CITY OF SACRAMENTO

1231 I Street, Sacramento, CA 95814

Permit No: 0113040

Insp Area:
Thos Bros:

337B4

Site Address: 7683 EL RITO WY SAC

Parcel No:

031-1250-014

Sub-Type:

NSFR

Housing (Y/N): N

CONTRACTOR

OWNER RODRIGUEZ 7683 EL RITO WY <u>ARCHITECT</u>

SAC CA. Nature of Work: NSFR - 1382 SF LVNG, 410 SF ATTCHD GAR, 143 SF CVRD PRCH CONSTRUCTION LENDING AGENCY: I hereby affirm under penalty of perjury that there is a construction lending agency for the performance of the work for which this permit is issued (Sec. 3097, Civ. C). Lender's Address Lender's Name LICENSED CONTRACTORS DECLARATION: I hereby affirm under penalty of perjury that I am licensed under provisions of Chapter 9 (commencing with section 7000) of Division 3 of the Business and Professions Code and my license is in full force and effect. License Class License Number Date Contractor Signature OWNER-BUILDER DECLARATION: I hereby affirm under penalty of perjury that I am exempt from the contractors License Law for the following reason (Sec. 7031.5, Business and Professions Code; any city or county which requires a permit to construct, alter, improve, demolish, or repair any structure, prior to its issuance, also requires the applicant for such permit to file a signed statement that he or she is licensed pursuant to the provisions of the Contractors License Law (Chapter 9 (commencing with Section 7000) of Division 8 of the Business and Professions Code) or that he or she is exempt therefrom and the basis for the alleged exemption. Any violation of Section 7031.5 by any applicant for a permit subjects the applicant to a civil penalty of not more than five hundred dollars (\$500.00); I, as a owner of the property, or my employees with wages as their sole compensation, will do the work, and the structure is not intended or offered for sale (Sec. 7044, Business and Professional Code: The Contractors License Law does not apply to an owner of property who builds or improves thereon, and who does such work himself or herself or through his/her own employees, provided that such improvements are not intended or offered for sale. If, however, the building or improvement is sold within one year of completion, the owner-builder will have the burden of proving that he/she did not build or improve for the purpose of sale.) I, as owner of the property, am exclusively contracting with licensed contractors to construct the project (Sec. 7044, Business and Professions Code: The Contractors License Law does not apply to an owner of property who builds or improves thereon, and who contracts for such projects with a contractor(s) licensed pursuant to the Contractors License Law). I am exempt under Sec._____B & PC for this reason:_____ Owner Signature____ IN ISSUING THIS BUILDING PERMIT, the applicant represents, and the city relies on the representation of the applicant, that the applicant verified all measurements and locations shown on the application or accompanying drawings and that the improvement to be constructed does not violate any law or private agreement relating to permissible or prohibited locations for such improvements. This building permit does not authorize any illegal location of any improvement or the violation of any private agreement relating to location of improvements. I certify that I have read this application and state that all information is correct. I agree to comply with all city and county ordinances and state laws relating to building construction and herby authorize representative(s) of this city to enter upon the abovementioned property for inspection purposes. ____ Applicant/Agent Signature_ WORKER'S COMPENSATION DECLARATION: I hereby affirm under penalty of perjury one of the following declarations: I have and will maintain a certificate of consent to self-insure for workers' compensation as provided for by Section 3700 of the Labor Code, forthe performance of work for which the permit is issued. I have and will maintain workers' compensation insurance, as required by Section 3700 of the Labor Code, for the performance of the work for which this permit is issued. My workers' compensation insurance carrier and policy number are: Policy Number Exp Date Carrier (This section need not be completed if the permit is for \$100 or less) I certify that in the performance of the work for which this permit is issued, I

WARNING: FAILURE TO SECURE WORKER'S COMPENSATION COVERAGE IS UNLAWFUL AND SHALL SUBJECT AN EMPLOYER TO CRIMINAL PENALTIES AND CIVIL FINES UP TO ONE HUNDRED THOUSAND DOLLARS (\$100,000) IN ADDITION TO THE COST OF COMPENSATION, DAMAGES AS PROVIDED FOR IN SECTION 3706 OF THE LABOR CODE, INTEREST AND ATTORNEY'S FEE.

shall not employ any person in any manner so as to become subject to the workers' compensation laws of California and agree that if I should become

subject to the workers' compensation provisions of Section 3700 of the Labor Code, I shall forthwith comply with those provisions.

Applicant Signature

Lupe Guerrrero Construction

6-22-04

1770 Toiyabe Ln. Stockton, CA 95206 Phone: 831-809-3685

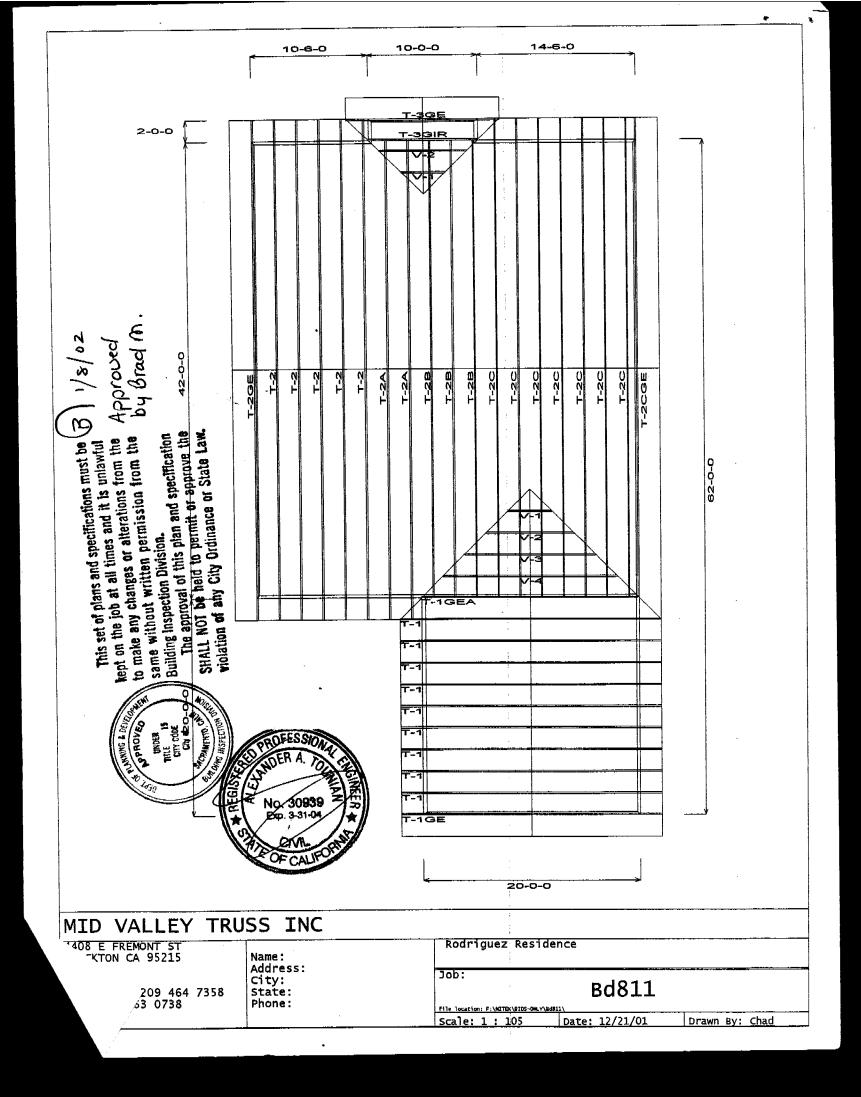
L.C. # 667980

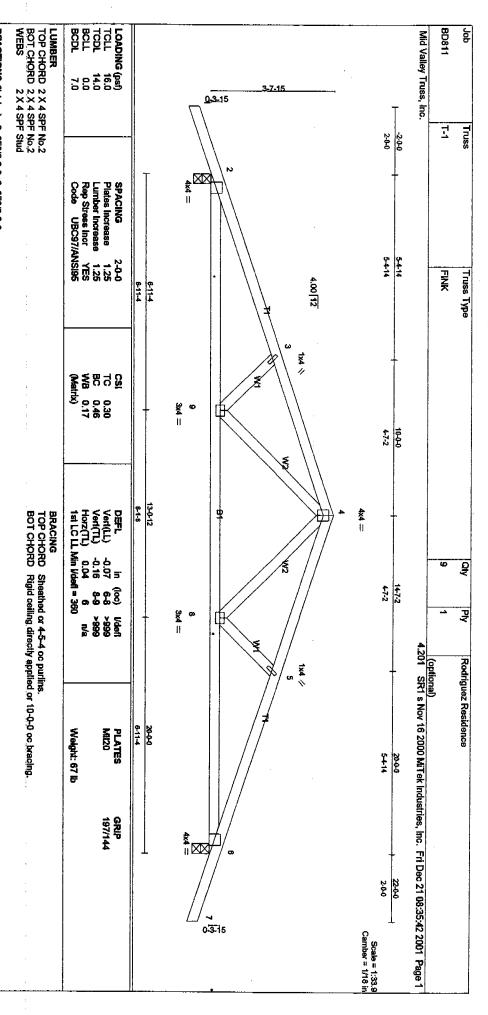
Mr. Frai halphy has pennision Mr. Frai halphy has pennision for Be Isvee to obtain permision to Beek for tinal Extension permit to a seek for tinal inspection on Mr. Costulo Rodriguez house inspection on Mr. Costulo Rodriguez house Address 7683 EL Rito Wy Assessing Percel No. 031-1230-014 Assessing Percel No. 031-1230-014 Sixensy yours Thompson Sixensy yours Thompson Sixensy yours Thompson Gang guestion guestion guestion

RE	VISION	ON A	CTIVE	PERM	IT		ı		· .
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CODE									
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I understand that I am responsible for all pland that any approved plans not claimed are invoice procedure for the amount due will delay of final approval for the subject project.	d paid for paid for paid paid paid paid paid paid paid paid	or withi	n 3 mor	iths of i	otificat	ion wil	l be dis	sposed of	and an
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DATE NOTIFIED	PLAN BIN
APP FEE	PAID
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AGENCY	TOTAL HRS	TOTAL FEES
BLDG	. with	
PW		
PLEASE PAY	THIS AMOUNT	
10,224,000,116,000		





REACTIONS (lb/size) 2=858/0-3-8, 6=858/0-3-8 Max Horz 2=10(load case 3)

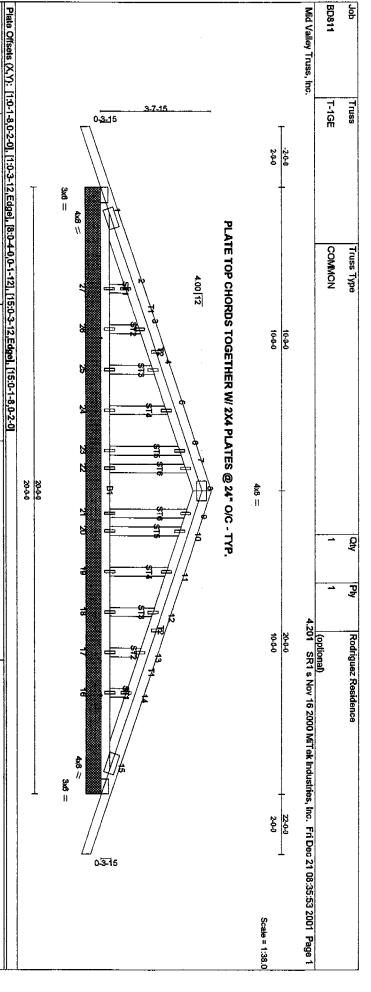
FORCES (ib) - First Load Case Only
TOP CHORD 1-2=37, 2-3=-1651, 3-4=-1438, 4-5=-1438, 5-6=-1651, 6-7=37
BOT CHORD 2-9=1510, 8-9=1053, 6-8=1510
WEBS 3-9=-277, 4-9=401, 4-8=401, 5-8=-277

1) This truss has been designed for unbalanced loading conditions.

2) This truss has been designed for the wind loads generated by 80 mph winds at 25 ft above ground level, using 9.3 psf top chord dead load and 5.0 psf bottom chord dead load, 100 ml from hurricane oceanline, on an occupancy category I, condition I enclosed building, of dimensions 45 ft by 24 ft with exposure B ASCE 7-93 per UBC97/ANSI95 if end verticals or cantilevers exist they are exposed to wind. If porches exist, they are not exposed to wind. The lumber DOL increase is 1.33, and the plate grip increase is 1.33

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads per Table No. 16-B, UBC-97.





TCLL 16.0 TCDL 14.0 BCLL 0.0 BCDL 7.0 Rep Stress Incr SPACING Lumber Increase Plates Increase ING 2-0-0 Increase 1.25 ar Increase 1.25 tress Incr YES UBC97/ANSI95 **3** 5 8 **8** 0.04 Yerf(LL) 1st LC LL Min Vdefl = 360 8 PLATES MIIZO Weight: 97 lb

REACTIONS (Ib/size) 1=78/20-0-0, 15=78/20-0-0, 22=78/20-0-0, 21=78/20-0-0, 23=66/20-0-0, 24=99/20-0-0, 25=107/20-0-0, 26=63/20-0-0, 27=188/20-0-0, 10=66/20-0-0, 10=99/20-0-0, 18=107/20-0-0, 17=63/20-0-0, 16=188/20-0-0

Max Horz 1=8(load case 3)

TOP CHORD BRACING

Except: Row at midpt

BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing

1-8, 8-15

TOP CHORD: 2 X 4 SPF No.2
BOT CHORD: 2 X 4 SPF No.2
OTHERS: 2 X 4 SPF Stud

Max Uplift23=-1 (load case 3), 20=-1 (load case 4)

Max Gray 1=78(load case 1), 15=78(load case 1), 22=78(load case 1), 21=78(load case 1), 23=68(load case 6), 24=100(load case 6), 25=107(load case 1), 26=63(load case 6), 27=188(load case 1), 20=68(load case 7), 19=100(load case 7), 18=107(load case 1), 17=63(load case 7), 16=188(load case 1)

FORCES (b) - First Load Case Only
TOP CHORD 1-2=18, 2-3=-26, 3-4=-18, 4-5=-20, 5-6=-20, 6-7=-12, 7-8=-21, 8-9=-21, 9-10=-12, 10-11=-20, 11-12=-20, 12-13=-18, 13-14=-26, 14-15=-29
BOT CHORD 1-27=7, 26-27=7, 25-26=7, 24-25=7, 23-24=7, 22-23=7, 21-22=7, 20-21=7, 19-20=7, 18-19=7, 17-18=7, 16-17=7, 16-16=7
BOT CHORD 1-27=7, 26-27=7, 25-26=7, 24-25=7, 23-24=7, 22-23=7, 21-22=7, 20-21=7, 19-20=-34, 11-19=-81, 12-18=-85, 13-17=-57, 14-16=-139
WEBS 7-22=-62, 9-21=-62, 6-23=-54, 5-24=-81, 4-25=-85, 3-26=-57, 2-27=-139, 10-20=-54, 11-19=-81, 12-18=-85, 13-17=-57, 14-16=-139

1) This truss has been checked for unbalanced loading conditions.

2) This truss has been designed for the wind loads generated by 80 mph winds at 25 ft above ground level, using 9.3 psf top chord dead load and 5.0 psf bottom chord dead kad, 100 mil from hurricane oceanline, on an occupancy category I, condition lenclosed building, of dimensions 45 ft by 24 ft with exposure B ASCE 7-93 per UBC97/ANSI95 if end verticals or cantilevers exist, they are exposed to wind. If porches exist, they are not exposed to wind. The lumber DOL increase is 1.33, and the plate grip increase is 1.33

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MTek "Standard Gable End Detail"

4) All plates are 1x4 Mil20 unless otherwise indicated.

5) Gable requires continuous bottom chord bearing.

6) Gable studs spaced at 1.4-0 oc.

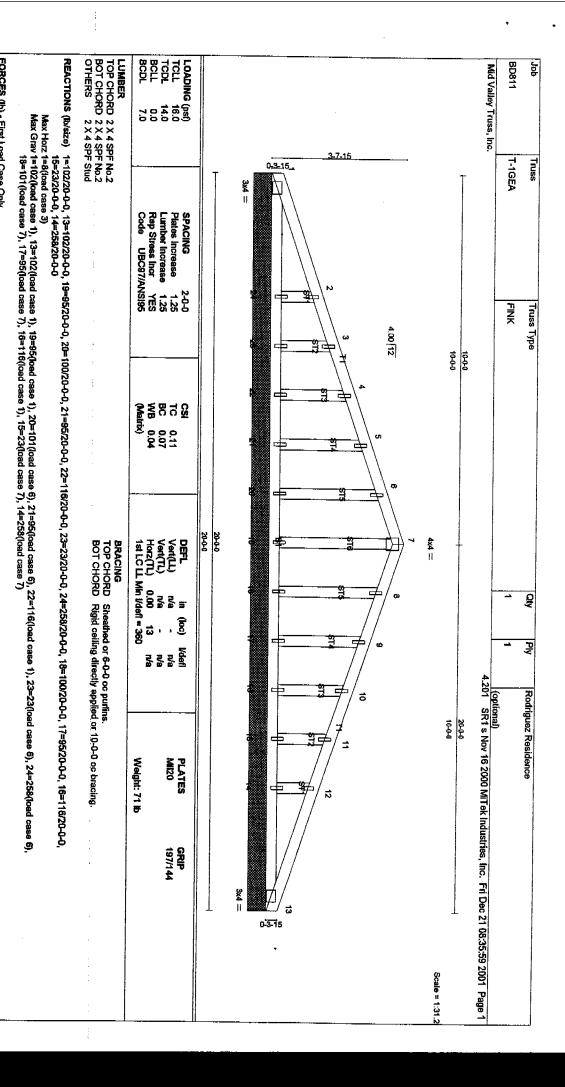
7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads per Table No. 16-B, UBC-97

Continued on page 2



Jab BD811	Mid Val	NOTES 8) Provic 9) This (10) Desi	 				
	Mid Valley Truss, Inc.	NOTES 8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 1 lb uplift at joint 23 and 1 lb uplift at joint 20. 9) This truss has been designed with ANSI/TPI 1-1995 criteria. 10) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails. LOAD CASE(S) Standard					
Truss	DC.	ical conne een desigr es 4x2 (flat tandard		•			
		ction (by of sed with AN orientation					
		hers) of tru ISI/TPI 1-11) purlins at			:		
S =		ss to beari 995 criteric			:	,	
Truss Type COMMON		ng ptate ca a. g indicated			:		
		pable of wi					
		ithstanding to truss TC					
		1 lb uplift a w/ 2-10d n					
		1 joint 23 au ails.		•	i		
		nd 1 ib uplii					
1 00		t at joint 20			;		
1 Ply					:		
Rodrigu	(options	<u> </u>			:		
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FORCES (lb) - First Load Case Only

TOP CHORD 1-2=30, 2-3=-30, 3-4=11, 4-5=-18, 5-6=8, 6-7=8, 7-6=-17, 8-9=-17, 9-10=-18, 10-11=-12, 11-12=-30, 12-13=-34

BOT CHORD 1-24=4, 23-24=4, 22-23=4, 21-22=4, 20-21=4, 19-20=4, 18-19=4, 17-18=4, 16-17=4, 15-16=4, 14-15=4, 13-14=4

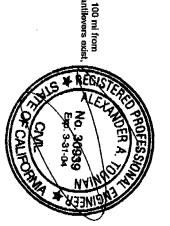
WEBS 7-19=-77, 6-20=-80, 5-21=-78, 4-22=-90, 3-23=-31, 2-24=-189, 8-18=-80, 9-17=-78, 10-16=-90, 11-15=-31, 12-14=-189

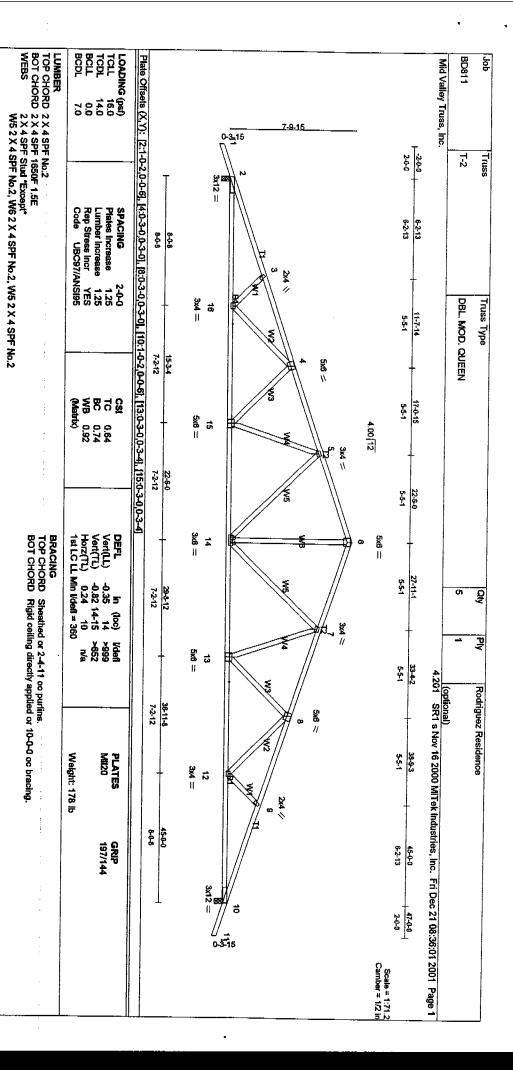
1) This truss has been checked for unbalanced loading conditions.

2) This truss has been designed for the wind loads generated by 80 mph winds at 25 ft above ground level, using 9.3 psf top chord dead load and 5.0 psf bottom chord dead load, 100 mi from hurricane oceanline, on an occupancy category I, condition I enclosed building, of dimensions 45 ft by 24 ft with exposure B ASCE 7-93 per UBC97/ANSI95 if end verticals or cantilevers exist, they are not exposed to wind. The lumber DOL increase is 1.33, and the plate grip increase is 1.33

3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MITek "Standard Gable End Detail"

Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MITek "Standard Gable A) All plates are 1x4 MII20 unless otherwise indicated.
 Gable requires continuous bottom chord bearing.
 Gable studs spaced at 1-4-0 oc.
 This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads per Table No. 16-B, UBC-97.
 This truss has been designed with ANSI/TPI 1-1995 criteria.



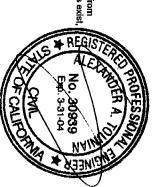


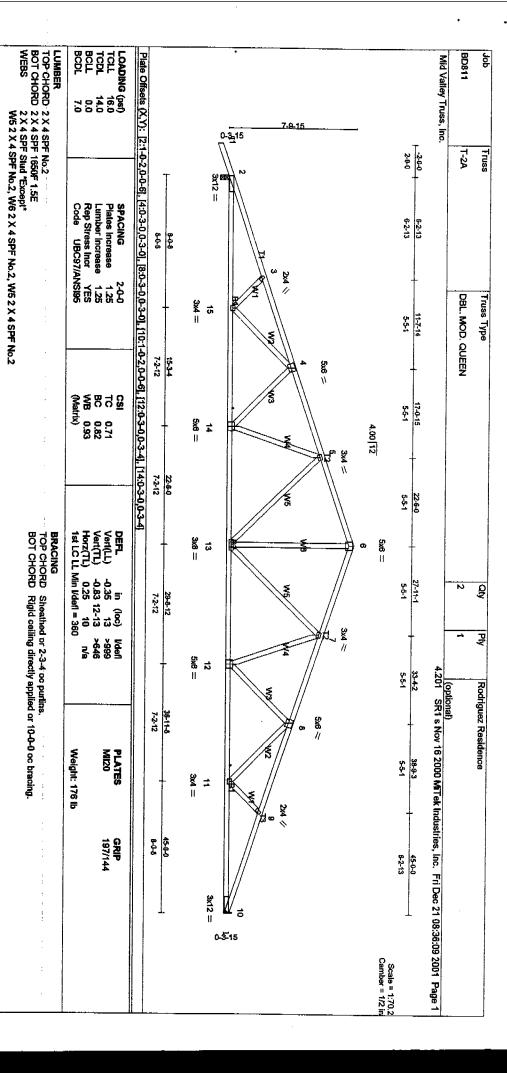
FORCES (lb) - First Load Case Only TOP CHORD 1-2=37, 2-3=-4402, 3-4=-4163, 4-5=-3430, 5-6=-2650, 6-7=-2650, 7-8=-3430, 8-9=-4163, 9-10=-4402, 10-11=37 BOT CHORD 2-16=4104, 15-16=3600, 14-15=3037, 13-14=3037, 12-13=3600, 10-12=4104 WEBS 3-16=-299, 4-16=442, 4-15=-583, 5-15=557, 5-14=-829, 6-14=1308, 7-14=-829, 7-13=557, 8-13=-583, 8-12=442, 9-12=-299

REACTIONS (lb/size) 2=1783/0-3-8, 10=1783/0-3-8

Max Horz 2=20(load case 3)

This truss has been checked for unbalanced loading conditions.
 This truss has been designed for the wind loads generated by 80 mph winds at 25 ft above ground level, using 9.3 psf top chord dead load and 5.0 psf bottom chord dead load, 100 ml from hurricane oceanline, on an occupancy category I, condition I enclosed building, of dimensions 45 ft by 24 ft with exposure B ASCE 7-33 per UBC97/ANSI95 if end verticals or cantilevers exist, they are exposed to wind. If porchas exist, they are not exposed to wind. The lumber DOL increase is 1.33, and the plate grip lincrease is 1.33
 This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads per Table No. 16-B, UBC-97.





REACTIONS (lb/size) 2=1786/0-3-8, 10=1651/Mechanical

Max Horz 2=42(load case 3)

NOTES

FORCES (Ib) - First Load Case Only
TOP CHORD 1-2=37, 2-3=-4411, 3-4=-4172, 4-5=-3440, 5-6=-2660, 6-7=-2660, 7-8=-3448, 8-9=-4219, 9-10=-4466
BOT CHORD 2-15=4112, 14-15=3609, 13-14=3046, 12-13=3052, 11-12=3627, 10-11=4170
WEBS 3-15=-298, 4-15=442, 4-14=-583, 5-14=556, 5-13=-829, 6-13=1314, 7-13=-837, 7-12=564, 8-12=-597, 8-11=480, 9-11=-319

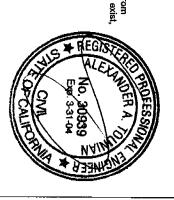
1) This truss has been checked for unbalanced loading conditions.

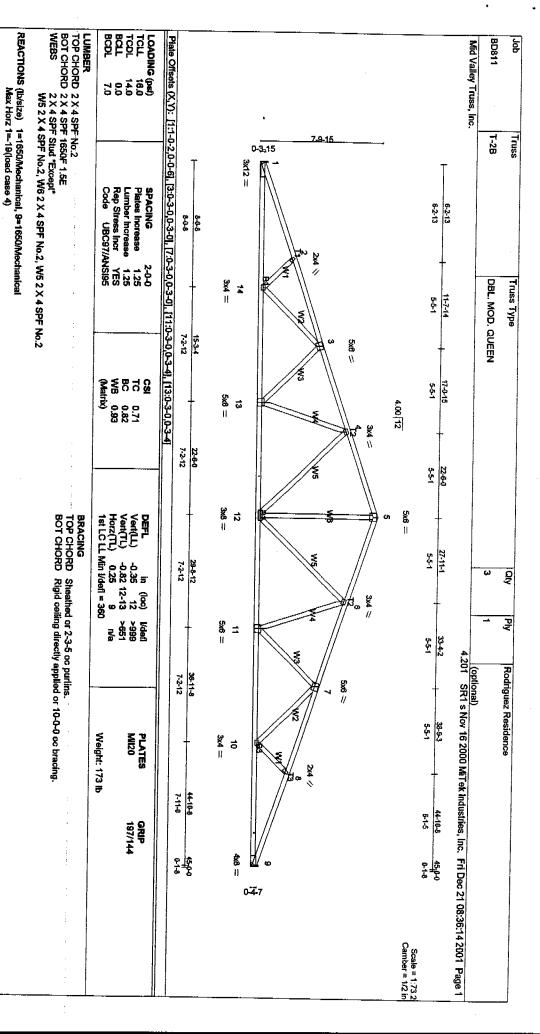
2) This truss has been designed for the wind loads generated by 80 mph winds at 25 ft above ground level, using 9.3 psf top chord dead load and 5.0 psf bottom chord dead load, 100 mi from hurricane oceanline, on an occupancy category, condition I enclosed building, of dimensions 45 ft by 24 ft with exposure B ASCE 7-33 per UBC97/ANS/95 ff end verticals or cantilevers exist, they are exposed to wind. If porches exist, they are not exposed to wind. The lumber DOL increase is 1.33, and the plate grip increase is 1.33

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads per Table No. 16-B, UBC-97.

4) Refer to girder(s) for truss to truss connections.

5) This truss has been designed with ANSI/TPI 1-1995 criteria.





TOP CHORD 1-2-4462, 2-3-4-214, 3-4-3443, 4-5-2655, 5-6-2655, 6-7-3429, 7-8-4160, 8-9-4392

BOT CHORD 1-14-4165, 13-14-3622, 12-13-3047, 11-12-3039, 10-11-3597, 9-10-4090

WEBS 2-14-319, 3-14-480, 3-13-597, 4-13-563, 4-12-837, 5-12-1312, 6-12-825, 6-11-550, 7-11-578, 7-10-443, 8-10-283

FORCES (lb) - First Load Case Only

1) This truss has been checked for unbelanced loading conditions.

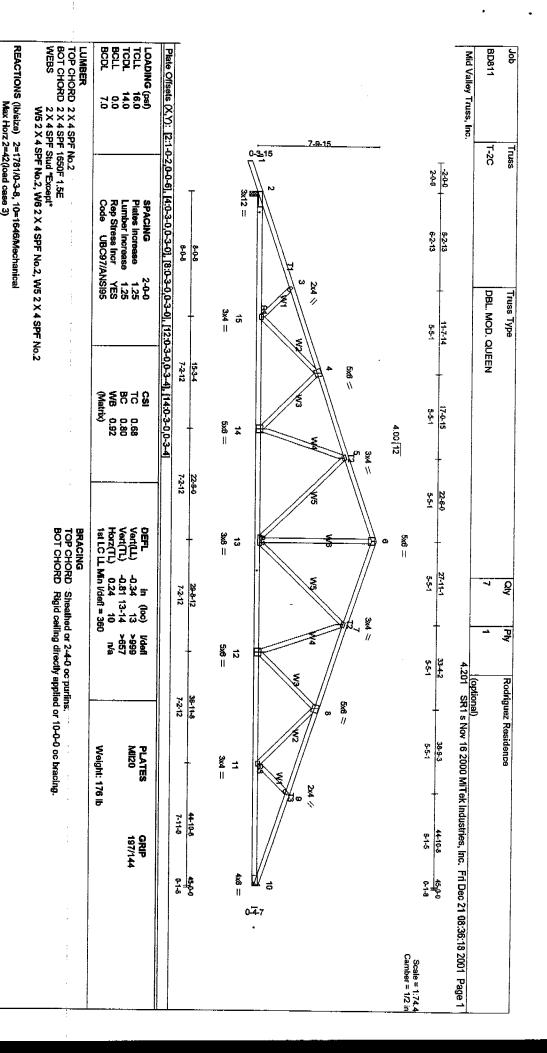
2) This truss has been designed for the wind loads generated by 80 mph winds at 25 ft above ground level, using 9.3 psf top chord dead load and 5.0 psf bottom chord dead load, 100 mi from hurricane oceanline, on an occupancy category I, condition I enclosed building, of dimensions 45 ft by 24 ft with exposure B ASCE 7-93 per UBC97/ANSI95 if end verticals or cantilevers exist, they are exposed to wind. If porches exist, they are not exposed to wind. The lumber DOL increase is 1.33, and the plate grip increase is 1.33

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads per Table No. 16-B, UBC-97.

4) Refer to girder(s) for truss connections.

5) This truss has been designed with ANS/ITPI 1-1995 criteria.





FORCES (fb) - First Load Case Only

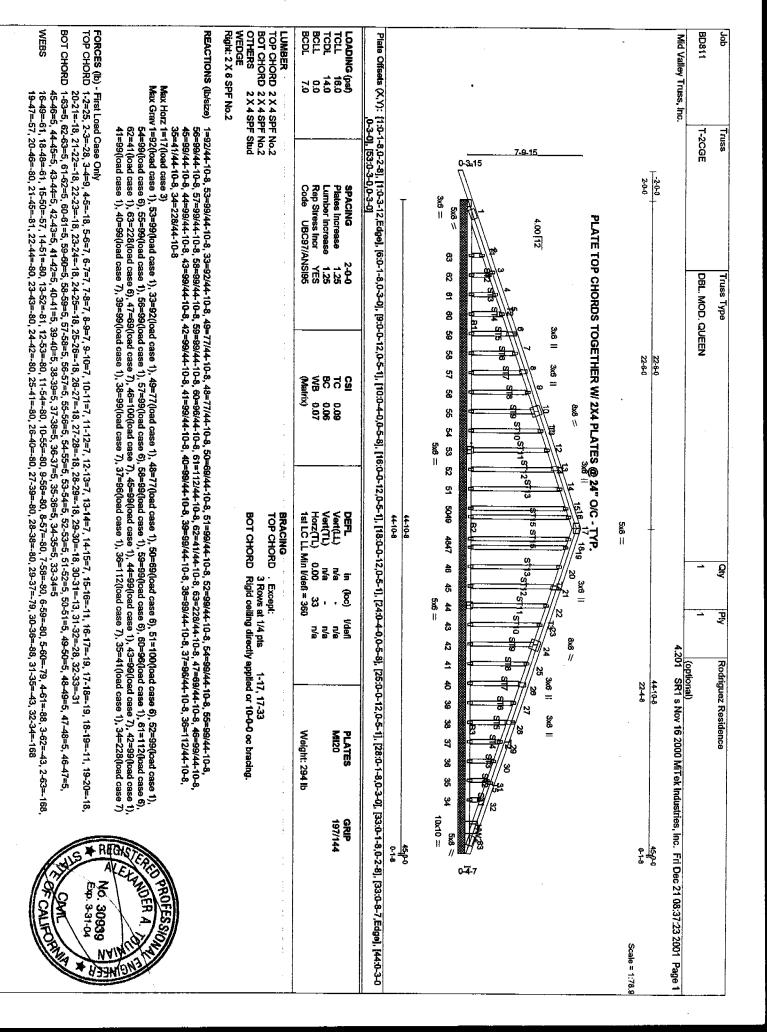
TOP CHORD 1-2=37, 2-3=-4397, 3-4=-4158, 4-5=-3426, 5-6=-2645, 6-7=-2645, 7-8=-3420, 8-9=-4151, 9-10=-4383

BOT CHORD 2-15=4099, 14-15=3596, 13-14=3033, 12-13=3030, 11-12=3588, 10-11=4082

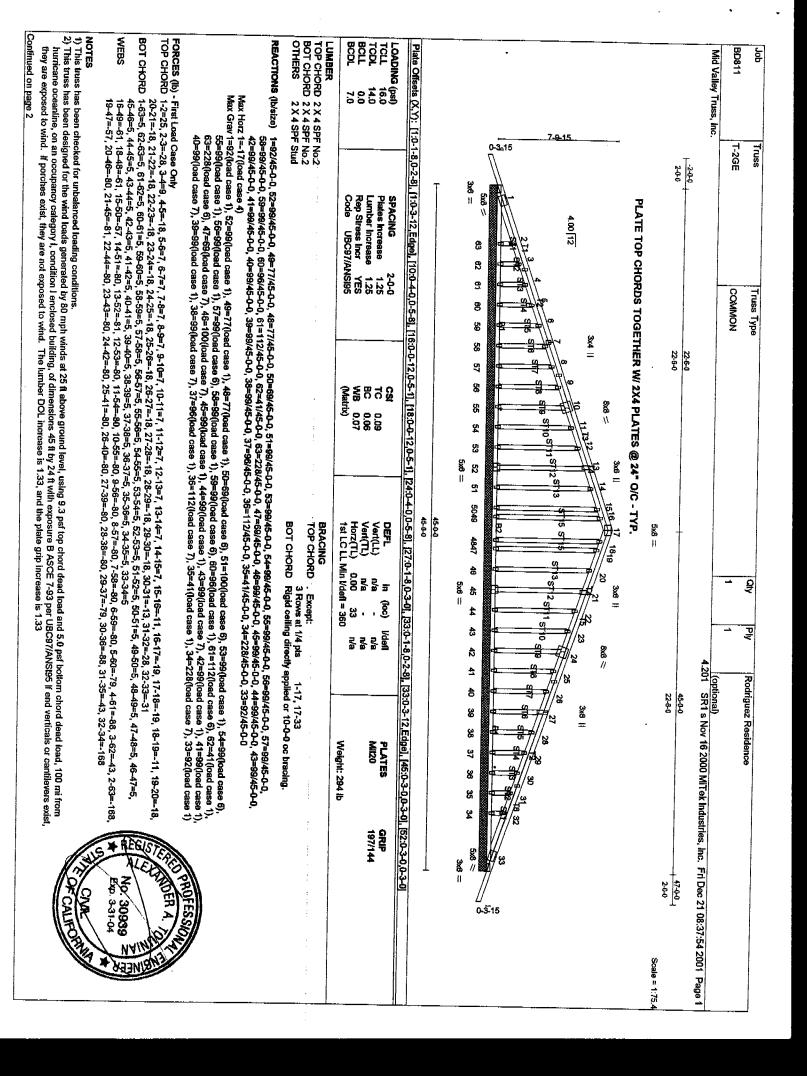
WEBS 3-15=-299, 4-15=442, 4-14=-583, 5-14=556, 5-13=-829, 6-13=1305, 7-13=-825, 7-12=550, 8-12=-579, 8-11=443, 9-11=-284

- 1) This truss has been checked for unbalanced loading conditions.
 2) This truss has been designed for the wind loads generated by 80 mph winds at 25 ft above ground level, using 9.3 psf top chord dead load and 5.0 psf bottom chord dead load, 100 mi from hurricane oceanline, on an occupancy category I, condition I enclosed building, of dimensions 45 ft by 24 ft with exposure B ASCE 7-33 per UBC97/ANSI95 if end verticals or cantilevers exist, they are exposed to wind. If percentage wind, if percentage is 1.33
 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads per Table No. 16-B, UBC-97.
 4) Refer to girder(s) for truss to truss connections.
 5) This truss has been designed with ANSI/TPI 1-1995 criteria.

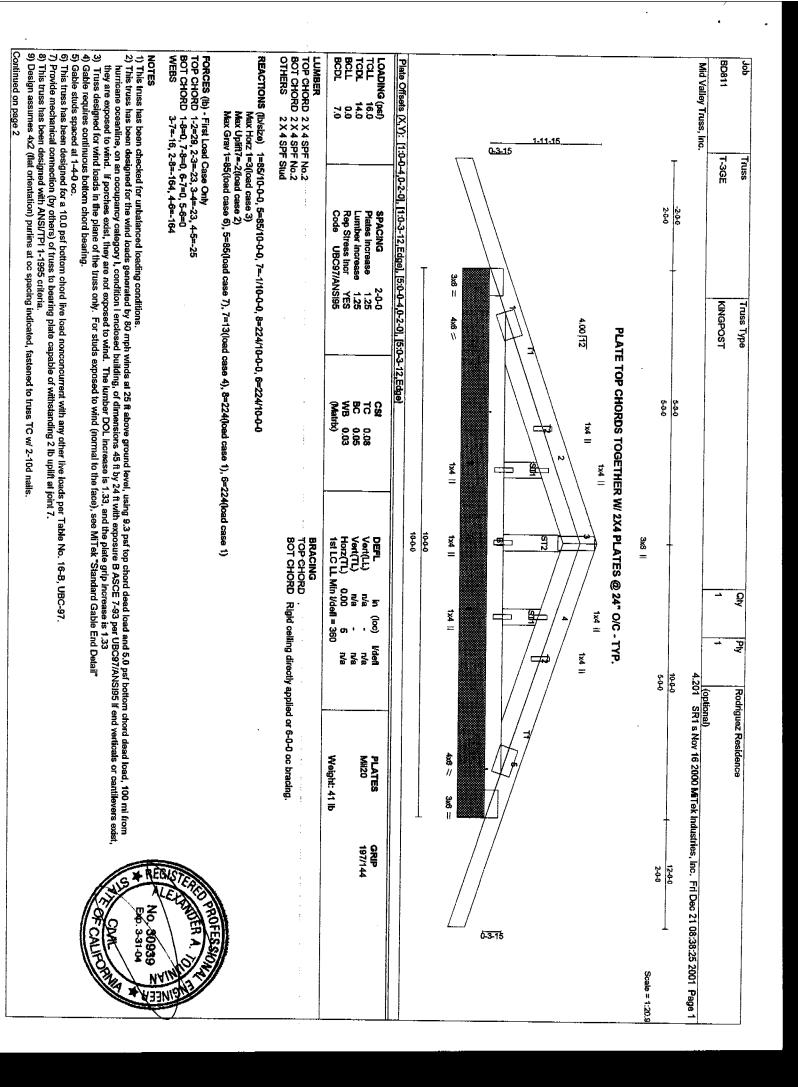




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	NOTES 1) This truss has been checategory i, condition ie lumber DOL Increase is lumber 2x4 Mill20 5) Gable requires continue 6) Gable stuts spaced at 7) This truss has been de 8) This truss has been de 8) Design assumes 4x2 (fill LOAD CASE(S) Standard	BD811	Job
	NOTES 1) This truss has been checked for unbalanced loading conditions. 2) This truss has been designed for the wind loads generated by 80 category 1, condition 1 enclosed building, of dimensions, 45 ft by 20 lumber DOL increase is 1.33, and the plate grip increase is 1.33 and the plate grip increase is 1.33 and the plate grip increase is 1.33 are the plate grip increase is 1.33 and the plate grip increase is 1.33 are the plate grip increase is 1.33 and the plate grip increase is 1.33 are the	T-2CGE	Truss
	ibalanced loading cone wind loads general ding, of dimensions, he plane of the truss, he plane of the truss invise indicated. Thord bearing. 10.0 psf bottom chord ANSI/TPI 1-1995 crit hi) purlins at oc space.		
	nditions. Ited by 80 mph winds 45 ft by 24 ft with exp is 1.33 only. For studs expo rd live load nonconculeria. ing indicated, fasten	DBL. MOD. QUEEN	Truss Type
	at 25 ft above groun xosure B ASCE 7-931 sed to wind (normal urrent with any other I and to truss TC w/ 2-1		
	NOTES 1) This truss has been checked for unbalanced loading conditions. 2) This truss has been designed for the wind loads generated by 80 mph winds at 25 ft above ground level, using 9.3 psf top chord dead locategory I, condition I enclosed building, of dimensions 45 ft by 24 ft with exposure B ASCE 7-93 per UBC97/ANS)95 if end verticals or category I, condition I enclosed building, of dimensions 45 ft by 24 ft with exposure B ASCE 7-93 per UBC97/ANS)95 if end verticals or category I, condition I enclosed building, of dimensions 45 ft by 24 ft with exposure B ASCE 7-93 per UBC97/ANS)95 if end verticals or category I, condition I enclosed building, of dimensions 45 ft by 24 ft with exposure B ASCE 7-93 per UBC97/ANS)95 if end verticals or category I, condition I enclosed building, of dimensions 42 ft end verticals or category I, cate		
	top chord dead load a end verticals or canti sk "Standard Gable E lo. 16-B, UBC-97.		Qly
	Mid Valley Truss, Inc. A201 SR1 s Nov 16 2000 MTek Industries, Inc. Fit Dec 21 08:37:24 2001 Page 1) This truss has been checked for unbalanced loading conditions. 2) This truss has been designed for the wind loads generated by 80 mph winds at 25 ft above ground level, using 9.3 psf top chord dead load and 5.0 psf bodom chord dead load, 190 mi from hurricane oceanline, on an occupancy lumber DOL increase is 1.33, and the plate grip increase is 1.33 3) Truss designed for wind leads in the plane of the truss only. For studs exposed to wind (normal to the face), see MTek "Standard Gable End Detail" 3) Truss designed for a 1.04 place of the truss only. For studs exposed to wind (normal to the face), see MTek "Standard Gable End Detail" 5) Gable studs spaced at 1.4-0 oc. 7) This truss has been designed for a 1.04 per total control live load nonconcurrent with any other live loads per Table No. 16-B, UBC-97. 8) Trust truss has been designed with AUSI/TP1 1.1995 criteria. 9) Design assumes 4x2 (flat orientation) purifins at oc spacing indicated, fastened to truss TC w/ 2-10d nails. LOAD CASE(S) Standard		Ply Rodriguez
	(Optional) 4.201 SR1 s Nov 16 2000 MTek Industries, Inc. Fri Dec 21 08:37:24 2001 Page 2 page 2 state of the provided load, 100 mi from hurricane oceanfine, on an occupancy safe, they are exposed to wind. If porches exist, they are not exposed to wind. The sil"	,	Rodriguez Residence
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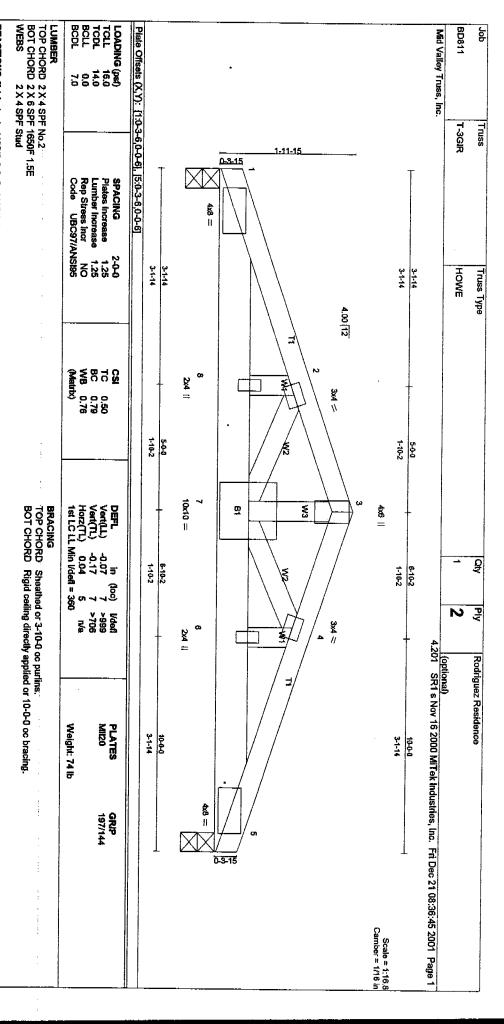


BD811	Mid V	NOTES 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail" 4) All plates are 2x4 Mil20 unless otherwise indicated. 5) Gable requires continuous bottom chord bearing. 6) Gable studs spaced at 1.4-0 or. 7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads per Table No. 16-B, UBC-97. 8) This truss has been designed with ANSI/TPI 1-1995 criteria. 9) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.	LOAD CASE(S) Standard				
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REACTIONS (lb/size) 1=4105/0-3-8, 5=4105/0-3-8 Max Horz 1=4(load case 3)

FORCES (lb) - First Load Case Only
TOP CHORD 1-2=-8377, 2-3=-6720, 3-4=-6720, 4-5=-8377
BOT CHORD 1-8=7921, 7-8=7921, 6-7=7921, 5-6=7921
WEBS 2-8=1353, 3-7=3841, 4-6=1353, 2-7=-1723, 4-7=-1723

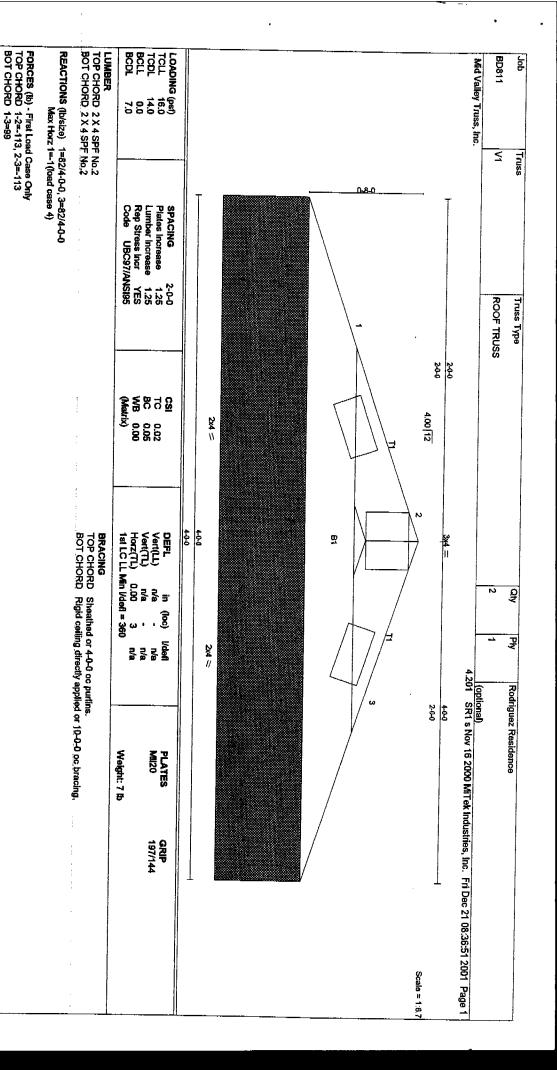
1) This truss has been checked for unbalanced loading conditions.
2) This truss has been designed for the wind loads generated by 80 mph winds at 25 ft above ground level, using 9.3 psf top chord dead load and 5.0 psf bottom chord dead load, 100 mi from hurricane oceanline, on an occupancy category I, condition tenclosed building, of dimensions 45 ft by 24 ft with exposure B ASCE 7-93 per UBC97/ANSI95 if end verticals or cantilevers exist, they are exposed to wind. If porches exist, they are not exposed to wind. The lumber DOL increase is 1.33, and the plate grip increase is 1.33
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads per Table No. 16-B, UBC-97.
4) This truss has been designed with ANSI/TPI 1-1995 criteria.
5) 2-ply truss to be connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
Bottom chords connected as follows: 2 X 6 - 2 rows at 0-6-0 oc.

Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.



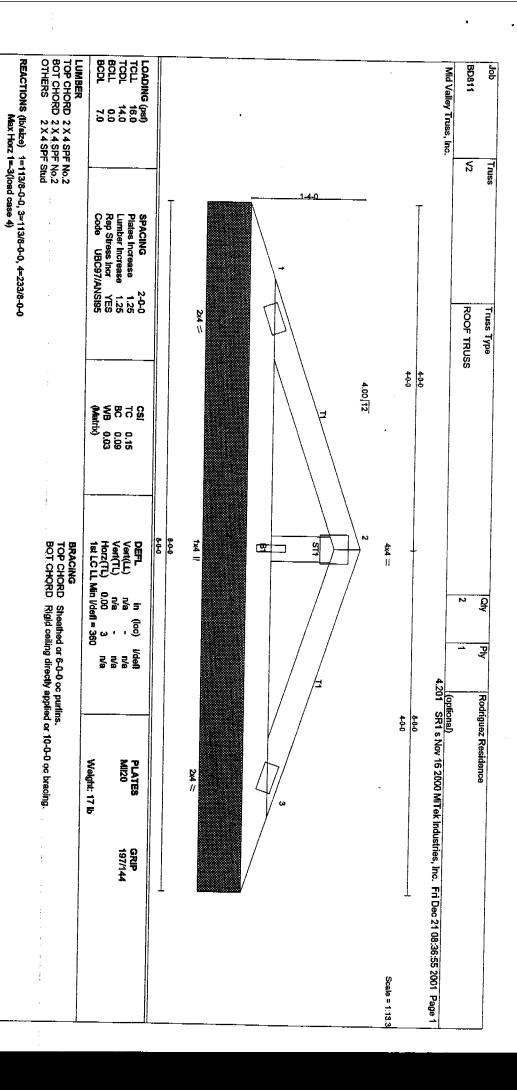
LOAD CASE(S) Standard	Mid Valley Truss, Inc.	BD811	dol
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		3	Ply
4.20			
4.201 SR1 s Nov 16 2000 MiTek Industries, Inc. Fri Dec 21 08:36:47 2001 Page 2	(optional)		Rodriguez Residence

1) Regular: Lumber Increase=1.25, Plate Increase=1.25
Uniform Loads (plf)
Vert: 1-5=-785.6, 1-3=-60.0, 3-5=-60.0



1) This truss has been checked for unbalanced loading conditions.
2) This truss has been designed for the wind loads generated by 80 mph winds at 25 ft above ground level, using 9.3 psf top chord dead load and 5.0 psf bottom chord dead load, 100 mi from hurricane oceanline, on an occupancy category I, condition I enclosed building, of dimensions 45 ft by 24 ft with exposure B ASCE 7-93 per UBC97/ANSI95 if end verticals or cantillevers exist, they are exposed to wind. If porches exist, they are not exposed to wind. The lumber DOL increase is 1.33, and the plate grip increase is 1.33
3) Gable requires continuous bottom chord bearing.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads per Table No. 16-B, UBC-97.

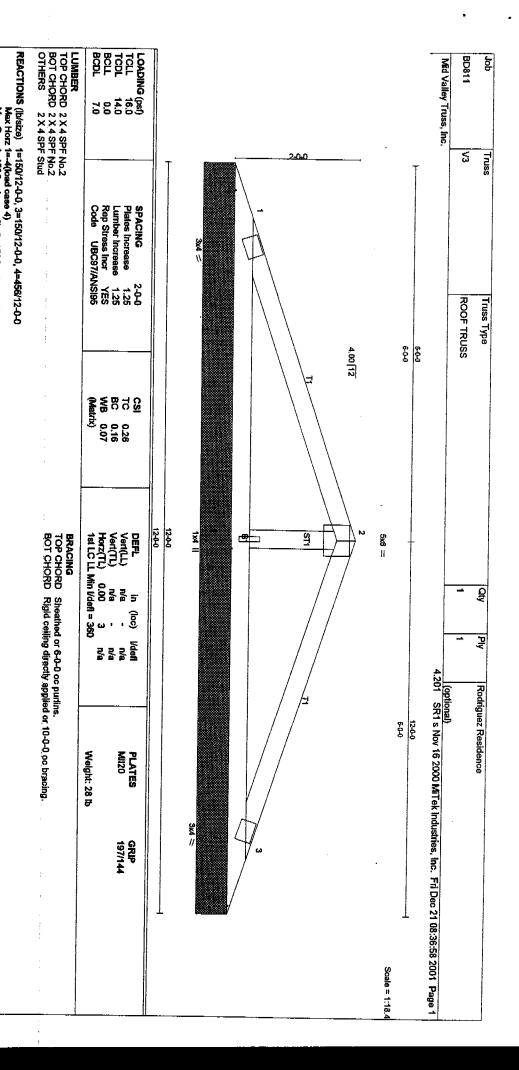
NOTES



FORCES (lb) - First Load Case Only TOP CHORD 1-2=12, 2-3=-35 BOT CHORD 1-4=11, 3-4=11 WEBS 2-4=-179

This truss has been checked for unbalanced loading conditions.
 This truss has been designed for the wind loads generated by 80 mph winds at 25 ft above ground level, using 9.3 psf top chord deed load and 5.0 psf bottom chord dead load, 100 mi from hurricane oceanline, on an occupancy category I, condition I enclosed building, of dimensions 45 ft by 24 ft with exposure B ASCE 7-93 per UBC97/ANSI95 if end verticals or cantilevers exist, they are exposed to wind. If porches exist, they are not exposed to wind. The lumber DOL increase is 1.33, and the plate grip increase is 1.33
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 This truss has been designed with ANSI/TPI 1-1995 criteria.



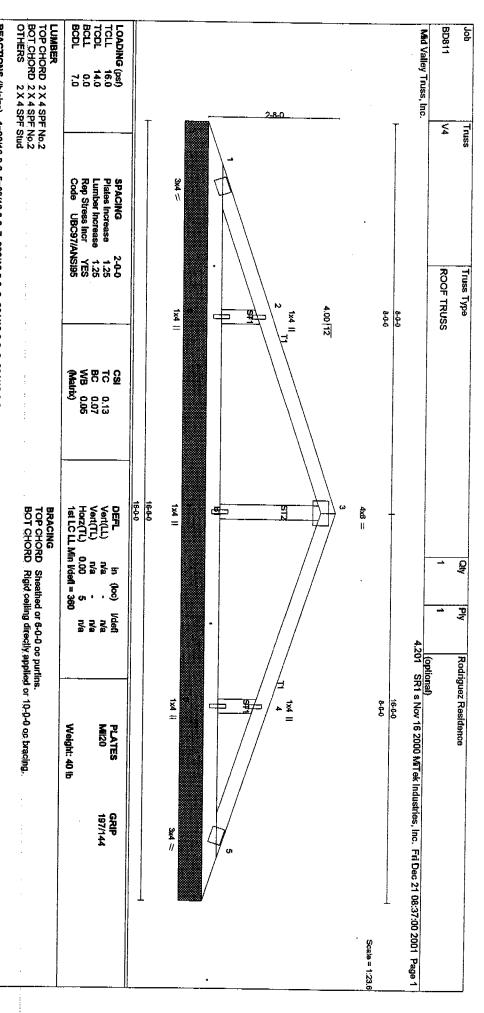


FORCES (fb) - First Load Case Only TOP CHORD 1-2=31, 2-3=-66 BOT CHORD 1-4=24, 3-4=24 WEBS 2-4=-336

Max Grav 1=158(load case 6), 3=158(load case 7), 4=456(load case 1)

- This truss has been checked for unbalanced loading conditions.
 This truss has been designed for the wind loads generated by 80 mph winds at 25 ft above ground level, using 9.3 psf top chord dead load and 5.0 psf bottom chord dead load, 100 mi from hurricans oceaniline, on an occupancy category I, condition I enclosed building, of dimensions 45 ft by 24 ft with exposure B ASCE 7-93 per UBC97/ANSI95 if end verticals or cantilevers exist, they are exposed to wind. If porches exist, they are not exposed to wind. The lumber DOL increase is 1.33, and the plate grip increase is 1.33
 Gable requires continuous bottom chord bearing.
 This truss has been designed with ANSI/TPI 1-1995 criteria.
- LOAD CASE(S) Standard



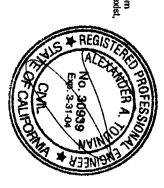


REACTIONS (lb/size) 1=90/16-0-0, 5=90/16-0-0, 7=289/16-0-0, 8=291/16-0-0, 6=291/18-0-0 Max Horz 1=-6(load case 4)

Max Grav 1=90(load case 1), 5=90(load case 1), 7=289(load case 1), 8=300(load case 6), 6=300(load case 7)

FORCES (lb) - First Load Case Only
TOP CHORD 1-2=19, 2-3=22, 3-4=-53, 4-5=-40
BOT CHORD 1-8=16, 7-8=16, 6-7=16, 5-6=16
WEBS 3-7=-235, 2-8=-228, 4-6=-228

- 1) This truss has been checked for unbalanced loading conditions.
 2) This truss has been designed for the wind toads generated by 80 mph winds at 25 ft above ground level, using 9.3 psf top chord dead load and 5.0 psf bottom chord dead load, 100 mi from hurricane oceanline, on an occupancy category I, condition I enclosed building, of dimensions 45 ft by 24 ft with exposure B ASCE 7-93 per UBC97/ANSI95 if end verticals or cantilevers exist, they are exposed to wind. If porches exist, they are not exposed to wind. The lumber DOL increase is 1.33, and the plate grip increase is 1.33
 3) Gable requires continuous bottom chord bearing.
 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads per Table No. 16-B, UBC-97.



BRACING WOOD TRUSSES: COMMENTARY AND RECOMMENDATIONS

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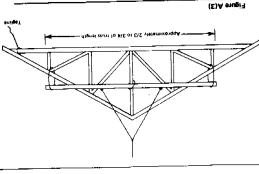


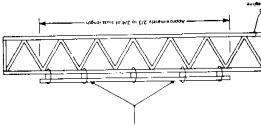
However carefully wood trusses are designed and fabricated, all this at stake in the final erection and bracing of a roof or floor system. It at this critical stage of construction that many of the really grificant design assumptions are either fulfilled or ignored. If ignored, le consequences may result in a collanse of the structure which at

The design of wood trusses in accordance with TPI design criteria

- Truss members are initially straight, uniform in cross section, and uniform in design properties.
- Trusses are plane structural components, installed vertically, braced to prevent lateral movement and parallel to each other

For lifting trusses with spans in excess of 60 feet, it is recommended that a strong-back be used as illustrated in Figure A(3). The atrong-back should be attached to the top chord and web members at oriong-back should be of approximately 10 feet. Further, the strong-backs should be at or above the mid-height of the truss on as to prevent overturning. The strong-back can be of any material with sufficient strength to safety extrong-back and the truss and sufficient rigidity to adequately resist the truss.



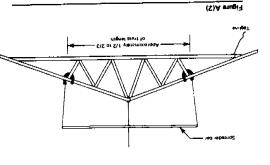


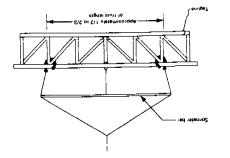
Trusses shall be installed plumb, at specified spacing and in-plane (i.e., trusses will be property aligned).

ERECTING TRUSSES. The truss erector or builder shall take the necessary precautions to insure that handling and erection procedures do not reduce the load-carrying capacity of the truss.

prior to everyon, they should be stored in a vertical position to prevent the pates. A further presention may be taken by providing a cover for the pates. A further presention may be taken by providing a cover for the pates will prevent moisture from coming in direct contact with the trusses and which can be ventilated to prevent condensation.

For truss spans between OC teet a suggested lifting procedure in short strict. The support in Colf suggested interest into spreader bar 'in-eon'. If these lines though the from the end of the spreader bar 'in-eon'. If they will tend to cause buckling of the truss.

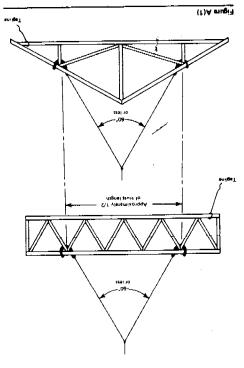




These recommendations for bracing wood trusses have been derived from the collective experience of leading technical personnel in the wood truss industry but must, due to the use of a qualified building involved, be presented only as a guide for the use of a qualified building designer, builder, or efection contractor.

user ome comprete pay or piagonal practing be installed at each and of any building, and additional such bays be located at specified intervals not to exceed 20 feet. Figure 5 illustrates the use of bracing in the

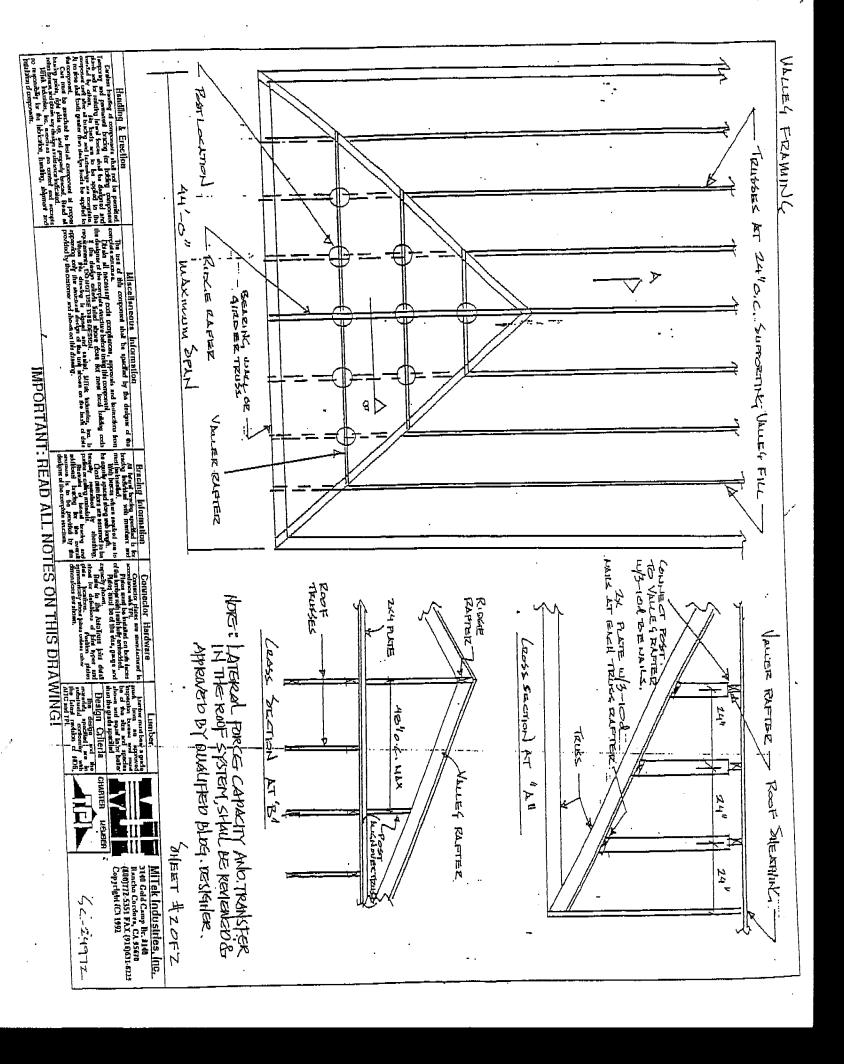
A suggested procedure for lifting trusses is illustrated in Figure A(1) if the truss span does not exceed 30 feet.



2. Web Member Plane. The purpose of this bracing is to hold the trustes in a vertical position and to maintain the design specing. In addition, this lateral bracing may be required to shorten the buckling.

It is recommended that this diagonal bracing, as shown in Figure (6U), be incritation both sides of the tidge line in all end bays. If the building exceeds 60 feet in length, this bracing should be repeated as intervals no greater than 20 feet.

Figure 6(a) illustrates the necessity for applying diagonal bracing in the plane of the top chord despite the use of closely spaced purlins.



DO NOT INSTALL THESE TRUSSES BEFORE READING INSTRUCTIONS.

ERECTOR

DO NOT INSTALL THESE TRUSSES BEFORE READING AND COMPLYING WITH THESE INSTRUCTIONS.

GENERAL CONTRACTOR

DO NOT LOAD THESE TRUSSES WITH PERMANENT OR TEMPORARY LOADS BEFORE READING THESE INSTRUCTIONS.

- 1. BEFORE ERECTING THESE TRUSSES ERECTOR SHOULD:
 - A. OBTAIN, STUDY AND COMPLY WITH "HANDLING, INSTALLING AND BRACING" (HIB 91) AS PUBLISHED BY TPI. OBTAIN THIS GUIDE FROM: THE FABRICATOR OR DEALER WHO PROVIDED THE TRUSSES OR TPI, 7411 RIGGS ROAD, HYATTSVILLE, MARYLAND 20783.

ALTERNATE

B. OBTAIN COMPLETE ERECTION BRACING PLAN FROM BUILDING ARCHITECT, ENGINEER OR STRUCTURALLY COMPETENT DESIGNER.

CAUTION

ERECTOR IS RESPONSIBLE FOR ALL DAMAGES OR INJURY AS A RESULT OF INADEQUATE BRACING FAILURES OCCURRING DURING ERECTION AND PRIOR TO INSTALLATION OF PERMANENT BRACING. COMPLIANCE WITH THIS INDUSTRY GUIDE IS IN YOUR BEST INTEREST.

(OVER)