Note yellow chalk circles. They indicate row of holes that come up from below floor and appeared to be nail holes.

Photo #11:**Photo description:**

WALKER LUMBER CO. Inc.

Trusses
Roofs
Floors

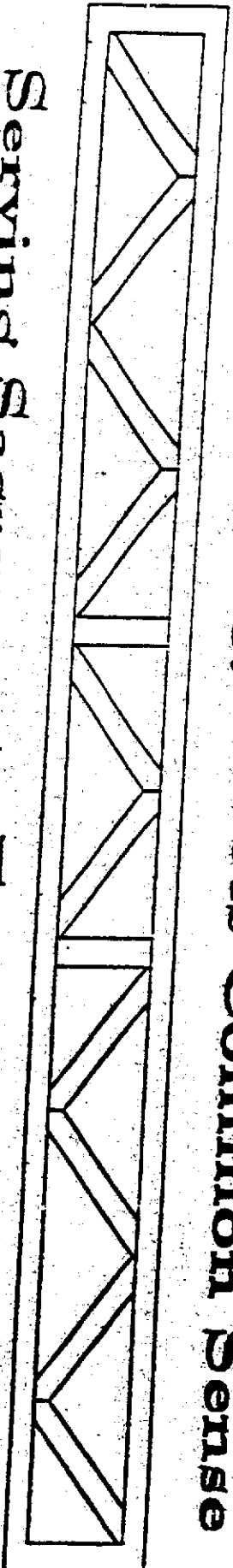


Lot # 30
21 Blue Feather Ct

Http://www.walker-lumber.com

(916) 338-2121 Fax (916) 338-5353

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Construction

Job Name

Salvage Plant

Phon 41 51

JOB NUMBER

44712

H4712A.r1100 Wednesday, January 05, 2000

11-23-99

H4712

1-11-00

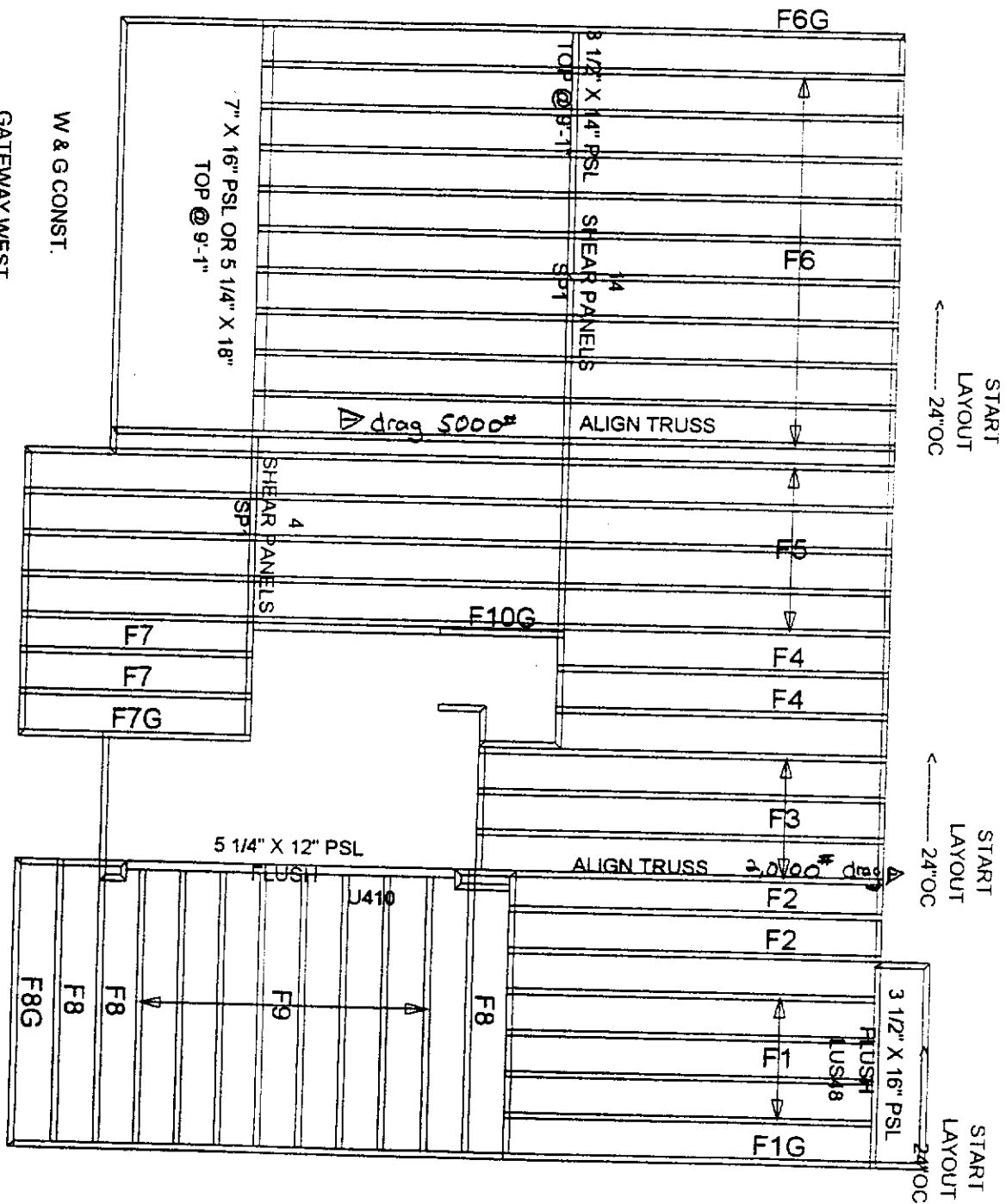
PLAN 4 ELEV. ALL (FLOORS)

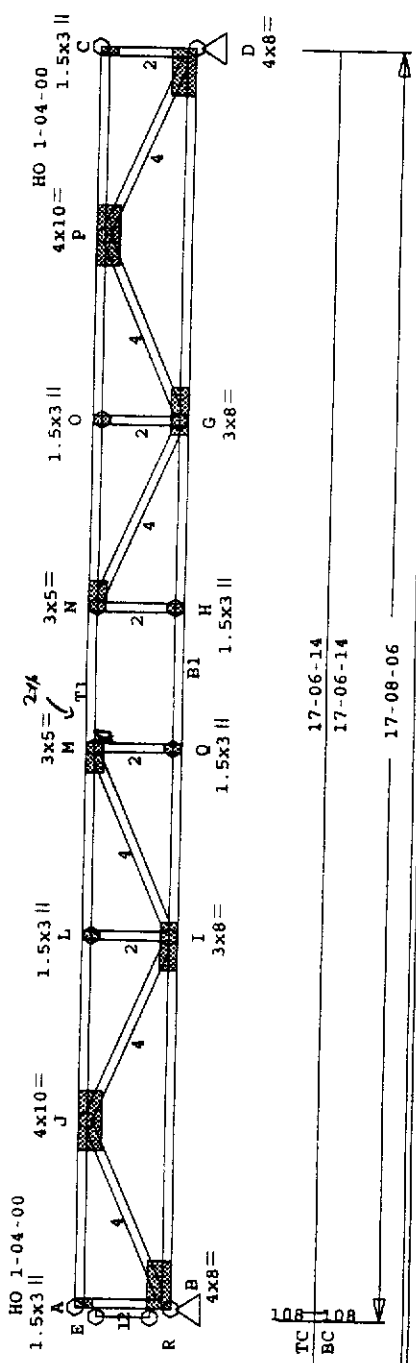
WIDE LOT SERIES

GATEWAY WEST

W & G CONST.

BEAM SIZES AND
LOCATIONS SEE PLANS.





Unistar -- Version 40.0.110
RUN DATE: 1-13-00

CSI SIZE LUMBER 1.15FB
TOP 0.45 4X 2 DFL-#1B 2183
BTM 0.64 4X 2 DFL-#1B 2183
WBS 0.75 4X 2 HF-STAN 698
LUMBER STRESS INCREASE: 0.0%
REPETITIVE MEMBER STRESS USED.

LATERAL BRACING:
TOP CHORD - CONTINUOUS
BTM CHORD - CONTINUOUS
TRUSS SPACING - 24.0 IN.

LOADING LIVE DEAD (PSF)
TOP CHD 40.0 10.0
BTM CHD 0.0 5.0
TOTAL 40.0 15.0 55.0

SUPPORT CRITERIA
JT REACT WIDTH JT REACT WIDTH
LBS IN-SX LBS IN-SX
B 966 3-8 D 966 1-12

MEMBER CSI P(LBS) MØ1ST MØ2ND
TOP CHORDS
A-J 0.27 0 T 0 -774
J-L 0.45 2978 C 774 -566
L-M 0.38 2978 C 566 -425
M-N 0.41 3493 C 425 -425
N-O 0.38 2978 C 425 -567
O-P 0.45 2978 C 567 -774
P-C 0.27 0 T 774 0
BOTTOM CHORDS
B-I 0.41 1819 T 0 -195
I-Q 0.64 3493 T 195 108
Q-H 0.63 3493 T -108 108
H-G 0.64 3493 T -108 -195

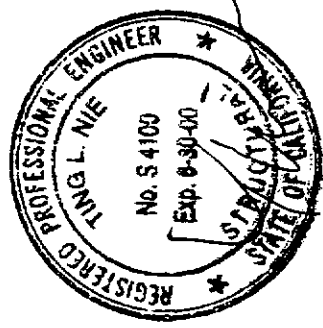
MEMBER CSI P(LBS) MØ1ST MØ2ND
G-D 0.41 1819 T 195 0
B-A = 107 C B-J = 2002 C
J-I = 1281 T I-L = 258 C
I-M = 568 C Q-M = 14 T
H-N = 14 T N-G = 568 C
G-O = 258 C G-P = 1281 T
P-D = 2002 C D-C = 107 C

DL+LL DEFL = 0.34" IN O-H
LL DEFL = 0.25" < BRG-SPAN/360
SPAN/DEFL (DL+LL) = 618

PLATING CONFORMS TO TPI-95
PLATE VALUES MAY BE VERIED
GRIP BASED ON DF/HF LUMBER
USING GROSS AREA TEST.
GRIP REDUCED 20% FOR
M.C. > 19%
PLATES - 20 GAUGE LOCK
GRIPPING 330-137 PSI PER PAIR
INCLUDES 0.0% INCREASE
TENSION 1339- 465 PLI PER PAIR
SHEAR 784- 506 PLI PER PAIR

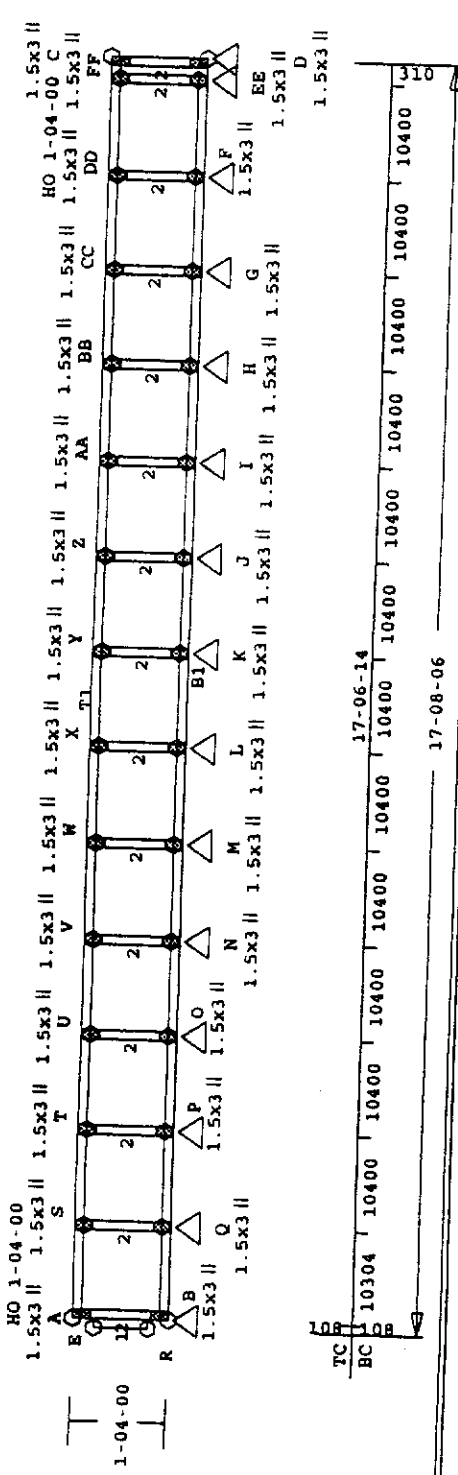
JT TYPE PLATE SIZE X Y
A 4000 1.50 X 3.00 CTR CTR
B 4010 4.00 X 8.00 1.5 1.5
C 4000 1.50 X 3.00 CTR CTR
D 4010 4.00 X 8.00 1.5 1.5
G 1070 3.00 X 8.00 5.6 CTR
H 1001 1.50 X 3.00 CTR CTR
I 1070 3.00 X 8.00 2.4 CTR
J 1010 4.00 X10.00 5.0 1.5
L 1001 1.50 X 3.00 CTR CTR
M 1030 3.00 X 5.00 1.5 CTR
N 1050 3.00 X 5.00 3.5 CTR
O 1001 1.50 X 3.00 CTR CTR
P 1010 4.00 X10.00 5.0 1.5
Q 1001 1.50 X 3.00 CTR CTR

NOTES:
1. TRUSSES MANUFACTURED BY - WALKER LUMBER
2. ANALYSIS CONFORMS TO TPI (ANSI/TPI 1-1995).
3. PROVIDE 2X6 CONTINUOUS STRONGBACKS (ON EDGE) AT EVERY FASTEN TO EACH TRUSS W/ 3-10d NAILS.



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CHECKED JAN 13 2000



UniStar -- Version 40.0.110
 RUN DATE: 1-5-00

CSI SIZE LUMBER 1.15FB
 TOP 0.07 4X 2 DFL-#1B 2183
 BTM 0.01 4X 2 DFL-#1B 2183
 WBS 0.05 4X 2 HF-STAN 698
 LUMBER STRESS INCREASE: 0.0%
 REPETITIVE MEMBER STRESS USED.

LATERAL BRACING:
 TOP CHORD - CONTINUOUS
 BTM CHORD - CONTINUOUS
 TRUSS SPACING - 24.0 IN.

LOADING LIVE DEAD (PSF)
 TOP CHD 40.0 10.0
 BTM CHD 0.0 5.0
 TOTAL 40.0 15.0 55.0

SUPPORT CRITERIA
 BOTTOM CHORD CONTINUOUSLY
 SUPPORTED ALONG ENTIRE LENGTH.

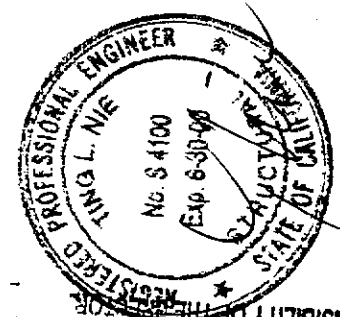
MEMBR	CSI	P (LBS)	M01ST	M02ND
CC-DD	0.07	3 C	174	-191
DD-FF	0.07	3 C	191	-121
FF-C	0.04	3 C	121	-38
BOTTOM CHORDS				
B-Q	0.01	3 T	0	-21
Q-P	0.01	3 T	21	-16
P-O	0.01	3 T	16	-18
O-N	0.01	3 T	18	-17
N-M	0.01	3 T	17	-17
M-L	0.01	3 T	17	-17
L-K	0.01	3 T	17	-17
K-J	0.01	3 T	17	-17
J-I	0.01	3 T	17	-17
I-H	0.01	3 T	17	-17
H-G	0.01	3 T	17	-17
G-F	0.01	3 T	17	-17
F-EE	0.01	3 T	18	-14
EE-D	0.01	3 T	14	0
WEBS				
B-A	0.04	54 C	0	-38
D-C	0.05	8 T	0	38
Q-U	143 C	P-T	132 C	
M-W	134 C	N-V	134 C	
K-Y	134 C	L-X	134 C	
I-AA	134 C	J-Z	134 C	
G-CC	132 C	H-BB	139 C	
EE-FF	101 C	F-DD		

MEMBR	CSI	P (LBS)	M01ST	M02ND
A-S	0.07	3 C	38	-198
S-T	0.07	3 C	198	-172
T-U	0.06	3 C	172	-179
U-V	0.06	3 C	179	-177
V-W	0.06	3 C	177	-177
W-X	0.06	3 C	177	-177
X-Y	0.06	3 C	177	-177
Y-Z	0.06	3 C	177	-177
Z-AA	0.06	3 C	177	-177
AA-BB	0.06	3 C	177	-178
BB-CC	0.06	3 C	178	-174

PLATING CONFORMS TO TPI-95
 PLATE VALUES MAY BE VERRIED
 GRIP BASED ON DF/HF LUMBER
 USING GROSS AREA TEST.
 GRIP REDUCED 20% FOR
 M.C. > 19%.

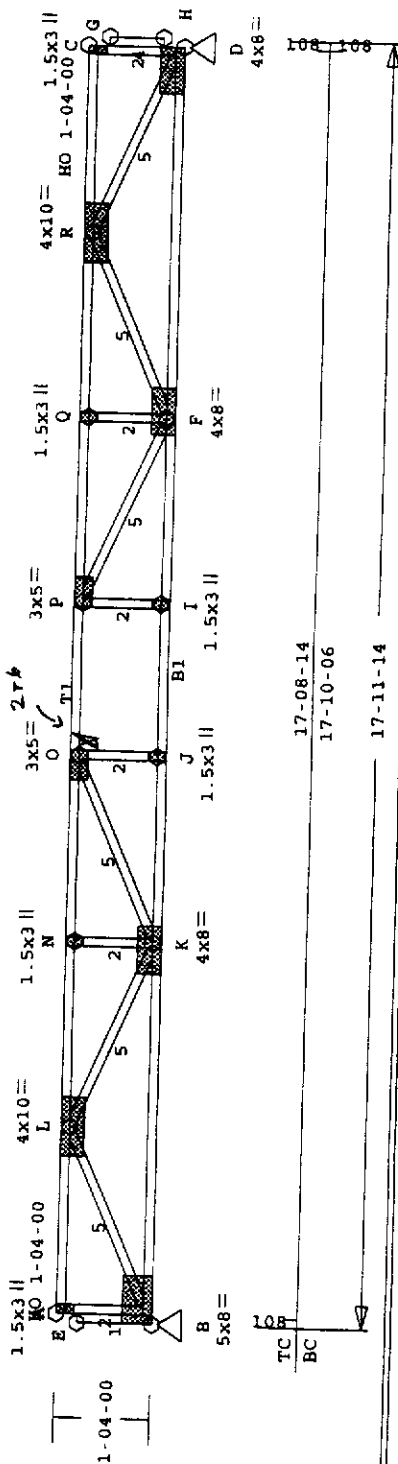
PLATES - 20 GAUGE LOCK
 GRIPPING 330-137 PSI PER PAIR
 INCLUDES 0.0% INCREASE
 TENSION 1339- 465 PLI PER PAIR
 SHEAR 784- 506 PLI PER PAIR

NOTES:
 1. TRUSSES MANUFACTURED BY -
 WALKER LUMBER
 2. ANALYSIS CONFORMS TO
 TPI (ANSI/TPI 1-1995).



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CHECKED JAN 13 2000



UniStar -- Version 40.0.110
RUN DATE: 1-13-00

CSI	SIZE LUMBER	1.15FB
TOP 0.45	4X 2 DFL-#1B	2183
BTM 0.65	4X 2 DFL-#1B	2183
WBS 0.76	4X 2 HF-STAN	698
LUMBER STRESS INCREASE: 0.0%		
REPETITIVE MEMBER STRESS USED.		

LATERAL BRACING:

TOP CHORD - CONTINUOUS
BTM CHORD - CONTINUOUS
TRUSS SPACING - 24.0 IN.

LOADING	LIVE	DEAD (PSF)
TOP CHD	40.0	10.0
BTM CHD	0.0	5.0
TOTAL	40.0	15.0

SUPPORT CRITERIA

JT	REACT WIDTH	JT	REACT WIDTH		
LBS	IN-SX	LBS	IN-SX		
B	975	3-8	D	975	3-8

MEMBER	CSI	P (LBS)	MØ1ST	MØ2ND
TOP CHORDS				
A-L	0.27	0	T	-776
L-N	0.45	3016	C	776
N-O	0.38	3016	C	557
O-P	0.35	3555	C	456
P-Q	0.38	3016	C	456
Q-R	0.45	3016	C	557
R-C	0.27	0	T	776
BOTTOM CHORDS				
B-K	0.41	1839	T	0
K-J	0.65	3555	T	193
J-I	0.64	3555	T	-104
I-F	0.65	3555	T	-104
F-D	0.41	1839	T	193

MEMBR CSI P (LBS) MØ1ST MØ2ND

B-E	=	54	C	B-E	=	54	C
E-A	=	107	C	B-L	=	2024	C
L-K	=	1302	T	K-N	=	256	C
K-O	=	594	C	J-O	=	15	T
I-P	=	15	T	P-F	=	594	C
F-Q	=	256	C	F-R	=	1302	T
R-D	=	2024	C	D-C	=	107	C

DL+LL DEFL = 0.35" IN J-I
LL DEFL = 0.26" < BRG-SPAN/360
SPAN/DEFL (DL+LL) = 602

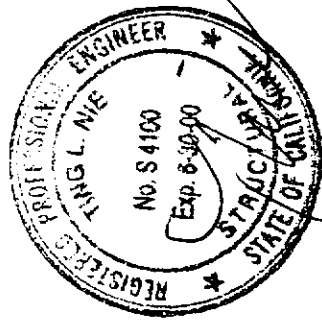
PLATING CONFORMS TO TPI-95

PLATE VALUES MAY BE VERIED
GRIP BASED ON DF/HF LUMBER
USING GROSS AREA TEST.
GRIP REDUCED 20% FOR
M.C. > 19%.

PLATES - 20 GAUGE LOCK
GRIPPING 330-137 PSI PER PAIR
INCLUDES 0.0% INCREASE
TENSION 1339-465 PLI PER PAIR
SHEAR 784-506 PLI PER PAIR

JT	TYPE	PLATE	SIZE	X	Y
A	4000	1.50	X 3.00	CTR	CTR
B	4010	5.00	X 8.00	3.0	1.5
C	4000	1.50	X 3.00	CTR	CTR
D	4010	4.00	X 8.00	1.5	1.5
E	1070	4.00	X 8.00	5.2	1.5
I	1001	1.50	X 3.00	CTR	CTR
J	1001	1.50	X 3.00	CTR	CTR
K	1070	4.00	X 8.00	2.8	1.5
L	1010	4.00	X10.00	5.0	1.5
N	1001	1.50	X 3.00	CTR	CTR
O	1030	3.00	X 5.00	1.5	CTR
P	1050	3.00	X 5.00	3.5	CTR
Q	1001	1.50	X 3.00	CTR	CTR
R	1010	4.00	X10.00	5.0	1.5

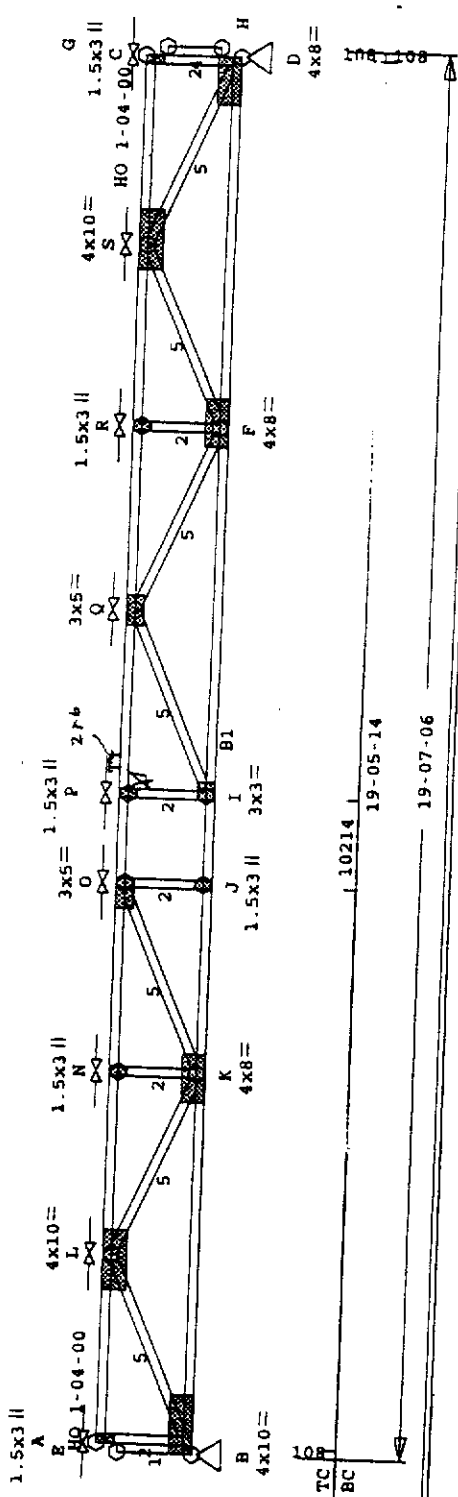
NOTES:
1. TRUSSES MANUFACTURED BY - WALKER LUMBER
2. ANALYSIS CONFORMS TO TPI (ANSI/TPI 1-1995).
3. PROVIDE 2X6 CONTINUOUS STRONGBACKS (ON EDGE) AT EVERY FASTEN TO EACH TRUSS W/ 3-10d NAILS.



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CHECKED JAN 13 2000

COLLECTOR LOAD 2000



Unistar -- Version 40.0.110
 RUN DATE: 1-13-00

CSI SIZE LUMBER 1.15FB
 TOP 0.63 4X 2 DFL-#1B 2183
 BTM 0.63 4X 2 DFL-#1B 2183
 WBS 0.88 4X 2 HF-STAN 698
 REPETITIVE MEMBER STRESS USED.

LATERAL BRACING:
 TOP CHORD - CONTINUOUS
 BTM CHORD - CONTINUOUS
 TRUSS SPACING - 24.0 IN.

LOAD CASE #1
 LUMBER STRESS INCREASE: 0.04
 PLATE STRESS INCREASE: 0.04
 LOADING LIVE DEAD (PSF)
 TOP CHD 40.0 10.0
 BTM CHD 0.0 5.0
 TOTAL 40.0 15.0
 SUPPORT CRITERIA
 JT REACT WIDTH JT REACT WIDTH
 LBS IN-SX LBS IN-SX
 B 1065 3-8 D 1065 3-8

LOAD CASE #2
 LUMBER STRESS INCREASE: 33.04
 PLATE STRESS INCREASE: 0.04
 LOADING LIVE DEAD (PSF)
 TOP CHD 0.0 10.0
 BTM CHD 0.0 5.0
 TOTAL 0.0 15.0
 CONCENTRATED LOADS (LBS)
 A 163 RIGHT L 264 RIGHT
 N 264 RIGHT O 197 RIGHT
 P 193 RIGHT Q 261 RIGHT
 R 261 RIGHT S 264 RIGHT
 C 133 RIGHT

SUPPORT CRITERIA
 JT TYPE HORZ VERT WIDTH
 LBS LBS IN-SX
 B PIN 1999 415 3-8
 D HORZ RLR 0 165 3-8

MEMBR CSI P(LBS) MOIST MO2ND
 TOP CHORDS
 A-L 0.28 163 C 0 -59
 L-N 0.50 3383 C 804 -406
 N-O 0.58 3383 C 406 -986
 O-P 0.58 4203 C 986 357
 P-Q 0.63 4203 C -357 -783
 Q-R 0.50 3400 C 783 -429
 R-S 0.50 3400 C 429 -799
 S-C 0.28 133 T 136 0
 BOTTOM CHORDS
 B-K 0.46 2262 T 0 -301
 K-J 0.82 4203 T 102 -380
 J-I 0.82 4203 T 380 321
 I-F 0.83 4125 T -321 -327
 F-D 0.45 2039 T 327 0
 WEBS
 B-E 53 C B-E 53 C
 E-A 106 C B-L 2237 C
 L-K 1494 T K-N 229 C
 K-O 903 C J-O 91 T
 I-P 101 T I-Q 396 T
 Q-F 802 C F-R 233 C
 F-S 1505 T S-D 2245 C
 D-C 106 C

LOAD CASE #3
 LUMBER STRESS INCREASE: 33.04
 PLATE STRESS INCREASE: 0.04
 LOADING LIVE DEAD (PSF)
 TOP CHD 0.0 10.0
 BTM CHD 0.0 5.0
 TOTAL 0.0 15.0
 CONCENTRATED LOADS (LBS)
 A 163 LEFT L 264 LEFT
 N 264 LEFT O 197 LEFT
 P 193 LEFT Q 261 LEFT
 R 261 LEFT S 264 LEFT
 C 133 LEFT

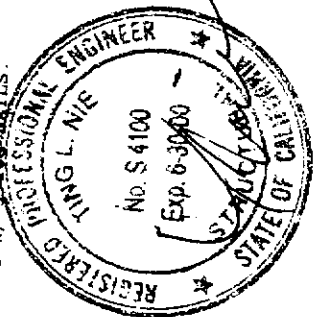
SUPPORT CRITERIA
 JT TYPE HORZ VERT WIDTH
 LBS LBS IN-SX
 B PIN 1999 415 3-8
 D HORZ RLR 0 165 3-8

MEMBR CSI P(LBS) MOIST MO2ND
 TOP CHORDS
 A-L 0.28 163 C 0 -59
 L-N 0.50 3383 C 804 -406
 N-O 0.58 3383 C 406 -986
 O-P 0.58 4203 C 986 357
 P-Q 0.63 4203 C -357 -783
 Q-R 0.50 3400 C 783 -429
 R-S 0.50 3400 C 429 -799
 S-C 0.28 133 T 136 0
 BOTTOM CHORDS
 B-K 0.46 2262 T 0 -301
 K-J 0.82 4203 T 102 -380
 J-I 0.82 4203 T 380 321
 I-F 0.83 4125 T -321 -327
 F-D 0.45 2039 T 327 0
 WEBS
 B-E 53 C B-E 53 C
 E-A 106 C B-L 2237 C
 L-K 1494 T K-N 229 C
 K-O 903 C J-O 91 T
 I-P 101 T I-Q 396 T
 Q-F 802 C F-R 233 C
 F-S 1505 T S-D 2245 C
 D-C 106 C

DL+LL DEFL - 0.55" IN P-Q
 LL DEFL - 0.39" < BRG-SPAN/360
 SPAN/DEFL (DL+LL) - 425

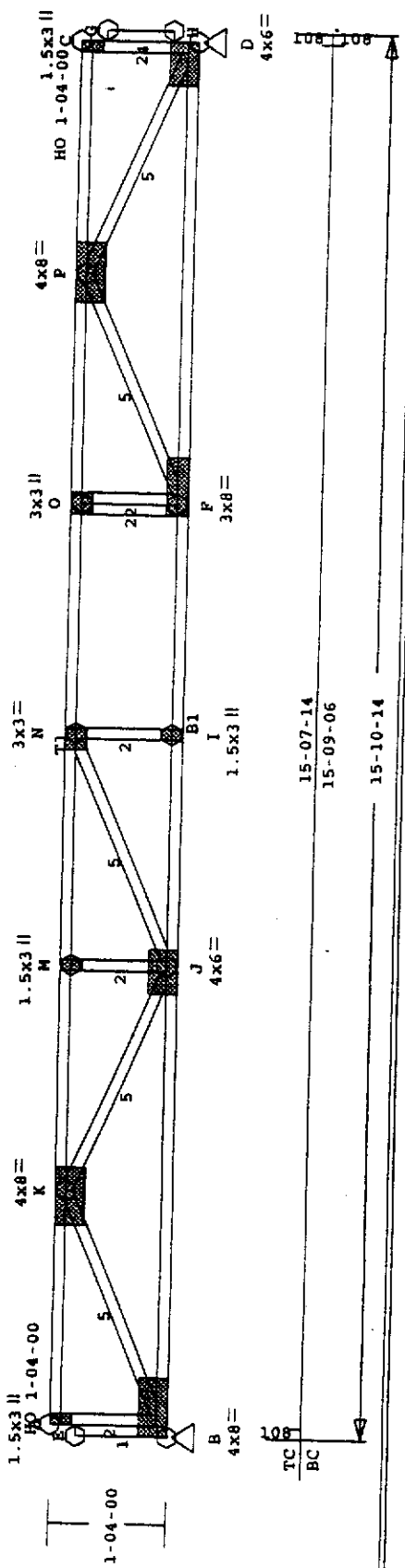
NOTES:
 1. TRUSSES MANUFACTURED BY - WALKER LUMBER
 2. ANALYSIS CONFORMS TO TPI (ANSI/TPI 1-1995).
 3. ANCHOR TRUSS FOR A TOTAL HORIZONTAL LOAD OF 2000 LBS.
 4. PROVIDE 2X6 CONTINUOUS STRONGBACKS (ON EDGE) AT EVERY FASTEN TO EACH TRUSS W/ 3-198-NAILS.

PLATING CONFORMS TO TPI-95
 PLATE VALUES MAY BE VERIFIED
 GRIP BASED ON DF/HF LUMBER
 USING GROSS AREA TEST.
 GRIP REDUCED 20% FOR
 M.C. > 19%
 PLATES - 20 GAUGE LOCK
 GRIPPING 330-137 PSI PER PAIR
 INCLUDES 0.04 INCREASE
 TENSION 1339-465 PLI PER PAIR
 SHEAR 784-506 PLI PER PAIR



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CHECKED JAN 13 2000



Unistar -- Version 40.0.110
 RUN DATE: 1-13-00

CSI	SIZE LUMBER	1.15FB
TOP 0.80	4X 2 DFL-#1B	2183
BTM 0.85	4X 2 DFL-#1B	2183
WBS 0.69	4X 2 HF-STAN	698

EXCEPTIONS:
 F-O 2 4X 2 HF-STAN 698
 LUMBER STRESS INCREASE: 0.04
 REPETITIVE MEMBER STRESS USED.

MEMBER	CSI	P (LBS)	M01ST	M02ND
I-F	0.85	2670 T	-1171	-936
F-D	0.59	1597 T	936	0

WEBS
 B-E = 55 C B-E = 55 C
 E-A = 109 C B-K = 1753 C
 K-J = 1066 T J-M = 308 C
 J-N = 126 C I-N = 91 C
 F-O = 373 C F-P = 1182 T
 P-D = 1758 C D-C = 115 C

DL+LL DEFL = 0.41" IN M-N
 LL DEFL = 0.31" < BRG-SPAN/360
 SPAN/DEFL (DL+LL) = 459

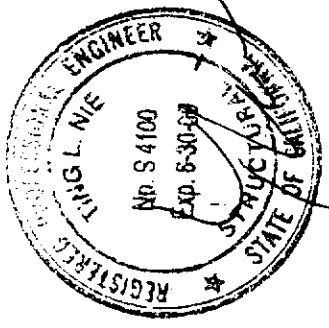
LATERAL BRACING:
 TOP CHORD - CONTINUOUS
 BTM CHORD - CONTINUOUS
 TRUSS SPACING - 24.0 IN.

LOADING	LIVE	DEAD	(PSF)
TOP CHD	40.0	10.0	
BTM CHD	0.0	5.0	
TOTAL	40.0	15.0	55.0

PLATING CONFORMS TO TPI-95
 PLATE VALUES MAY BE VERIED
 GRIP BASED ON DF/HF LUMBER
 USING GROSS AREA TEST.
 GRIP REDUCED 20% FOR
 M.C. > 19%
 PLATES - 20 GAUGE LOCK
 GRIPPING 330-137 PSI PER PAIR
 INCLUDES 0.04 INCREASE
 TENSION 1339- 465 PLI PER PAIR
 SHEAR 784- 506 PLI PER PAIR

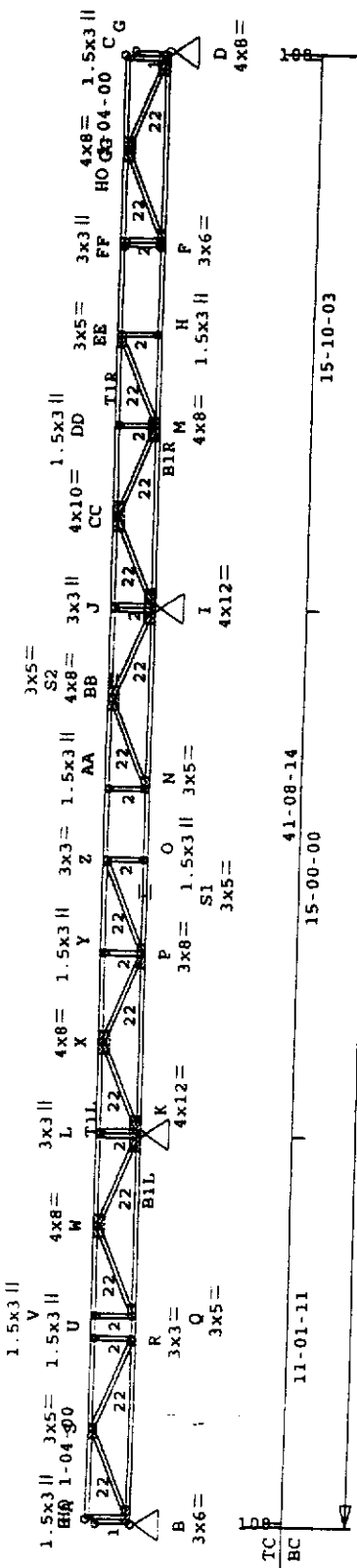
JT TYPE	PLATE	SIZE	X	Y
A	4000	1.50 X 3.00	CTR	CTR
B	4010	4.00 X 8.00	3.0	1.5
C	4000	1.50 X 3.00	CTR	CTR
D	4010	4.00 X 6.00	1.5	1.5
E				
F	1050	3.00 X 8.00	5.0	CTR
I	1001	1.50 X 3.00	CTR	CTR
J	1070	4.00 X 6.00	2.3	1.5
K	1010	4.00 X 8.00	3.3	1.5
M	1001	1.50 X 3.00	CTR	CTR
N	1030	3.00 X 3.00	CTR	CTR
O	1001	3.00 X 3.00	CTR	CTR
P	1010	4.00 X 8.00	4.2	1.5

NOTES:
 1. TRUSSES MANUFACTURED BY WALKER LUMBER
 2. ANALYSIS CONFORMS TO TPI (ANSI/TPI 1-1995).



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CHECKED JAN 13 2000



UniStar -- Version 40.0.110
RUN DATE: 1-13-00

CSI SIZE LUMBER 1.15FB
TOP 0.64 4X 2 DFL-#1B 2183
BTM 0.48 4X 2 DFL-#1B 2183
WBS 0.78 4X 2 HF-STAN 698
EXCEPTIONS:
K-L 2 4X 2 HF-STAN 698
I-J F-PF SAME AS K-L
REPETITIVE MEMBER STRESS USED.

LATERAL BRACING:
TOP CHORD - CONTINUOUS
BTM CHORD - CONTINUOUS
TRUSS SPACING - 24.0 IN.

LOAD CASE #1
LUMBER STRESS INCREASE: 0.0%
PLATE STRESS INCREASE: 0.0%
LOADING LIVE DEAD (PSF)
TOP CHD 40.0 10.0
BTM CHD 0.0 5.0
TOTAL 40.0 15.0 55.0
SUPPORT CRITERIA
JT REACT WIDTH JT REACT WIDTH
LBS IN-SX LBS IN-SX
B 388 3-8 K 1679 3-8
I 1780 3-8 D 742 3-8

LOAD CASE #2 UNBALANCED LOAD
LUMBER STRESS INCREASE: 0.0%
PLATE STRESS INCREASE: 0.0%
LOADING LIVE DEAD (PSF)
TOP CHD 40.0 10.0
BTM CHD 0.0 5.0
TOTAL 40.0 15.0 55.0
EXCEPTIONS:
A-L 40.0 10.0
L-J 0.0 10.0
J-C 40.0 10.0

SUPPORT CRITERIA
JT REACT WIDTH JT REACT WIDTH
LBS IN-SX LBS IN-SX
B 507 3-8 K 896 3-8
I 1223 3-8 D 764 3-8

LOAD CASE #3 UNBALANCED LOAD
LUMBER STRESS INCREASE: 0.0%
PLATE STRESS INCREASE: 0.0%
LOADING LIVE DEAD (PSF)
TOP CHD 40.0 10.0

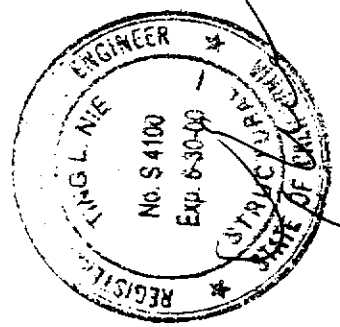
MEMBR	BTM CHD	TOP CHORDS	BTM CHD	TOP CHORDS	JT REACT WIDTH	JT REACT WIDTH
	TOTAL	EXCEPTIONS:	TOTAL	EXCEPTIONS:	LBS IN-SX	LBS IN-SX
A-L	40.0	0.0	5.0	55.0		
L-J	0.0	0.0	10.0			
J-C	40.0	0.0	10.0			
SUPPORT CRITERIA						
K-L	2	4X 2	HF-STAN	698		
I-J	F-PF	SAME AS	K-L			
B	-9	3-8	K	1237	3-8	
I	1042	3-8	D	181	3-8	

MEMBR	CSI	P (LBS)	M01ST	M02ND
A-S	0.35	0	T	0
S-U	0.36	969	C	932
U-V	0.36	969	C	74
V-W	0.37	969	C	683
W-L	0.64	1895	T	658
L-X	0.64	1895	T	514
X-Y	0.35	1112	C	957
Y-Z	0.35	1112	C	529
Z-AA	0.52	1412	C	624
AA-BB	0.53	1412	C	824
BB-S2	0.45	1506	T	174
S2-J	0.58	1506	T	1068
J-CC	0.58	1506	T	687
CC-DD	0.29	1670	C	585
DD-EE	0.29	1670	C	968
EE-FF	0.37	2164	C	651
FF-GG	0.40	2164	C	654
GG-C	0.30	0	T	396
B-R	0.25	831	T	773
R-Q	0.26	969	T	812
Q-K	0.24	949	C	0
K-P	0.18	572	C	156
P-S1	0.35	1412	T	305
S1-O	0.24	1412	T	274
O-N	0.39	1412	T	374
N-I	0.27	832	C	0
I-M	0.19	511	T	38
M-H	0.45	2164	T	38
H-F	0.48	2164	T	454
F-D	0.35	1386	T	299
B-E	0.51	C	B-E	176
E-A	102	C	B-S	283
S-R	315	C	R-U	364
Q-W	336	C	Q-W	51
W-K	1567	C	K-L	914
				121
				1000
				291

DL+LL DEFL = 0.27" AT S1
LL DEFL = 0.20" < BRG-SPAN/360
SPAN/DEFL (DL+LL) = 999
PLATING CONFORMS TO TPI-95
PLATE VALUES MAY BE VERIFIED
GRIP BASED ON DF/HF LUMBER
USING GROSS AREA TEST.
GRIP REDUCED 20% FOR
M.C. > 19%
PLATES - 20 GAUGE LOCK
GRIPPING 330-137 PSI PER PAIR
INCLUDES 0.0% INCREASE
TENSION 1339-465 PLI PER PAIR
SHEAR 784-506 PLI PER PAIR

JT TYPE	PLATE	SIZE	X	Y
A	4000	1.50 X 3.00	CTR	CTR
B	4010	3.00 X 6.00	CTR	CTR
C	4000	1.50 X 3.00	CTR	CTR
D	4010	4.00 X 8.00	3.0	1.5
E	1050	3.00 X 6.00	CTR	CTR
F	1001	1.50 X 3.00	CTR	CTR
G	1070	4.00 X12.00	6.6	1.5
H	1070	3.00 X 3.00	CTR	CTR
I	1070	4.00 X12.00	6.0	1.5
J	1070	3.00 X 3.00	CTR	CTR
K	1070	4.00 X12.00	6.0	1.5
L	1001	1.50 X 3.00	CTR	CTR
M	1070	4.00 X 8.00	2.9	1.5
N	1050	3.00 X 5.00	3.5	CTR
O	1001	1.50 X 3.00	CTR	CTR
P	1070	3.00 X 8.00	3.2	CTR
Q	1050	3.00 X 5.00	3.5	CTR
R	1030	3.00 X 3.00	CTR	CTR
S	1010	3.00 X 5.00	1.6	CTR
U	1001	1.50 X 3.00	CTR	CTR
V	1001	1.50 X 3.00	CTR	CTR
W	1010	4.00 X 8.00	4.0	CTR
X	1010	4.00 X 8.00	4.0	CTR

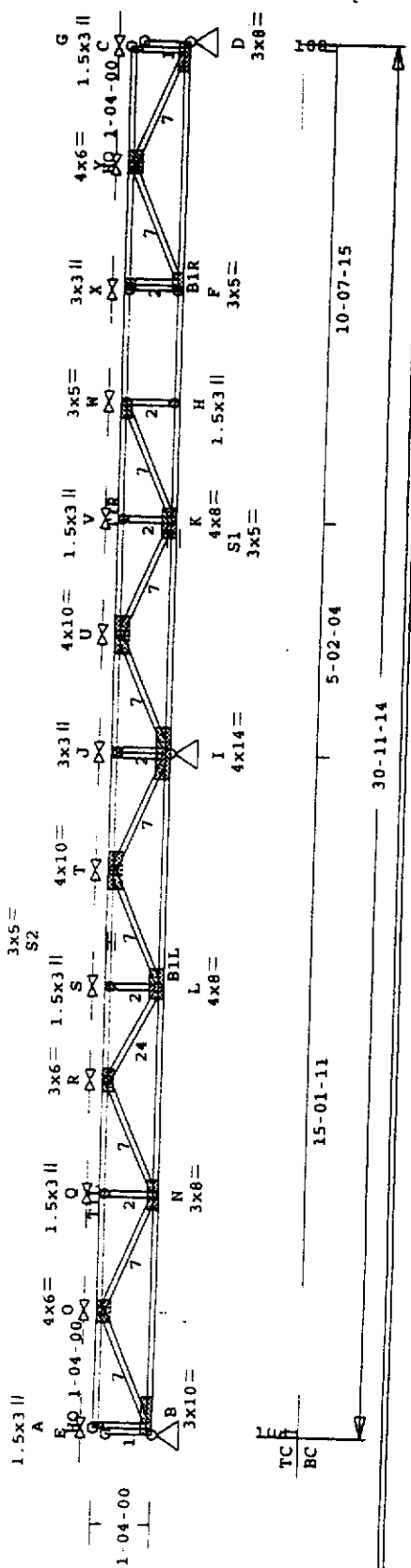
- NOTES:
1. TRUSSES MANUFACTURED BY - WALKER LUMBER
 2. ANALYSIS CONFORMS TO TPI (ANSI/TPI 1-1995).
 3. UNBALANCED LOADS CHECKED (UNBLN LD FAC = 1.00, 0.00).
 4. SHIM EACH BEARING WALL AS REQUIRED TO INSURE FULL BEARING CONTACT WITH TRUSS.



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CHECKED JAN 13 2000

COLLECTOR LOAD 5000



miStar -- Version 40.0.110
RUN DATE: 1-13-00

CSI SIZE LUMBER 1.15FB
TOP 0.65 4X 2 DFL-#1B 2183
BTM 0.99 4X 2 DFL-#1B 2183
WBS 0.84 4X 2 HF-STAN 698
EXCEPTIONS:
I-J 2 4X 2 HF-STAN 698
F-X SAME AS I-J
REPETITIVE MEMBER STRESS USED.

LATERAL BRACING:
TOP CHORD - CONTINUOUS
BTM CHORD - CONTINUOUS
TRUSS SPACING - 24.0 IN.

LOAD CASE #1
LUMBER STRESS INCREASE: 0.0%
PLATE STRESS INCREASE: 0.0%
LOADING LIVE DEAD (PSF)
TOP CHD 40.0 10.0
BTM CHD 0.0 5.0
TOTAL 40.0 15.0 55.0
SUPPORT CRITERIA
JT REACT WIDTH JT REACT WIDTH
LBS IN-SX LBS IN-SX
J 648 3-8 I 2036 3-8
J 695 3-8

LOAD CASE #2

LUMBER STRESS INCREASE: 33.0%
PLATE STRESS INCREASE: 0.0%
LOADING LIVE DEAD (PSF)
TOP CHD 0.0 10.0
BTM CHD 0.0 5.0
TOTAL 0.0 15.0 15.0

CONCENTRATED LOADS (LBS)

A	385 RIGHT	O	385 RIGHT
Q	385 RIGHT	R	385 RIGHT
S	385 RIGHT	T	385 RIGHT
J	385 RIGHT	U	385 RIGHT
V	385 RIGHT	W	385 RIGHT
X	385 RIGHT	Y	385 RIGHT
C	385 RIGHT		

SUPPORT CRITERIA

JT	TYPE	HORZ	VERT	WIDTH
B	PIN	-5004	-178	3-8
I	HORZ RLR	0	865	3-8

D HORZ RLR 0 235 3-8

LOAD CASE #3
LUMBER STRESS INCREASE: 33.0%
PLATE STRESS INCREASE: 0.0%
LOADING LIVE DEAD (PSF)
TOP CHD 0.0 10.0
BTM CHD 0.0 5.0
TOTAL 0.0 15.0 15.0

CONCENTRATED LOADS (LBS)

A	385 LEFT	O	385 LEFT
Q	385 LEFT	R	385 LEFT
S	385 LEFT	T	385 LEFT
J	385 LEFT	U	385 LEFT
V	385 LEFT	W	385 LEFT
X	385 LEFT	Y	385 LEFT
C	385 LEFT		

SUPPORT CRITERIA

JT	TYPE	HORZ	VERT	WIDTH
B	PIN	5004	0	3-8
I	HORZ RLR	0	235	3-8
D	HORZ RLR	0	149	3-8

MEMBER CSI P (LBS) MOIST %2ND

B-E	54 C	B-E	54 C
E-A	107 C	B-O	1246 C
O-N	1004 T	N-O	257 C
N-R	923 T	R-L	830 C
L-S	262 C	L-I	1374 T
T-I	2007 C	I-J	272 C
I-U	2064 C	U-K	1425 T
K-V	241 C	K-W	812 C
F-Y	50 T	F-X	215 C
G-C	611 T	Y-D	1360 C
D-G	101 C	D-G	51 C

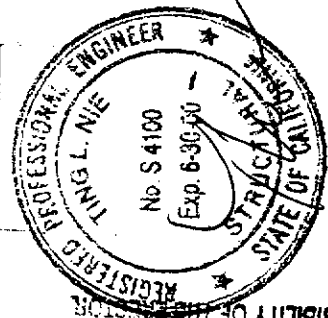
DL+LL DEFL = 0.19" IN F-D
LL DEFL = 0.12" < BRG-SPAN/360
SPAN/DEFL (DL+LL) = .999

PLATING CONFORMS TO TPI-95
GRIP VALUES MAY BE VERIFIED
BASED ON DF/HF LUMBER
USING GROSS AREA TEST.
GRIP REDUCED 20% FOR
M.C. > 19%.

PLATES - 20 GAUGE LOCK
GRIPPING 330-137 PSI PER PAIR
INCLUDES 0.0% INCREASE
TENSION 1339-465 PLI PER PAIR
SHEAR 784-506 PLI PER PAIR

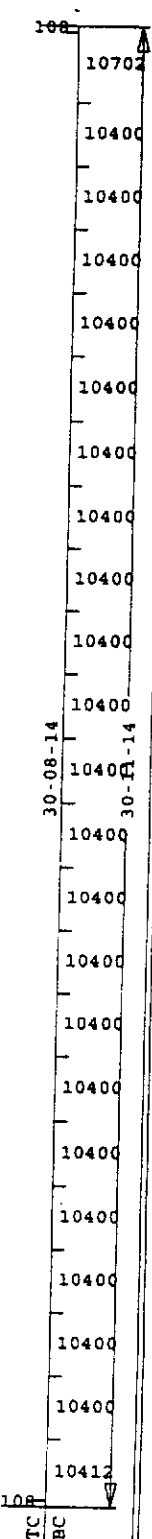
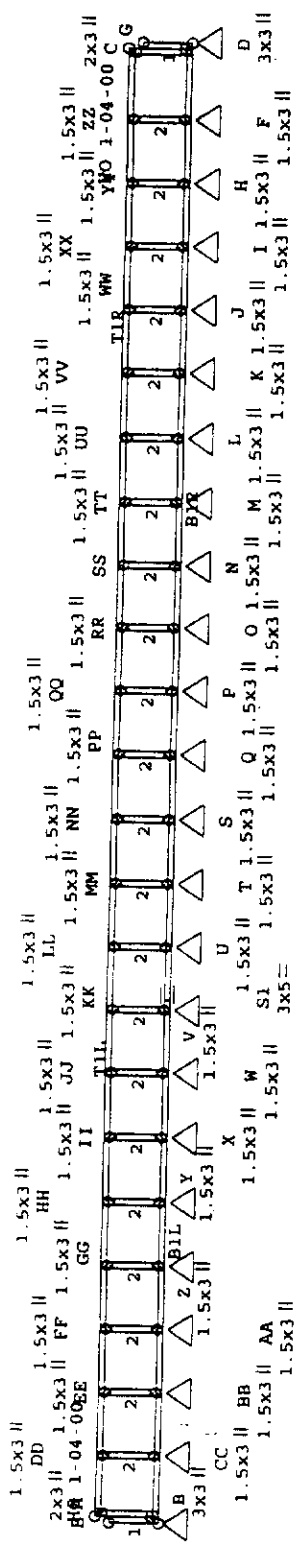
JT	TYPE	PLATE	SIZE	X	Y
A	4000	1.50 X	3.00	CTR	CTR
B	4010	3.00 X	10.00	3.0	CTR
C	4000	1.50 X	3.00	CTR	CTR
D	4010	3.00 X	8.00	3.0	CTR
E	1050	3.00 X	5.00	2.0	CTR
F	1001	1.50 X	3.00	CTR	CTR
G	1070	4.00 X	14.00	7.0	1.5
H	1001	3.00 X	3.00	CTR	CTR
I	1001	4.00 X	8.00	3.1	1.5
K	1070	4.00 X	8.00	4.9	1.5
L	1070	4.00 X	8.00	3.1	1.5
N	1070	3.00 X	8.00	CTR	CTR
O	1010	4.00 X	6.00	2.8	1.5
Q	1001	1.50 X	3.00	CTR	CTR
R	1010	3.00 X	6.00	CTR	CTR
S	1001	1.50 X	3.00	CTR	CTR
T	1010	4.00 X	10.00	5.0	1.5
U	1010	4.00 X	10.00	5.0	1.5

- NOTES:
1. TRUSSES MANUFACTURED BY WALKER LUMBER
 2. ANALYSIS CONFORMS TO TPI (ANSI/TPI 1-1995).
 3. PREVENT TRUSS ROTATION AT ALL BEARING LOCATIONS.
 4. ANCHOR TRUSS FOR A TOTAL HORIZONTAL LOAD OF 5004 LBS.



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CHECKED JAN 13 2000



Unistar -- Version 40.0.110
 RUN DATE: 1- 5-00

CSI SIZE LUMBER 1.15FB
 TOP 0.08 4X 2 DFL-#1B 2183
 BTM 0.01 4X 2 DFL-#1B 2183
 WBS 0.08 4X 2 HF-STAN 698
 LUMBER STRESS INCREASE: 0.04
 REPETITIVE MEMBER STRESS USED.

LATERAL BRACING:
 TOP CHORD - CONTINUOUS
 BTM CHORD - CONTINUOUS
 TRUSS SPACING - 24.0 IN.

LOADING LIVE DEAD (PSF)
 TOP CHD 40.0 10.0
 BTM CHD 0.0 5.0
 TOTAL 40.0 15.0 55.0

SUPPORT CRITERIA
 BOTTOM CHORD CONTINUOUSLY
 SUPPORTED ALONG ENTIRE LENGTH.

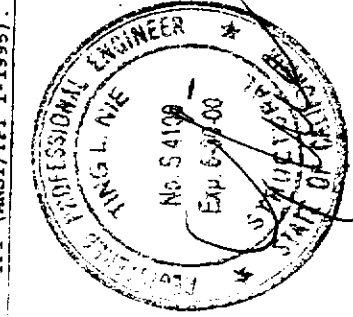
MEMBR	CSI	P (LBS)	Me1ST	Me2ND	TOP CHORDS	Me1ST	Me2ND
YY-ZZ	0.08	6 C	153	-234			
ZZ-C	0.08	6 C	234	-77			
B-CC	0.01	6 T	0	-21			
CC-BB	0.01	6 T	21	-16			
BB-AA	0.01	6 T	16	-18			
AA-Z	0.01	6 T	18	-17			
Z-Y	0.01	6 T	17	-17			
Y-X	0.01	6 T	17	-17			
X-W	0.01	6 T	17	-18			
W-V	0.01	6 T	17	-18			
V-S	0.01	6 T	18	-14			
S-U	0.01	6 T	14	0			
U-T	0.01	6 T	0	-16			
T-S	0.01	6 T	16	-19			
S-R	0.01	6 T	19	-14			
R-Q	0.01	6 T	14	0			
Q-P	0.01	6 T	0	-13			
P-O	0.01	6 T	13	-18			
O-N	0.01	6 T	18	-17			
N-M	0.01	6 T	17	-17			
M-L	0.01	6 T	17	-17			
L-K	0.01	6 T	17	-17			
K-J	0.01	6 T	17	-17			
J-I	0.01	6 T	17	-18			
I-H	0.01	6 T	17	-18			
H-F	0.01	6 T	18	-15			
F-D	0.01	6 T	15	-25			
B-E	0.07	29 C	0	-62			
E-A	0.08	57 C	62	-77			
D-G	0.07	33 C	0	62			
G-C	0.08	65 C	-62	77			
B-E	29 C	CC-DD		139 C			
BB-EE	133 C	AA-FF		134 C			
Z-GG	134 C	Y-HH		134 C			
X-II	134 C	W-JJ		134 C			
V-KK	134 C	U-LL		134 C			
T-MM	134 C	S-NN		134 C			
Q-PP	135 C	P-OQ		129 C			
RR-SS	151 C	N-SS		106 C			
M-TT	151 C	L-UU		129 C			
K-VV	135 C	J-WV		133 C			
I-XX	135 C	H-YY		128 C			
F-ZZ	154 C	D-G		33 C			

DL+LL DEFL = 0.00" IN ZZ-C
 LL DEFL = 0.00" < BRG-SPAN/360

SPAN/DEFL (DL+LL) - 999
 PLATING CONFORMS TO TPI-95
 PLATE VALUES MAY BE VERIFIED
 GRIP BASED ON DF/HF LUMBER
 USING CROSS AREA TEST.
 GRIP REDUCED 20% FOR
 M.C. > 19%
 PLATES - 20 GAUGE LOCK
 GRIPPING 330-137 PSI PER PAIR
 INCLUDES 0.04 INCREASE
 TENSION 1339-465 PLI PER PAIR
 SHEAR 784-506 PLI PER PAIR

II	III	IV	V	VI	VII	VIII	IX	X	XI	XII
II 1001	1.50 X 3.00	CTR	CTR							
JJ 1001	1.50 X 3.00	CTR	CTR							
KK 1001	1.50 X 3.00	CTR	CTR							
LL 1001	1.50 X 3.00	CTR	CTR							
MM 1001	1.50 X 3.00	CTR	CTR							
NN 1001	1.50 X 3.00	CTR	CTR							
OO 1001	1.50 X 3.00	CTR	CTR							
PP 1001	1.50 X 3.00	CTR	CTR							
QQ 1001	1.50 X 3.00	CTR	CTR							
RR 1001	1.50 X 3.00	CTR	CTR							
TT 1001	1.50 X 3.00	CTR	CTR							
UU 1001	1.50 X 3.00	CTR	CTR							
VV 1001	1.50 X 3.00	CTR	CTR							
WW 1001	1.50 X 3.00	CTR	CTR							
XX 1001	1.50 X 3.00	CTR	CTR							
YY 1001	1.50 X 3.00	CTR	CTR							
ZZ 1001	1.50 X 3.00	CTR	CTR							
S1 1200	3.00 X 5.00	CTR	CTR							

NOTES:
 1. TRUSSES MANUFACTURED BY -
 WALKER LUMBER
 2. ANALYSIS CONFORMS TO
 TPI (ANSI/TPI 1-1995).



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 DESIGN. IT IS A VERTICAL PLANE, TO BE INCORPORATED INTO THE
 OVERALL DESIGN AS SPECIFIED BY THE BUILDING DESIGNER. LATERAL
 BRACING SHOWN IS FOR INDIVIDUAL MEMBERS ONLY. ADDITIONAL
 PERMANENT BRACING OF THE OVERALL STRUCTURE IS THE RESPONSIBILITY
 OF THE BUILDING DESIGNER. TEMPORARY BRACING TO INSURE STABILITY
 DURING CONSTRUCTION IS THE RESPONSIBILITY OF THE CONTRACTOR.

CHECKED JAN 13 2000

QUAN TYPE SPAN P1-H1
2 M100 110306 1600

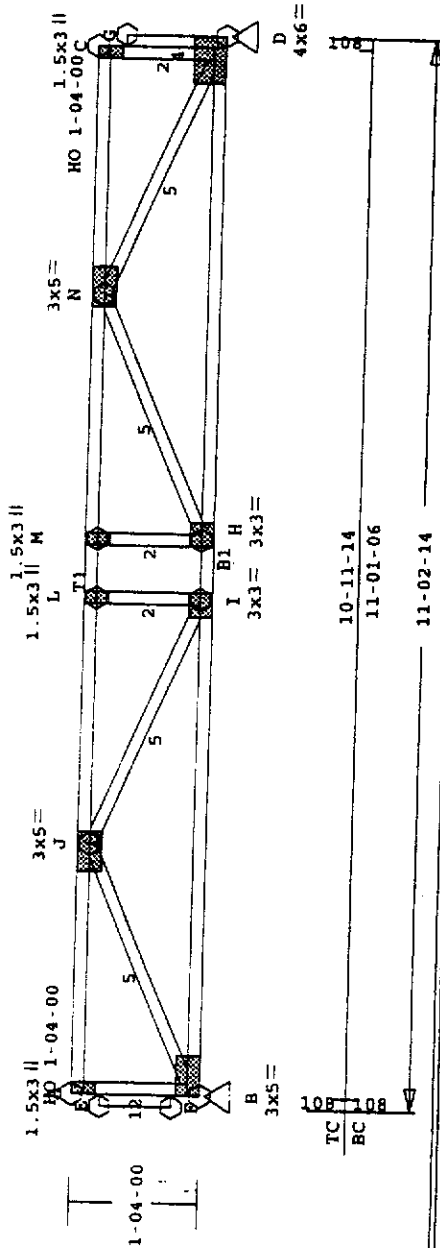
OVERHANGS
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JOB

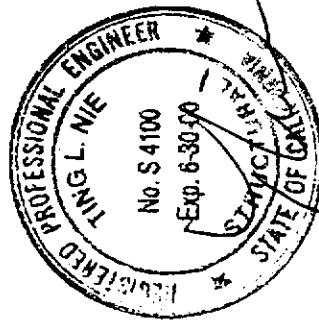
GATEWAY PLAN 4 H4712

MARK

F7



NOTES:
1. TRUSSES MANUFACTURED BY - WALKER LUMBER
2. ANALYSIS CONFORMS TO TPI (ANSI/TPI 1-1995).



MEMBR	CSI	P (LBS)	MOIST	MO2ND
J-I	412	T	I-L	143 C
H-M	143	C	H-N	412 T
N-D	1146	C	G-C	104 C
D-G	52	C	D-G	52 C

DL+LL DEFL = 0.09" IN H-D
LL DEFL = 0.05" < BRG-SPAN/360
SPAN/DEFL (DL+LL) = 999

PLATING CONFORMS TO TPI-95
PLATE VALUES MAY BE VERIED
GRIP BASED ON DF/HF LUMBER
USING GROSS AREA TEST.
GRIP REDUCED 20% FOR
M.C. > 19%
PLATES - 20 GAUGE LOCK
GRIPPING 330-137 PSI PER PAIR
INCLUDES 0.0% INCREASE
TENSION 1339- 465 PLI PER PAIR
SHEAR 784- 506 PLI PER PAIR

MEMBR	CSI	P (LBS)	MOIST	MO2ND
A	4000	1.50 X 3.00	CTR	CTR
B	4010	3.00 X 5.00	1.5	CTR
C	4000	1.50 X 3.00	CTR	CTR
D	4010	4.00 X 6.00	3.0	1.5
H	1050	3.00 X 3.00	CTR	CTR
I	1030	3.00 X 3.00	CTR	CTR
J	1010	3.00 X 5.00	1.5	CTR
L	1001	1.50 X 3.00	CTR	CTR
M	1001	1.50 X 3.00	CTR	CTR
N	1010	3.00 X 5.00	3.5	CTR

LATERAL BRACING:
TOP CHORD - CONTINUOUS
BTM CHORD - CONTINUOUS
TRUSS SPACING - 24.0 IN.

LOADING	LIVE	DEAD (PSE)
TOP CHD	40.0	10.0
BTM CHD	0.0	5.0
TOTAL	40.0	15.0

UPPORT CRITERIA	JT REACT WIDTH	JT REACT WIDTH
LBS	IN-SX	LBS
B	604	3-8
D	604	3-8

MEMBR	CSI	P (LBS)	MOIST	MO2ND
A-J	0.30	0	T	0
J-L	0.35	1413	C	858 -345
L-M	0.15	1413	C	345 -345
M-N	0.35	1413	C	345 -858
N-C	0.30	0	T	858 0

TOP CHORDS

MEMBR	CSI	P (LBS)	MOIST	MO2ND
B-I	0.27	1041	T	0 -273
I-H	0.33	1413	T	273 -273
H-D	0.27	1041	T	273 0

WEBS

B-A - 104 C B-J - 1146 C

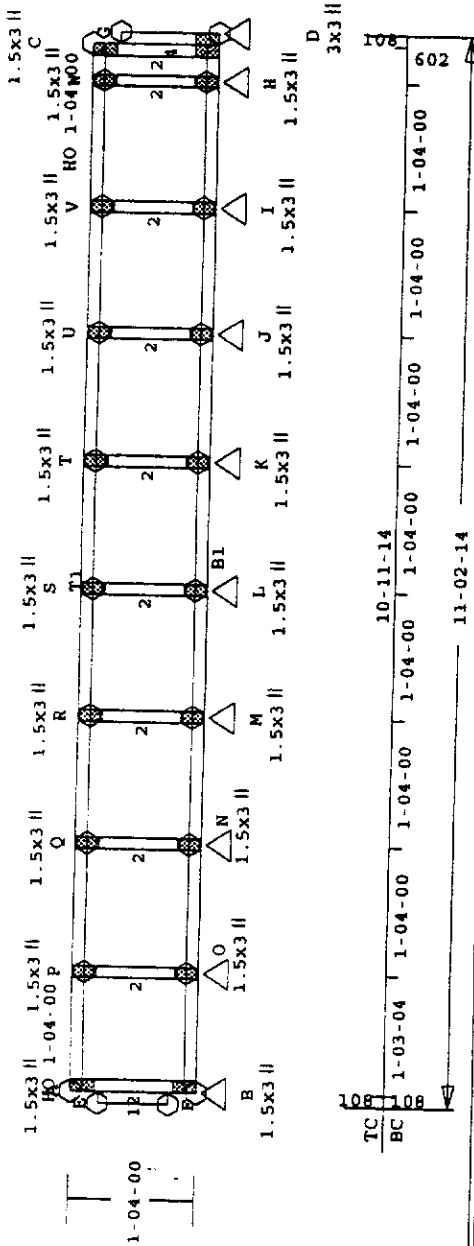
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CHECKED JAN 13 2000

QUAN TYPE SPAN P1-H1 OVERHANGS
 1 M100 110306 1600 0 0

JOB
 GATEWAY PLAN 4 H4712

MARK
 F7G



Unistar -- Version 40.0.110
 RUN DATE: 1- 5-00

CSI SIZE LUMBER 1.15FB
 TOP 0.07 4X 2 DFL-#1B 2183
 BTM 0.01 4X 2 DFL-#1B 2183
 WBS 0.04 4X 2 HF-STAN 698
 LUMBER STRESS INCREASE: 0.0%
 REPETITIVE MEMBER STRESS USED.

LATERAL BRACING:

TOP CHORD - CONTINUOUS
 BTM CHORD - CONTINUOUS
 TRUSS SPACING - 24.0 IN.

LOADING LIVE DEAD (PSF)
 TOP CHD 40.0 10.0
 BTM CHD 0.0 5.0
 TOTAL 40.0 15.0 55.0

SUPPORT CRITERIA

BOTTOM CHORD CONTINUOUSLY
 SUPPORTED ALONG ENTIRE LENGTH.

MEMBER	CSI	P (LBS)	MØ1ST	MØ2ND
A-P	0.07	3 C	36	-199
P-Q	0.07	3 C	199	-172
Q-R	0.06	3 C	172	-179
R-S	0.06	3 C	179	-177
S-T	0.06	3 C	177	-178
T-U	0.06	3 C	178	-174
U-V	0.07	3 C	174	-191
V-W	0.07	3 C	191	-122
W-C	0.04	3 C	122	-36

TOP CHORDS
 BOTTOM CHORDS

PLATING CONFORMS TO TPI-95
 PLATE VALUES MAY BE VERIFIED
 GRIP BASED ON DF/HF LUMBER
 USING GROSS AREA TEST.
 GRIP REDUCED 20% FOR
 M.C. > 19%.

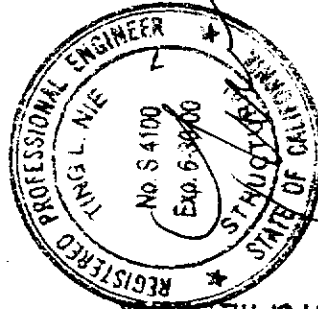
PLATES - 20 GAUGE LOCK
 GRIPPING 330-137 PSI PER PAIR
 INCLUDES 0.0% INCREASE
 TENSION 1339- 465 PLI PER PAIR
 SHEAR 784- 506 PLI PER PAIR

JT TYPE	PLATE	SIZE	X	Y
A	4000	1.50 X 3.00	CTR	CTR
B	4000	1.50 X 3.00	CTR	CTR
C	4000	1.50 X 3.00	CTR	CTR
D	4000	3.00 X 3.00	CTR	CTR
G	1001	1.50 X 3.00	CTR	CTR
H	1001	1.50 X 3.00	CTR	CTR
I	1001	1.50 X 3.00	CTR	CTR
J	1001	1.50 X 3.00	CTR	CTR
K	1001	1.50 X 3.00	CTR	CTR
L	1001	1.50 X 3.00	CTR	CTR
M	1001	1.50 X 3.00	CTR	CTR
N	1001	1.50 X 3.00	CTR	CTR
O	1001	1.50 X 3.00	CTR	CTR
P	1001	1.50 X 3.00	CTR	CTR
Q	1001	1.50 X 3.00	CTR	CTR
R	1001	1.50 X 3.00	CTR	CTR
S	1001	1.50 X 3.00	CTR	CTR
T	1001	1.50 X 3.00	CTR	CTR
U	1001	1.50 X 3.00	CTR	CTR
V	1001	1.50 X 3.00	CTR	CTR
W	1001	1.50 X 3.00	CTR	CTR

MEMBER	CSI	P (LBS)	MØ1ST	MØ2ND
B-O	0.01	3 T	0	-21
O-N	0.01	3 T	21	-16
N-M	0.01	3 T	16	-18
M-L	0.01	3 T	18	-17
L-K	0.01	3 T	17	-17
K-J	0.01	3 T	17	-17
J-I	0.01	3 T	17	-18
I-H	0.01	3 T	18	-13
H-D	0.00	3 T	13	0
WEBS				
B-A	0.04	53 C	0	-36
G-C	0.04	0 T	-29	36
D-G	0.03	0 T	0	29
O-P	143 C	N-Q	-	132 C
M-R	134 C	L-S	-	134 C
K-T	134 C	J-U	-	132 C
I-V	139 C	H-W	-	101 C
D-G	0	0 T	-	-

DL+LL DEFL = 0.00" IN A-P
 LL DEFL = 0.00" < BRG-SPAN/360
 SPAN/DEFL (DL+LL) = 999

- NOTES:
- TRUSSES MANUFACTURED BY - WALKER LUMBER
 - ANALYSIS CONFORMS TO TPI (ANSI/TPI 1-1995).



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CHECKED JAN 13 2000

QUAN TYPE SPAN P1-H1
3 M100 131114 1600

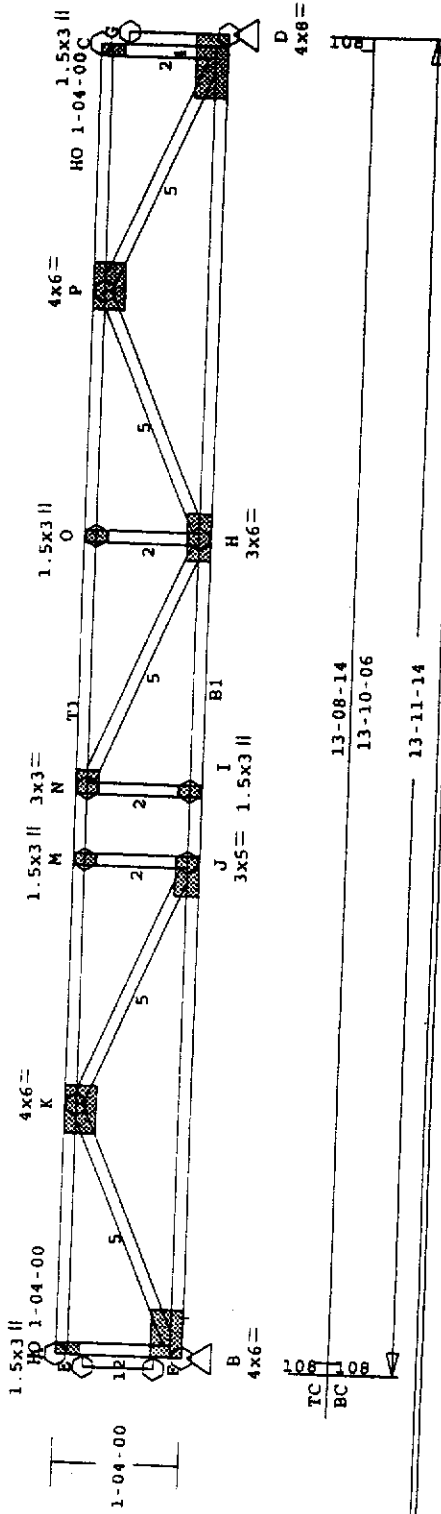
OVERHANGS
0 0

JOB

GATEWAY PLAN 4 H4712

MARK

F8



UniStar -- Version 40.0.110
RUN DATE: 1-13-00

CSI SIZE LUMBER 1.15FB
TOP 0.37 4X 2 DFL-#1B 2183
BTM 0.49 4X 2 DFL-#1B 2183
WBS 0.47 4X 2 HF-STAN 698
LUMBER STRESS INCREASE: 0.0%
REPETITIVE MEMBER STRESS USED.

LATERAL BRACING:

TOP CHORD - CONTINUOUS
BTM CHORD - CONTINUOUS
TRUSS SPACING - 24.0 IN.

LOADING LIVE DEAD (PSF)
TOP CHD 40.0 10.0
BTM CHD 0.0 5.0
TOTAL 40.0 15.0 55.0

SUPPORT CRITERIA

REACT WIDTH JT REACT WIDTH
LBS IN-SX LBS IN-SX
B 755 3-8 D 755 3-8

MEMBER CSI P(LBS) M01ST M02ND

TOP CHORDS

A-K	0.27	0 T	0	-784
K-M	0.37	2090 C	784	-598
M-N	0.26	2090 C	598	-158
N-O	0.32	2085 C	158	-667
O-P	0.36	2085 C	667	-768
P-C	0.27	0 T	768	0

BOTTOM CHORDS

B-J	0.37	1370 T	0	-425
J-I	0.49	2090 T	425	415
I-H	0.49	2090 T	-415	-273
H-D	0.32	1363 T	273	0

MEMBER CSI P(LBS) M01ST M02ND

WEBS
B-A = 107 C B-K = 1508 C
K-J = 797 T J-M = 209 C
I-N = 100 C N-H = 7 C
H-O = 273 C H-P = 798 T
P-D = 1501 C G-C = 107 C
D-G = 54 C D-G = 54 C

DL+LL DEFL = 0.18" IN N-O
LL DEFL = 0.14" < BRG-SPAN/360
SPAN/DEFL (DL+LL) = 923

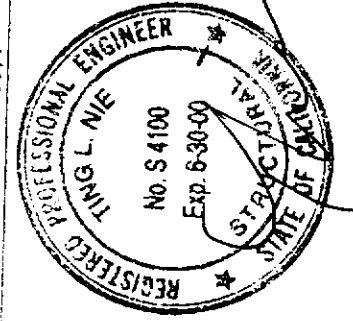
PLATING CONFORMS TO TPI-95
PLATE VALUES MAY BE VERIED
GRIP BASED ON DF/HF LUMBER
USING GROSS AREA TEST.
GRIP REDUCED 20% FOR
M.C. > 19%.

PLATES - 20 GAUGE LOCK
GRIPPING 330-137 PSI PER PAIR
INCLUDES 0.0% INCREASE
TENSION 1339-465 PLI PER PAIR
SHEAR 784-506 PLI PER PAIR

JT TYPE PLATE SIZE X Y

A	4000	1.50 X 3.00	CTR	CTR
B	4010	4.00 X 6.00	1.5	1.5
C	4000	1.50 X 3.00	CTR	CTR
D	4010	4.00 X 8.00	3.0	1.5
G				
H	1070	3.00 X 6.00	3.3	CTR
I	1001	1.50 X 3.00	CTR	CTR
J	1030	3.00 X 5.00	1.5	CTR
K	1010	4.00 X 6.00	2.3	1.5
M	1001	1.50 X 3.00	CTR	CTR
N	1050	3.00 X 3.00	CTR	CTR
O	1001	1.50 X 3.00	CTR	CTR
P	1010	4.00 X 6.00	3.7	1.5

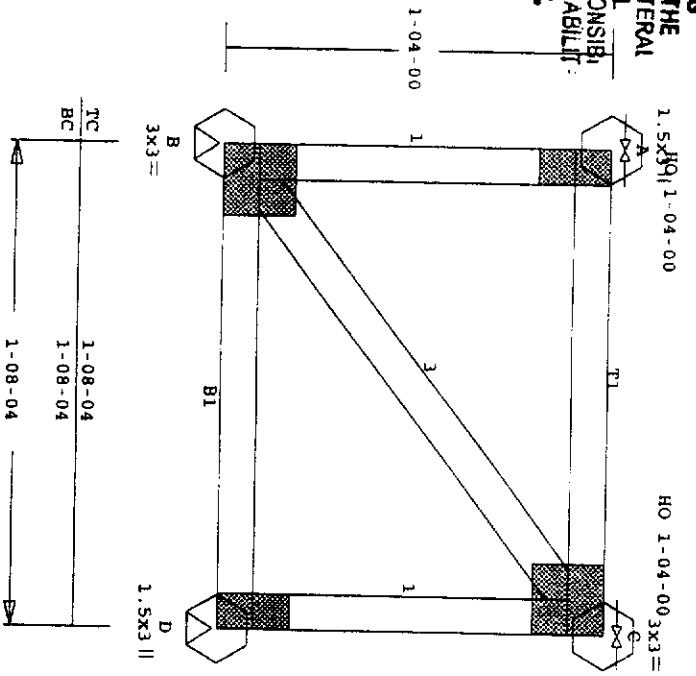
- NOTES:
1. TRUSSES MANUFACTURED BY -
WALKER LUMBER
2. ANALYSIS CONFORMS TO
TPI (ANSI/TPI 1-1995).



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CHECKED JAN 13 2000

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COLLECTOR LOAD 175 #.

UniStar -- Version 40.0.110
 RUN DATE: 11-24-99

CSI SIZE LUMBER 1.15FB
 TOP 0.15 4X 2 DFL-#1B 2183
 BTM 0.01 4X 2 DFL-#1B 2183
 WBS 0.01 4X 2 HP-STAN 698
 REPELTIVE MEMBER STRESS USED.

LATERAL BRACING:
 TOP CHORD - CONTINUOUS
 BTM CHORD - CONTINUOUS
 TRUSS SPACING - 24.0 IN.

LOAD CASE #1
 LUMBER STRESS INCREASE: 0.0%
 PLATE STRESS INCREASE: 0.0%
 LOADING LIVE DEAD (PSF)
 TOP CHD 40.0 10.0
 BTM CHD 0.0 5.0
 TOTAL 40.0 15.0 55.0
 SUPPORT CRITERIA
 JT REACT WIDTH JT REACT WIDTH
 LBS IN-SX LBS IN-SX
 B 92 3-8 D 92 3-8

LOAD CASE #2
 LUMBER STRESS INCREASE: 33.0%
 PLATE STRESS INCREASE: 0.0%
 LOADING LIVE DEAD (PSF)
 TOP CHD 0.0 10.0
 BTM CHD 0.0 5.0
 TOTAL 0.0 15.0 15.0
 CONCENTRATED LOADS (LBS)
 A 150 RIGHT A 150 LEFT
 SUPPORT CRITERIA
 JT REACT WIDTH JT REACT WIDTH
 LBS IN-SX LBS IN-SX
 B 25 3-8 D 25 3-8

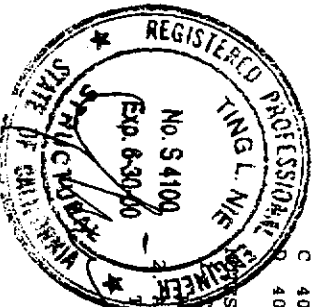
LOAD CASE #3
 LUMBER STRESS INCREASE: 33.0%
 PLATE STRESS INCREASE: 0.0%
 LOADING LIVE DEAD (PSF)
 TOP CHD 0.0 10.0
 BTM CHD 0.0 5.0
 TOTAL 0.0 15.0 15.0
 CONCENTRATED LOADS (LBS)
 C 150 RIGHT C 150 LEFT
 SUPPORT CRITERIA
 JT REACT WIDTH JT REACT WIDTH
 LBS IN-SX LBS IN-SX
 B 25 3-8 D 25 3-8

MEMBR	CSI	P(LBS)	MO1ST	MO2ND
A-C	0.15	0 T	0	0
B-D	0.01	0 T	0	0
B-A	=	85 C	B-C	=
D-C	=	85 C		0 T

DL+LL DEFL = 0.01" IN A-C
 LL DEFL = 0.01" < BRG-SPAN/360
 SPAN/DEFL (DL+LL) = 999

PLATING CONFORMS TO TPI-95
 PLATE VALUES MAY BE VERIED
 GRIP BASED ON DF/HP LUMBER
 USING GROSS AREA TEST.
 GRIP REDUCED 20% FOR
 M.C. > 19%
 PLATES - 20 GAUGE LOCK
 GRIPPING 330-137 PSI PER PAIR
 INCLUDES 0.0% INCREASE
 TENSION 1339- 465 PLI PER PAIR
 SHEAR 784- 506 PLI PER PAIR

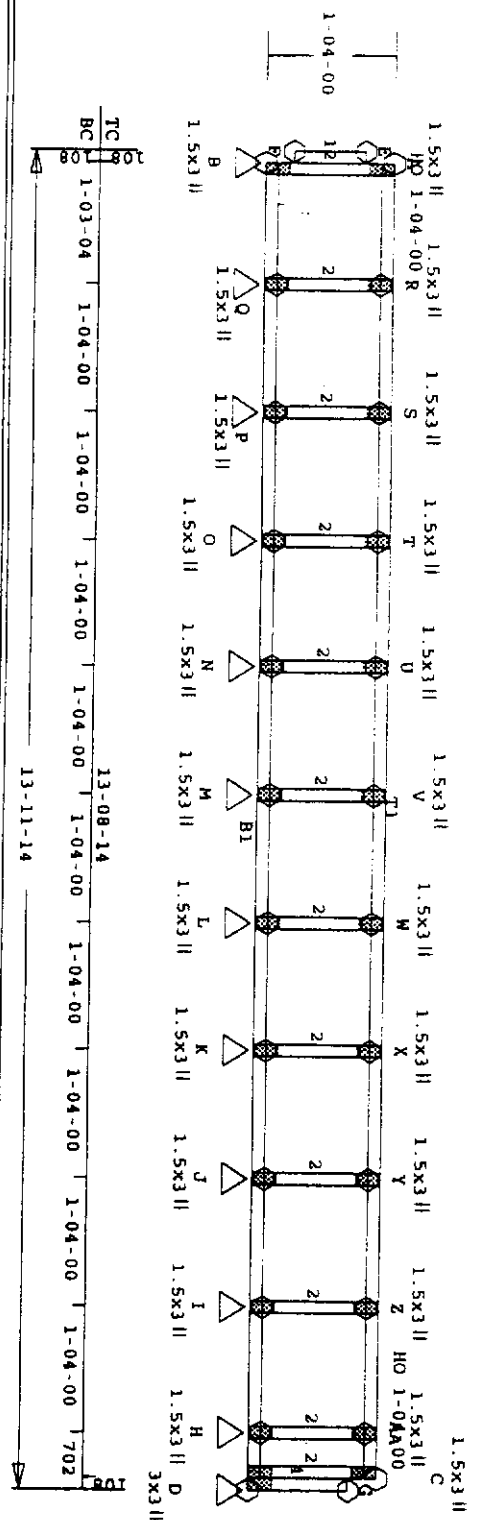
JT TYPE PLATE SIZE X Y
 A 4000 1.50 X 3.00 CTR CTR
 B 4010 3.00 X 3.00 CTR CTR
 C 4010 3.00 X 3.00 CTR CTR
 D 4000 1.50 X 3.00 CTR CTR



RUSSUS MANUFACTURED BY
 WALKER LUMBER
 ANALYSIS CONFORMS TO
 TPI (ANSI/TPI 1-1995).

CHECKED JAN 13 2000

QUAN TYPE	18 M100	SPAN	10804	1600	OVERHANGS	0	0	GATEWAY PLAN 4	H4712	SP1	MARK
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UnlStar -- Version 40.0.110
 RUN DATE: 1-5-00

CSI SIZE LUMBER 1.15FB
 TOP 0.07 4X 2 DFL-#1B 2183
 BTM 0.01 4X 2 DFL-#1B 2183
 WBS 0.04 4X 2 HF-STAN 698
 LUMBER STRESS INCREASE: 0.04
 REPETITIVE MEMBER STRESS USED.

LATERAL BRACING:
 TOP CHORD - CONTINUOUS
 BTM CHORD - CONTINUOUS
 TRUSS SPACING - 24.0 IN.

LOADING LIVE DEAD (RSF)
 TOP CHD 40.0 10.0
 BTM CHD 0.0 5.0
 TOTAL 40.0 15.0 55.0

SUPPORT CRITERIA
 BOTTOM CHORD CONTINUOUSLY
 SUPPORTED ALONG ENTIRE LENGTH.

MEMBR CSI P(LBS) MØ1ST MØ2ND
 TOP CHORDS

A-R	0.07	3 C	35	-199
R-S	0.07	3 C	199	-172
S-T	0.06	3 C	172	-179
T-U	0.06	3 C	179	-177
U-V	0.06	3 C	177	-177
V-W	0.06	3 C	177	-177
W-X	0.06	3 C	177	-178
X-Y	0.06	3 C	178	-174
Y-Z	0.07	3 C	174	-191
Z-AA	0.07	3 C	191	-122

MEMBR CSI P(LBS) MØ1ST MØ2ND
 AA-C 0.04 3 C 122 -35
 BOTTOM CHORDS

B-Q	0.01	3 T	0	-21
Q-P	0.01	3 T	21	-16
P-O	0.01	3 T	16	-18
O-N	0.01	3 T	18	-17
N-M	0.01	3 T	17	-17
M-L	0.01	3 T	17	-17
L-K	0.01	3 T	17	-17
K-J	0.01	3 T	17	-17
J-I	0.01	3 T	17	-18
I-H	0.01	3 T	18	-13
H-D	0.00	3 T	13	0

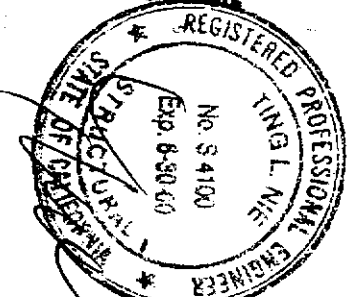
PLATING CONFORMS TO TPI-95
 PLATE VALUES MAY BE VERIFIED
 GRIP BASED ON DF/HF LUMBER
 USING GROSS AREA TEST.
 GRIP REDUCED 20% FOR
 M.C. > 19%
 PLATES - 20 GAUGE LOCK
 GRIPPING 330-137 PSI PER PAIR
 INCLUDES 0.04 INCREASE
 TENSION 1339-465 PLI PER PAIR
 SHEAR 784-506 PLI PER PAIR

DL+LL DEFL. = 0.00" IN A-R
LL DEFL. = 0.00" < BRG-SPAN/360
SPAN/DEFL (DL+LL) = 999

DL+LL DEFL. = 0.00" IN A-R
LL DEFL. = 0.00" < BRG-SPAN/360
SPAN/DEFL (DL+LL) = 999

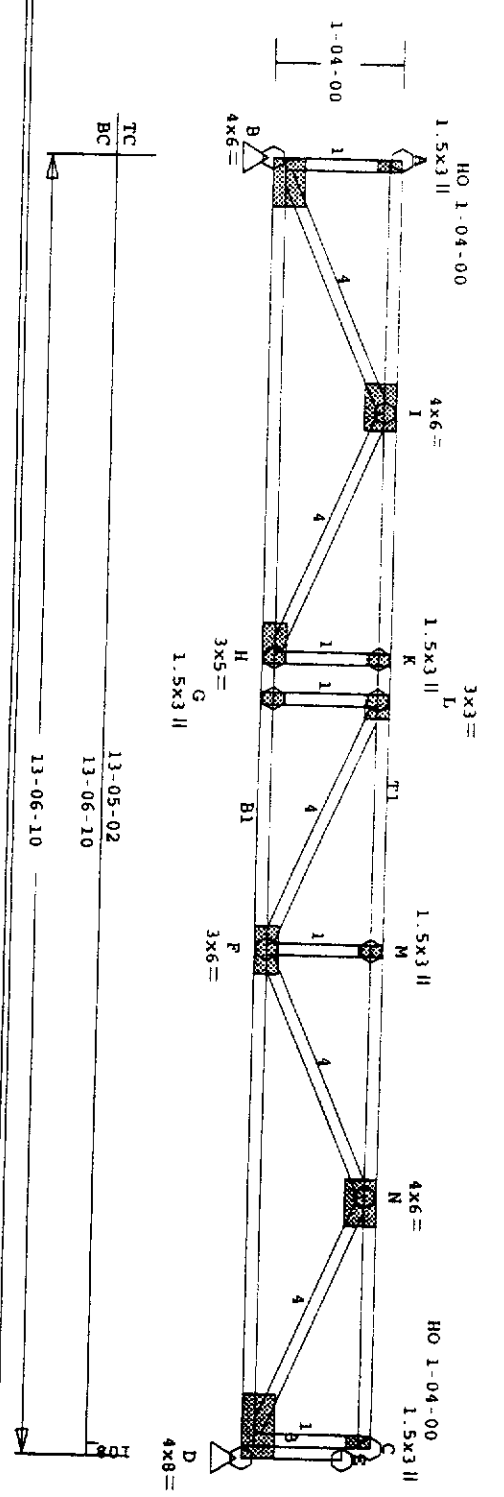
CHECKED JAN 13 2000

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NOTES:
 1. TRUSSES MANUFACTURED BY - WALKER LUMBER
 2. ANALYSIS CONFORMS TO TPI (ANSI/TPI 1-1995).

QUAN TYPE SPAN P1-H1	1 M100 13114	1600	0	0	0
OVERHANGS	0	0	0	0	0
JOB	GATEWAY PLAN 4	H4712	P8G	MARK	



Unistar -- Version 40.0.110
 RUN DATE: 1-13-00

CSI SIZE LUMBER 1.15FB
 TOP 0.37 4X 2 DFL-#1B 2183
 BTM 0.45 4X 2 DFL-#1B 2183
 MBS 0.44 4X 2 HR-STAN 698
 LUMBER STRESS INCREASE: 0.04
 REPETITIVE MEMBER STRESS USED.

LATERAL BRACING:
 TOP CHORD - CONTINUOUS
 BTM CHORD - CONTINUOUS
 TRUSS SPACING - 24.0 IN.

LOADING LIVE DEAD (PSF)
 TOP CHD 40.0 10.0
 BTM CHD 0.0 5.0
 TOTAL 40.0 15.0 55.0

SUPPORT CRITERIA
 JT REACT WIDTH JT REACT WIDTH
 LBS IN-SX LBS IN-SX
 B 738 1-12 D 738 3-8

MEMBR CSI P(LBS) M01ST M02ND
 TOP CHORDS

A-I	0.28	0 T	0	-811
I-K	0.37	2005 C	811	-493
K-L	0.22	2005 C	493	-377
L-M	0.30	2006 C	377	-613
M-N	0.36	2006 C	613	-783
N-C	0.27	0 T	783	0
BOTTOM CHORDS				
B-H	0.34	1332 T	0	-359
H-G	0.45	2005 T	359	264
G-F	0.42	2005 T	-264	-251

MEMBR CSI P(LBS) M01ST M02ND
 F-D 0.32 1326 T 251 0

WEBS
 B-A - 106 C B-I - 1466 C
 I-H - 745 T H-K - 162 C
 G-L - 124 C L-F - 0 T
 F-M - 262 C F-N - 752 T
 N-D - 1460 C D-R - 54 C
 E-C - 107 C D-E - 54 C

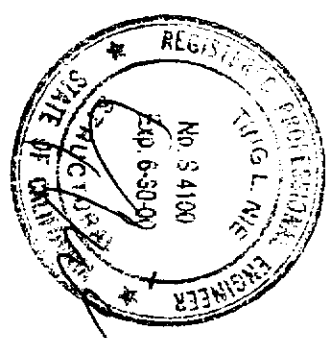
DL+LL DEFL = 0.16" IN L-M
 LL DEFL = 0.12" < BRG-SPAN/360
 SPAN/DEFL (DL+LL) = 999

13-05-02
 13-06-10
 13-06-10

PLATING CONFORMS TO TPI-95
 PLATE VALUES MAY BE VERIFIED
 GRIP BASED ON DF/HF LUMBER
 USING GROSS AREA TEST.
 GRIP REDUCED 20% FOR
 M.C. > 19%
 PLATES - 20 GAUGE LOCK
 GRIPPING 330-137 PSI PER PAIR
 INCLUDES 0.04 INCREASE
 TENSION 1339-465 PLI PER PAIR
 SHEAR 784-506 PLI PER PAIR

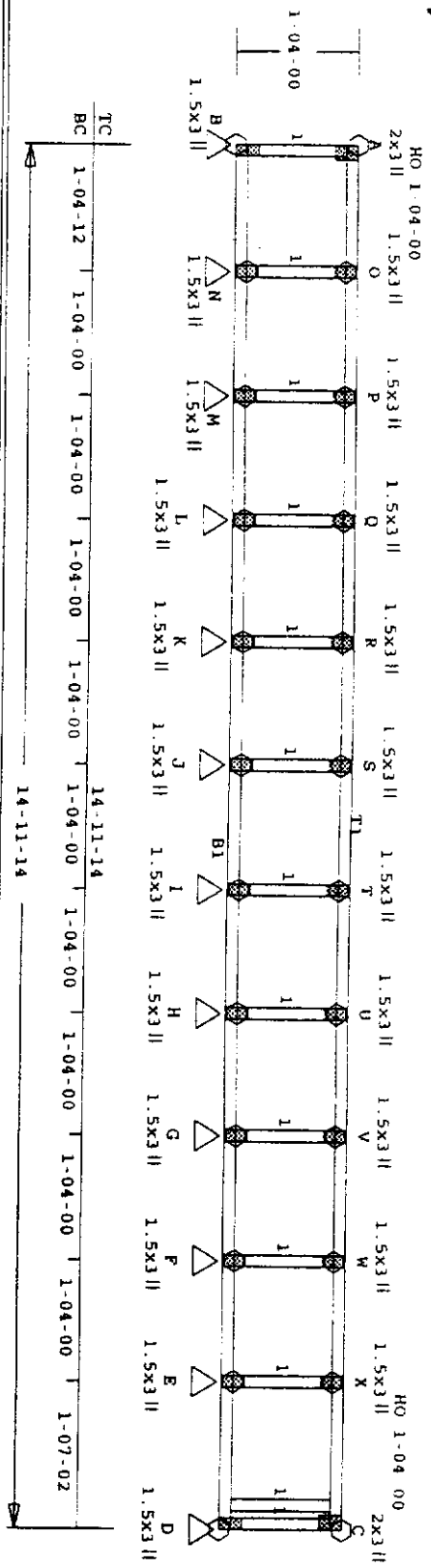
JT TYPE	PLATE SIZE	X	Y
A	4000	1.50 X 3.00	CTR
B	4010	4.00 X 6.00	1.5 1.5
C	4000	1.50 X 3.00	CTR
D	4010	4.00 X 8.00	3.0 1.5
E	1070	3.00 X 6.00	3.1 CTR
F	1001	1.50 X 3.00	CTR
G	1001	1.50 X 3.00	CTR
H	1030	3.00 X 5.00	1.5 CTR
I	1010	4.00 X 6.00	2.3 1.5
J	1001	1.50 X 3.00	CTR
K	1001	1.50 X 3.00	CTR
L	1050	3.00 X 3.00	CTR
M	1001	1.50 X 3.00	CTR
N	1010	4.00 X 6.00	3.7 1.5

NOTES:
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 2. ANALYSIS CONFORMS TO TPI (ANSI/TPI 1-1995).



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CHECKED JAN 13 2000



Unistar -- Version 40.0.110
 RUN DATE: 1-11-00

CSI SIZE LUMBER 1.15FB
 TOP 0.28 4X 2 HF-STAN 698
 BTM 0.04 4X 2 HF-STAN 698
 WBS 0.13 4X 2 HF-STAN 698
 LUMBER STRESS INCREASE: 0.04
 REPETITIVE MEMBER STRESS USED.

LATERAL BRACING:
 TOP CHORD - CONTINUOUS
 BTM CHORD - CONTINUOUS
 TRUSS SPACING - 24.0 IN.

LOADING LIVE DEAD (PSF)
 TOP CHD 40.0 10.0
 BTM CHD 0.0 5.0
 TOTAL 40.0 15.0 55.0

UPPORT CRITERIA
 BOTTOM CHORD CONTINUOUSLY
 SUPPORTED ALONG ENTIRE LENGTH.

MEMBR	CSI	P(LBS)	M@1ST	M@2ND
A-O	0.22	9 C	122	-205
O-P	0.22	9 C	205	-170
P-Q	0.20	9 C	170	-179
Q-R	0.20	9 C	179	-177
R-S	0.19	9 C	177	-177
S-T	0.19	9 C	177	-178
T-U	0.19	9 C	178	-176
U-V	0.20	9 C	176	-183
V-W	0.20	9 C	183	-157
W-X	0.28	9 C	157	-258
X-C	0.28	9 C	258	-122

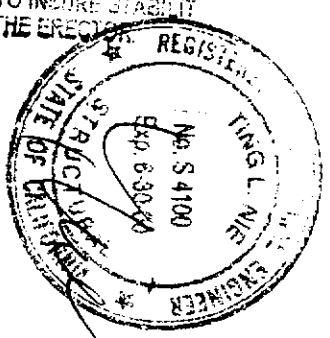
MEMBR	CSI	P(LBS)	M@1ST	M@2ND
B-N	0.03	9 T	0	-24
N-M	0.03	9 T	24	-16
M-L	0.02	9 T	16	-18
L-K	0.02	9 T	18	-17
K-J	0.02	9 T	17	-17
J-I	0.02	9 T	17	-17
I-H	0.02	9 T	17	-17
H-G	0.02	9 T	17	-18
G-F	0.02	9 T	18	-14
F-E	0.04	9 T	14	-29
E-D	0.04	9 T	29	0

PLATING CONFORMS TO TPI-95
 PLATE VALUES MAY BE VERIED
 GRIP BASED ON DF/HF LUMBER
 USING GROSS AREA TEST.
 GRIP REDUCED 20% FOR
 M.C. > 19%
 PLATES - 20 GAUGE LOCK
 GRIPPING 298-147 PSI PER PAIR
 INCLUDES 0.04 INCREASE
 TENSION 1339- 465 PLI PER PAIR
 SHEAR 784- 506 PLI PER PAIR

DL+LL DEFL.	LL DEFL.	SPAN/DEFL.
DL+LL DEFL. = 0.01" IN X-C	LL DEFL. = 0.00" < BRG-SPAN/360	SPAN/DEFL. (DL+LL) = 999

JT TYPE	PLATE SIZE	X	Y
A	4000 X 3.00	1.5	0.7
B	4000 X 3.00	CTR	CTR
C	4000 X 3.00	1.5	0.7
D	4000 X 3.00	CTR	CTR
E	1001 X 3.00	CTR	CTR
F	1001 X 3.00	CTR	CTR
G	1001 X 3.00	CTR	CTR
H	1001 X 3.00	CTR	CTR
I	1001 X 3.00	CTR	CTR
J	1001 X 3.00	CTR	CTR
K	1001 X 3.00	CTR	CTR
L	1001 X 3.00	CTR	CTR
M	1001 X 3.00	CTR	CTR
N	1001 X 3.00	CTR	CTR
O	1001 X 3.00	CTR	CTR
P	1001 X 3.00	CTR	CTR
Q	1001 X 3.00	CTR	CTR
R	1001 X 3.00	CTR	CTR
S	1001 X 3.00	CTR	CTR
T	1001 X 3.00	CTR	CTR
U	1001 X 3.00	CTR	CTR
V	1001 X 3.00	CTR	CTR
W	1001 X 3.00	CTR	CTR
X	1001 X 3.00	CTR	CTR

- NOTES:
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MARK	FL0G	JOB	OVERHANGS	0	0	0	GATEWAY PLAN 4	H4712	MARK
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CHECKED JAN 13 2000

RESIDENTIAL SUBDIVISION BUILDING PERMIT APPLICATION

Project Address: 21 Blue Fern Court
Lot Number: 43

Assessor Parcel # 274-0570-023
Subdivision Natomas Village 2

OWNER INFORMATION:

Legal Property Owner: KB Home Phone# (707)469-2464
Owner Address: 611 Orange Drive City Vacaville State CA Zip 95687

CONTRACTOR INFORMATION:

Contractor: KB Home Lic. # 761970 Phone # (707)469-2464 Fax (707)469-2405

PROJECT INFORMATION:

Land Use Zone RIA Occupancy Group R3 Construction Type VN Fed Code 1A
No. of Stories: 2 No. of Rooms: _____ Street Width: _____
1st Floor Area 1054 2nd Floor Area 1264 Basement _____ Roof Material _____
AREA IN SQUARE FOOT OF:
Dwelling/Living 2318
Garage/Storage 380
Decks/Balconies 19
Carports _____

SCOPE OF WORK: New single B Family Dwelling

FOR OFFICE USE ONLY

- Information Above Complete
- Violation Files Checked
- Standard Setbacks
- County Sewer
- AR Flood Waiver Required
- Flood Elevation Certificate Required
- Water Development Infill Area
- Planning Approval
- Design Review Approval
- Special Fee Districts Apply:

~THE FOLLOWING MUST BE PROVIDED IN ORDER TO SUBMIT FOR PERMIT~

- 2 COMPLETE PLOT PLANS, LEGIBLE & DRAWN TO SCALE
- 11 X 17 COPY OF FLOOR PLAN WITH FOLLOWING INFORMATION
 - a) Assessor's Parcel Number
 - b) New Floor Area
 - c) Owners Name
 - d) Project Address

CERTIFICATION OF INSULATION

PART I GENERAL
PART II AREAS INSULATED
PART III CERTIFICATION

ADDRESS OR TRACT <div style="font-size: 1.2em; font-family: cursive;">K&B</div> <div style="float: right; margin-top: -20px;">LOT # 43</div> <div style="font-size: 1.5em; font-family: cursive; margin-top: 10px;">21 BLUE FERN CT.</div> <div style="font-size: 1.5em; font-family: cursive; margin-top: 10px;">California Gardens</div>	SACRAMENTO BUILDING PRODUCTS <input checked="" type="checkbox"/> P.O. BOX 854, WEST SACRAMENTO, CA 95691 LIC. #202026 <input type="checkbox"/> 1309 MELODY ROAD, MARYSVILLE, CA 95901 LIC. #202026 <input type="checkbox"/> P.O. BOX 9651, FRESNO, CA 93793-9651 LIC. #202026 <input type="checkbox"/> P.O. BOX 1631, RENO, NV 89505 LIC. #10675 <input type="checkbox"/> 3326 A PONDEROSA WAY, LAS VEGAS, NV 89118 LIC. #10675 DATE INSULATION COMPLETED <div style="font-size: 1.2em; font-family: cursive;">6-24-02</div>
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WALLS			CEILING			FLOORS		
SQUARE FEET			SQUARE FEET			SQUARE FEET		
TYPE OF INSULATION			TYPE OF INSULATION			TYPE OF INSULATION		
MATERIAL FIBERGLASS			MATERIAL FIBERGLASS			MATERIAL FIBERGLASS		
FORM BATTS			FORM BATTS & BLOW			FORM BATTS		
MANUFACTURER'S PRODUCT I.D.			MANUFACTURER'S PRODUCT I.D.			MANUFACTURER'S PRODUCT I.D.		
MANUFACTURER			MANUFACTURER			MANUFACTURER		
CT	OC	JM	CT	OC	JM	CT	OC	JM
R - VALUE INSTALLED			R - VALUE INSTALLED			R - VALUE INSTALLED		
APPLIED THICKNESS			APPLIED THICKNESS			APPLIED THICKNESS		
MIN. INSTALLED WEIGHT PER SQUARE FOOT			MIN. INSTALLED WEIGHT PER SQUARE FOOT			MIN. INSTALLED WEIGHT PER SQUARE FOOT		
KNEE WALLS IF R-VALUE IS OTHER THAN WALLS ABOVE			KNEE WALLS IF R-VALUE IS OTHER THAN WALLS ABOVE			KNEE WALLS IF R-VALUE IS OTHER THAN WALLS ABOVE		
MATERIAL FIBERGLASS			FORM BATTS			R VALUE		
AIR INFILTRATION SEALANT			AIR INFILTRATION SEALANT			AIR INFILTRATION SEALANT		
MATERIAL FOAM			MANUFACTURER HILTI			MANUFACTURER HANDY FOAM		

THIS IS TO CERTIFY THAT INSULATION AND/OR SEALANT HAS BEEN INSTALLED IN CONFORMANCE WITH APPLICABLE CODES, MATERIAL STANDARDS AND REGULATIONS.

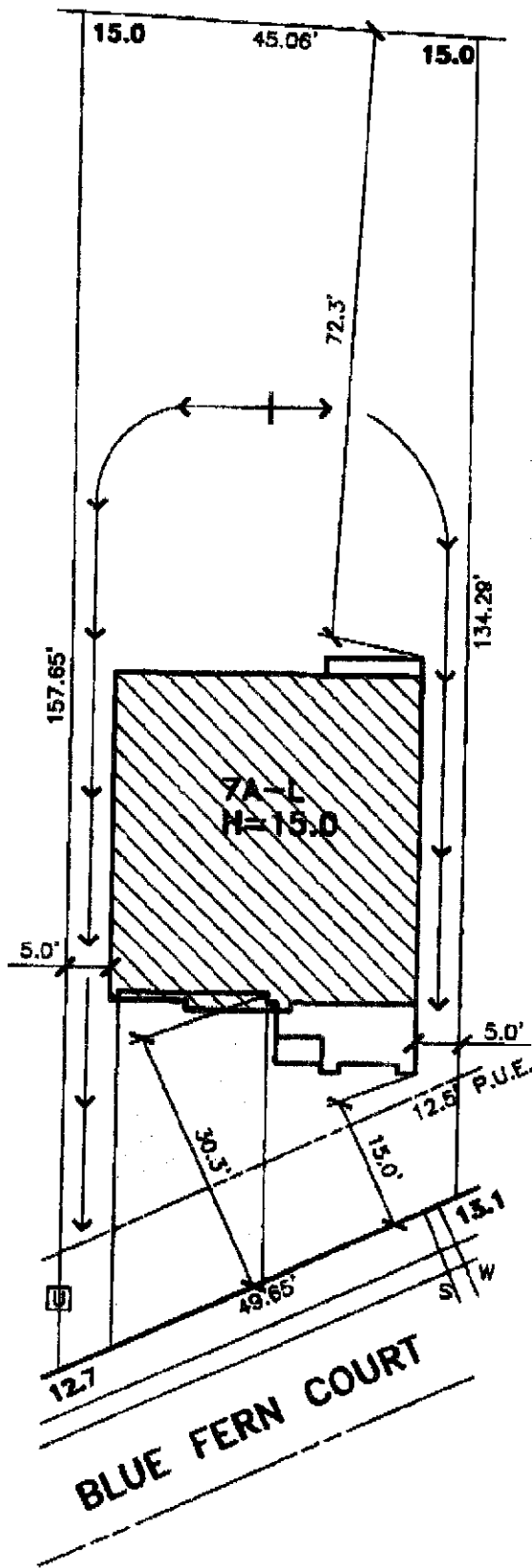
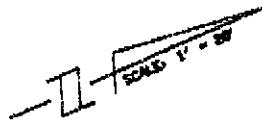
SIGNATURE — INSULATION CONTRACTOR <i>Jeff Cable</i>	TITLE MANAGER	DATE 7-1-02
SIGNATURE — GENERAL CONTRACTOR	TITLE	DATE
REMARKS		



WALLACE • KUHL & ASSOCIATES INC.
 GEOTECHNICAL ENGINEERING • CONSTRUCTION TESTING

3050 Industrial Blvd.
 PO Box 1137
 West Sacramento
 California 95691
 916-372-1434

DATE 5/31/02		JOB NO.		WEATHER		TEMP. ° at ° at		AM PM	
PROJECT Cal Gardens / K+B				Technician I <input type="checkbox"/>		Staff E/G <input type="checkbox"/>			
LOCATION Natoma s				Technician II <input type="checkbox"/>		Project E/G <input type="checkbox"/>			
TYPE OF WORK Pull tests				Technician III <input type="checkbox"/>		Senior E/G <input type="checkbox"/>			
Inside 50 mi. radius <input type="checkbox"/>		Outside 50 mi. radius <input type="checkbox"/>		Nuclear Densities <input type="checkbox"/>		Principal E/G <input type="checkbox"/>			
PERSONNEL		REG. HRS	OT HRS	TOTAL HRS	TRAVEL	ON JOB	VEHICLE		MILES
DB							#59		
CF									
OBSERVATIONS:									
Performed pull test on 7/8" anchor bolts									
to 11,700 LBS of pressure. 1/2" anchor bolts									
to 4000 LBS of pressure.									
Lot # 36 7/8" a.b. 2-each 1/2" o.b. 2-each									
Lot # 43 7/8" a.b. 2-each 1/2" a.b. 2-each									
Lot # 44 7/8" a.b. 2-each 1/2" a.b. 2-each									
All passed.									
FIELD REPORT				Signed					



DATE: 7-27-01
 A.P.N.: 274-0570-023
 ADDRESS: 21 BLUE FERN COURT

LOT AREA: 5,589 SF
 LOT COVERAGE: 22%

Stantec

Stantec Consulting Inc.
 2590 Venture Oaks Way
 Sacramento, CA 95833-3288
 Tel. 916.925.5550
 Fax. 916.921.9274
 www.stantec.com

**NATOMAS WEST
 VILLAGE 2**
 LOT 43
 PLAN 7A

CALIFORNIA GARDENS
 CITY OF SACRAMENTO, CA
 CLIENT: KAUFMAN & BROAD